

TRAFFIC IMPACT ANALYSIS – ADDENDUM #1

RESOLUTION COPPER MINE

SUPERIOR, ARIZONA

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PREPARED FOR RESOLUTION COPPER 402 W MAIN STREET SUPERIOR, ARIZONA 85173

> SOUTHWEST TRAFFIC ENGINEERING, LLC 3838 NORTH CENTRAL AVENUE, SUITE 1810 PHOENIX, AZ 85012 T 602.266.SWTE (7983) F 602.266.1115



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Traffic Counts Capacity Calculations Turn Lane Calculations Traffic Signal Warrant Analysis Crash Data

Prepared By:

Andrew Smigielski, PE, PTOE, PTP Shane Gutknecht, PE



TRAFFIC IMPACT ANALYIS - ADDENDUM #1 RESOLUTION COPPER MINE PROJECT SUPERIOR, ARIZONA

Project Description

Resolution Copper Mining is proposing an underground mine, ore processing operation with associated facilities, and infrastructure. West Plant Site (WPS) will be located just north of Superior, Arizona. East Plant Site (EPS) is located approximately six miles east of the WPS. Additionally, a new Filter Plant and Loadout Facility will be constructed east of San Tan Valley, Arizona, seven miles northeast of Magma Junction. A Tailings Storage Facility (TSF) will also be constructed at Skunk Camp south of Superior (the preferred alternative identified in the draft Environmental Impact Statement, near the Ray open pit mining complex). The proposed project facilities will be connected via a series of transmission lines, conveyors and pipelines. The vicinity of the project is shown in **Figure 1**.

The impacts of the proposed mine expansion were previously analyzed in the *Resolution Copper Mine Traffic Impact Analysis* (Original TIA) completed by Southwest Traffic Engineering on *13 April 2017*. The Original TIA evaluated traffic impacts to the surrounding roadway network based on two scenarios; peak construction activities and typical operations after construction is completed. Traffic volumes related to the expansion will be higher during peak construction activities than during normal operations.

In response to USFS (Untied States Forest Service) review of the Original TIA this updated traffic impact analysis addendum has been completed. There have also been discussions with the Arizona Department of Transportation (ADOT) regarding the Original TIA. Specifically, regarding the study intersections under ADOT control that have the potential to be negatively impacted by traffic associated with the mine expansion during peak construction.

This Addendum to the Original TIA (TIA Addendum) also further explores possible mitigation measures and evaluate crash rates at the intersections of N Smeltertown Road/Main Street, Silver King Mine Road/US 60 and Main Street/US 60.

The author of this report is a registered Professional Engineer (Civil) in the State of Arizona having specific expertise and experience in the preparation of traffic analyses.

Existing Conditions

United States Route 60 (US 60) has an east/west alignment and posted speed limits of 45 miles per hour (mph), 50 mph and 65 mph in the project area. US 60 is a regional route in the area linking Superior, Miami and Globe to the Phoenix metropolitan area. The roadway is undivided east of Superior and divided west of Superior.

Main Street is an undivided two-lane roadway through downtown Superior with an east/west alignment. Main Street has a posted speed limit of 35 mph west of N Smeltertown Road (analyzed in the Original TIA as Lonetree Road) and 25 mph east of N Smeltertown Road. There are existing overhead utility poles along the south side of the roadway.







Providing access to Tonto National Forest Service lands and various mining operations, Silver King Mine Road (Forest Service (FS) Road 229) exists as a two-lane graded dirt roadway with a north/south alignment. There is no posted speed limit on Silver King Mine Road (FS Road 229). Silver King Mine Road (FS Road 229) extends north of US 60. South of US 60 this roadway becomes FS Road 989.

As a two-lane graded dirt roadway, N Smeltertown Road provides access to Resolution Copper mining operations north of Main Street. There are no curb, gutter or sidewalks along N Smeltertown Road.

The un-signalized 'T' intersection of Main Street/US 60 is controlled by a STOP sign on the southbound approach. The approaches on US 60 are free flow. The eastbound and westbound approaches offer an exclusive left turn lane, a single through lane, and a shared through/right turn lane. Southbound traffic at the intersection of Main Street/US 60 offers an exclusive left turn lane.

The un-signalized intersection of Silver King Mine Road (FS Road 229)/US 60 is controlled by a STOP sign on the northbound and southbound approaches. Eastbound and westbound traffic on US 60 is free flow and provided with a left turn lane, a through lane and a shared through/right turn lane. A single shared left turn/through/right turn lane is offered for the northbound and southbound approaches.

The un-signalized 'T' intersection of N Smeltertown Road/Main Street is controlled by a STOP sign on the southbound approach. Eastbound and westbound traffic on Main Street is free flow. The eastbound approach offers a single shared left turn/through lane, while the westbound approach provides a shared through/right turn lane. The southbound approach to the intersection offers a single shared left turn/right turn lane. N Smeltertown Road currently provides access to existing mining Resolution Copper Mine facilities.

The study intersection locations, lane configurations, and intersection control are shown in **Figure 2**.

Updates to the Original TIA

The Original TIA evaluated the project intersections assuming that the Tailings Storage Facility would be located west of the West Plant Site. The preferred alternative was identified as Skunk Camp and as such the TIA has been updated to incorporate this decision. Access to the Tailing Storage Facility's previous location would be provided by the intersection of Silver King Mine Road/US 60. Now that the Tailing Storage Facility has been relocated to Skunk Camp the intersection of Silver King Mine Road/US 60 would be utilized only for equipment and material deliveries associated with the West Plant and is expected to experience less traffic.





Figure 2 – Existing Lane Configurations and Traffic Control

The Original TIA also assumed that a relatively large portion of mine-related traffic during peak construction would travel to/from an existing mine facility located just north of Superior, Arizona. This traffic would utilize Magma Avenue. In response to the DEIS comments from the Town of Superior and to reduce traffic on town roads, it is now proposed that all mine-related traffic associated with the West Plant Site facilities would utilize the existing entrance at the intersection of Main Street/N Smeltertown Road during construction and operations.

It is expected that almost all the vehicles traveling to/from the mine entrance at N Smeltertown Road would utilize the intersection of US 60/Main Street. This is the fastest and most direct route to the West Plant. Trips into Superior using Main Street (north of N Smeltertown Road) and/or Magma Avenue are expected to be limited and consist of employees utilizing restaurants and other amenities within the Town.



Existing Traffic Data

Peak hour turning movement traffic counts and 24-hour bi-directional traffic counts were utilized to form a basis for analysis of the project impacts. These traffic counts were originally presented in the Original TIA. Per discussion with ADOT, and based on seasonal factors, all traffic counts taken for the Original TIA were obtained on a Friday. This is the day traffic volumes are typically highest due to people traveling for the weekend in the region. The collection of counts on the peak day results in a conservative analysis approach where traffic levels would be analyzed at a much higher level when compared to 'typical week in/week out' traffic levels.

Traffic counts for the project were originally collected in summer (August 2015) and winter (November 2016). The winter traffic counts revealed higher traffic volumes. To ensure that the results and calculations presented were conservative, the Original TIA utilized only the winter traffic counts. All turning movement counts were taken between 7:00 AM and 10:00 PM.

Turning movement counts (taken for the Original TIA) at the following intersections were utilized in this TIA Addendum:

- Main Street/US 60
- Silver King Mine Road (FS Road 229)/US 60
- N Smeltertown Road/Main Street

In addition, the following Friday 24-hour bi-directional traffic volumes were utilized:

- Main Street, west of Pinal Avenue
- US 60, west of Silver King Mine Road (FS Road 229)
- US 60, between Silver King Mine Road (FS Road 229) and Main Street
- US 60, between Main Street and SR 177

The 2016 daily and peak hour traffic volumes are shown in **Figure 3**. Complete traffic count data can be found in the Appendix

A review of historical traffic data in the area showed increasing and decreasing traffic volumes. Despite this, a 2% annual traffic growth rate was assumed to estimate current 2020 traffic volumes resulting in a conservative approach as this is higher than ADOT anticipated growth rates on the study roadways. Using a 2% annual growth rate, 2020 (existing) weekday peak hour traffic volumes without the project were estimated as shown in **Figure 4**.

It should be mentioned that the AM and PM peak hour volumes presented in this TIA Addendum are not the same as the single daily peak hour presented in the Original TIA. However, in both cases the traffic volumes originated from the turning movement counts conducted between 7:00 AM and 10:00 PM in November 2016. To ensure a conservative analysis, the Original TIA considered the highest single peak hour of the day and it was assumed that all traffic from the mine would occur within this peak hour. A conservative analysis was appropriate to ensure that all potential impacts were accounted for. Based



on conversations with ADOT and in response to USFS review of the Original TIA, this TIA Addendum refines the expected impacts at specific study intersections. The majority of mine expansion related traffic will occur during the AM and PM peak hours as workers travel to and from the construction site, with the majority of inbound mine traffic arriving during the AM peak hour and most of the outbound mine traffic occurring during the PM peak hour. As a result, all analyses in this TIA Addendum consider the AM peak hour (between 7:00 AM and 9:00 AM) and PM peak hour (between 4:00 PM and 6:00 PM).

Trip Generation

The trips expected to be generated by the project during peak construction and normal operations are shown in **Table 1** and **Table 2**.

Table 1 – Weekday Project Site Generated Trips (Peak Construction)

Time Period	East Plant		West Plant		Skunk (Camp TSF	Filter Plant		
	Personnel	Materials and Equipment	Personnel	Materials and Equipment	Personnel	Materials and Equipment	Personnel	Materials and Equipment	
Peak Hour, Inbound (vtph)	219	11	498	11	21	11	30	8	
Peak Hour, Outbound (vtph)	219	11	498	11	21	11	30	8	
Total Peak	438	22	996	22	42	22	60	16	

-Personnel trips based on anticipated number of workers with a .66 shift reduction factor and a 1.7 divisor to account for carpooling. -Materials trips based on the materials and equipment quantities anticipated to be required during construction and a maximum of 11 trucks per hour. -vtpd - vehicle trips per day, vtph - vehicle trips per hour

Table 2 – Weekday Project Site Generated Trips (Normal Operations)

Time Period	East	Plant	West Plant		Plant Skunk Camp TSF		Filter Plant	
	Personnel	Materials and Equipment	Personnel	Materials and Equipment	Personnel	Materials and Equipment	Personnel	Materials and Equipment
Peak Hour, Inbound (vtph)	166	11	156	11	12	11	9	3
Peak Hour, Outbound (vtph)	166	11	156	11	12	11	9	3
Total Peak	332	22	312	22	24	22	18	6

-Personnel trips based on anticipated number of workers with a .66 shift reduction factor and a 1.7 divisor to account for carpooling.

-Materials trips based on the materials and equipment quantities anticipated to be required during construction and a maximum of 11 trucks per hour.

-vtpd - vehicle trips per day, vtph - vehicle trips per hour

It should be mentioned that the trip generation presented in this Addendum differs slightly from the Original TIA due to the proposed changes in development and relocation of the tailings storage facility. However, this change is minor and the overall trips generated by each facility is very similar in both documents.





Figure 3 – 2016 Weekday Peak Hour Traffic Volumes

Figure 4 – Existing (2020) Weekday Peak Hour Traffic Volumes





Trip Distribution & Assignment

Trip distribution for the project is described in the Original TIA. The primary consideration for the distribution was the relative accessibility of cities and towns near the site that could provide housing for construction workers. **Figure 5** shows the weekday trip distribution for the project as a percentage of net new primary trips.

The Original TIA weekday peak hour traffic assignment of trips expected to be generated by the project are shown in **Figure 6** (peak construction) and **Figure 7** (normal operations).

As previously mentioned, the traffic assignment in the Original TIA assumed a different location for the Tailings Storage Facility (previously located west of the West Plant Site, currently located at Skunk Camp). The Original TIA also assumed that a portion of mine-related traffic would travel to/from an existing mine facility just north of Superior via Magma Avenue. Magma Avenue is no longer expected to be utilized by traffic associated with the mine expansion. Mine expansion traffic would be required to enter/exit the existing main access point at Main Street/N Smeltertown Road via the intersection of Main Street/US 60.

Moreover, the Original TIA analyzed a single peak hour and assumed that all mine traffic occurred at one time. In actuality, the vast majority of inbound mine traffic is expected to occur during the AM peak hour and most of the outbound mine traffic will occur during the PM peak hour. As a result, the Original TIA analysis was conservative.

For the purposes of this TIA Addendum, the traffic assignment from the Original TIA was revised to account for the relocation of the Tailings Storage Facility, to account for the reassignment of traffic from Magma Avenue to N Smeltertown Road, and to account for AM and PM travel patterns. The revised assignments are shown in **Figure 8** (peak construction) and **Figure 9** (normal operations).



Figure 5 – Weekday Peak Hour Trip Distribution







Figure 6 – Original TIA Weekday Peak Hour Trip Assignment (Peak Construction)

Figure 7 – Original TIA Weekday Peak Hour Trip Assignment (Normal Operations)







Figure 8 – Revised Weekday Peak Hour Trip Assignment (Peak Construction)

Figure 9 – Revised Weekday Peak Hour Trip Assignment (Normal Operations)





Existing Traffic Operations

Analysis of current intersection operations was conducted for the weekday AM and PM peak hours using the nationally accepted methodology set forth in the *Highway Capacity Manual*, Transportation Research Board, 2016 (HCM 6). The computer software Synchro 10 was utilized to calculate the levels of service for individual movements and approaches.

LOS is a qualitative measure of the traffic operations at an intersection or on a roadway segment. Level of service is ranked from LOS A, which signifies little or no congestion and is the highest rank, to LOS F, which signifies congestion and jam conditions. LOS D is typically considered adequate operation at signalized and un-signalized intersections in developed areas.

At signalized intersections, level of service is calculated for each movement and then summed in a weighted fashion to yield the LOS for the approach and for the intersections as a whole. Criteria for level of service at signalized intersections are shown in **Table 3**.

Level-of-Service	Average Total Delay
А	\leq 10.0 seconds/vehicle
В	> 10.0 and ≤ 20.0 seconds/vehicle
С	> 20.0 and ≤ 35.0 seconds/vehicle
D	$>$ 35.0 and \leq 55.0 seconds/vehicle
Е	> 55.0 and ≤ 80.0 seconds/vehicle
F	> 80.0 seconds/vehicle

 Table 3 – Level of Service Criteria – Signalized Intersections

In calculating the levels of service, assumed signal phasing and timing data was used. Other assumptions included:

- Cycle length 90 seconds
- Lane widths 12 feet
- Approach grade -0%
- Right turn on red allowed

At un-signalized intersections, level of service is predicted/calculated for those movements, which must either stop for or yield to oncoming traffic and is based on average control delay for the particular movement. Control delay is the portion of total delay attributed to traffic control measures such as stop signs and traffic signals. The criteria for level of service at un-signalized intersections are shown in **Table 4**.



Level-of-Service	Delay
А	< 10 seconds/vehicle
В	> 10 and < 15 seconds/vehicle
С	> 15 and < 25 seconds/vehicle
D	> 25 and < 35 seconds/vehicle
Е	> 35 and < 50 seconds/vehicle
F	> 50 seconds/vehicle

Table 4 – Level of Service Criteria – Un-signalized Intersections

Table 5 shows the existing levels of service that were calculated for the study intersections. Complete capacity calculations are included in the Appendix.

Intersection		Peak	PM Peak		
	LOS	Delay	LOS	Delay	
Un-signalized Intersections					
Silver King Mine Road (FS Road 229)/US 60					
Eastbound Left	Α	8.1	Α	8.9	
Westbound Left	Α	7.9	Α	8.6	
Northbound Left/Through/Right	В	11.4	С	18.6	
Southbound Left/Through/Right	В	12.5	С	18.5	
Main Street/Smeltertown Road					
Eastbound Left	Α	0.0	Α	0.0	
Southbound Left/Right	Α	9.1	Α	9.2	
Main Street/US 60					
Eastbound Left/Through	Α	8.1	Α	9.1	
Southbound Left	В	13.5	С	22.7	
Southbound Right	А	9.5	В	10.8	

Table 5 – Existing Peak Hour Levels of Service

Delay - seconds per vehicle

As shown in **Table 5**, all movements at the study intersections currently operate at an adequate LOS.

Future Traffic Operations Without Project

Due to the uncertainty regarding the Environmental Impact Study (EIS) timeline, the results and recommendations outlined in this report are based upon an assumed peak construction year (2022) and operations starting year (2027). It should be noted that these 'assumed' years for peak construction and normal operations are necessary to account for the expected changes in traffic levels associated with the project and growth rates in traffic. While these assumed timelines may change due to project development demands, the results of the analysis are not expected to be impacted due to the conservative nature of the analysis if the timelines shift by a few years.



In order to assess the impacts of the project on future traffic operations, traffic projections were made for the years peak construction and normal operations.

A review of historical traffic data in the area showed increasing and decreasing traffic volumes. Despite this, a 2% growth rate was used to provide a conservative analysis. Using a 2% annual traffic growth rate, 2022 and 2027 weekday peak hour traffic volumes without the project were estimated as shown in **Figures 10** and **11**.

As with the current volumes, levels of service were calculated for each of the intersections in the study area for peak construction and normal operations without the project.

Levels of service for peak construction and normal operations without the project are shown in **Tables 6** and **7**. Complete capacity calculations are included in the Appendix.



Figure 10 – 2022 Weekday Peak Hour Traffic Volumes Without Project





Figure 11 – 2027 Weekday Peak Hour Traffic Volumes Without Project

Table 6 – 2022 Peak Hour Levels of Service Without Project

Intersection	AM	Peak	PM Peak		
	LOS	Delay	LOS	Delay	
Un-signalized Intersections					
Silver King Mine Road (FS Road 229)/US 60					
Eastbound Left	Α	8.1	Α	9.0	
Westbound Left	A 8.0		Α	8.8	
Northbound Left/Through/Right	В	11.8	С	20.3	
Southbound Left/Through/Right	В	12.9	С	20.0	
Main Street/Smeltertown Road					
Eastbound Left	Α	0.0	Α	0.0	
Southbound Left/Right	А	9.1	Α	9.2	
Main Street/US 60					
Eastbound Left/Through	Α	8.2	Α	9.3	
Southbound Left	В	14.2	D	25.7	
Southbound Right	А	9.7	В	11.1	

Delay - seconds per vehicle



Intersection		Peak	PM Peak		
	LOS	Delay	LOS	Delay	
Un-signalized Intersections					
Silver King Mine Road (FS Road 229)/US 60					
Eastbound Left	А	8.3	Α	9.3	
Westbound Left	Α	8.1	Α	9.0	
Northbound Left/Through/Right	В	12.3	С	23.1	
Southbound Left/Through/Right	В	13.6	С	22.8	
Main Street/Smeltertown Road					
Eastbound Left	Α	0.0	А	0.0	
Southbound Left/Right	Α	9.2	Α	9.3	
Main Street/US 60					
Eastbound Left/Through	Α	8.3	Α	9.5	
Southbound Left	В	14.9	D	28.3	
Southbound Right	А	9.8	В	11.4	

Table 7 – 2027 Peak Hour Levels of Service Without Project

Delay - seconds per vehicle

As shown in **Tables 6** and **7**, all movements at the study intersections are expected to operate at an adequate LOS in 2022 and 2027 without traffic from the project.

Future Traffic Volumes With Project

In order to assess the impacts of the project on future traffic operation, levels of service were calculated for each project intersection for peak construction and normal operations with the project. Weekday peak hour traffic volumes for 2022 and 2027 without the project were combined with the estimated trips generated by the project (**Figures 8** and 9) to yield weekday peak hour traffic volumes with the project as shown in **Figures 12** and **13**.

Weekday intersection levels of service for peak construction and normal operations with the project were then calculated as shown in **Tables 8** and **9**. Complete capacity calculations are included in the Appendix.



	2022 Without Project				2022 With Project			
Intersection	AM Peak		PM Peak		AM	Peak	PM Peak	
	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay
Un-signalized Intersections								
Silver King Mine Road (FS Road 229)/US 60								
Eastbound Left	Α	8.1	A	9.0	A	8.2	В	12.5
Westbound Left	Α	8.0	Α	8.8	В	10.5	Α	8.7
Northbound Left/Through/Right	В	11.8	С	20.3	D	25.2	D	32.1
Southbound Left/Through/Right	В	12.9	С	20.0	С	18.7	Е	41.8
Main Street/Smeltertown Road								
Eastbound Left	Α	0.0	Α	0.0	Α	9.7	Α	7.4
Southbound Left/Right	Α	9.1	А	9.2	В	11.2	С	20.9
Main Street/US 60								
Eastbound Left/Through	A	8.2	A	9.3	В	14.9	A	9.4
Southbound Left	В	14.2	D	25.7	F	>120	E	67.0
Southbound Right	А	9.7	В	11.1	В	10.0	F	111.9

Table 8 – 2022 Peak Hour Levels of Service With Project (Peak Construction)

Delay - seconds per vehicle

Table 9 – 2027 Peak Hour Levels of Service With Project (Normal Operations)

	202	27 With	out Pro	ject	2027 With Project					
Intersection	AM Peak		PM	Peak	AM	Peak	PM Peak			
	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay		
Un-signalized Intersections										
Silver King Mine Road (FS Road 229)/US 60										
Eastbound Left	Α	8.3	Α	9.3	Α	8.3	В	10.7		
Westbound Left	Α	8.1	Α	9.0	Α	9.1	Α	8.9		
Northbound Left/Through/Right		12.3	С	23.1	С	17.0	D	27.9		
Southbound Left/Through/Right	В	13.6	С	22.8	С	16.2	D	28.0		
Main Street/Smeltertown Road										
Eastbound Left	Α	0.0	Α	0.0	Α	7.6	Α	0.0		
Southbound Left/Right	Α	9.2	Α	9.3	В	12.7	Α	9.5		
Main Street/US 60										
Eastbound Left/Through	Α	8.3	А	9.5	Α	8.9	В	10.4		
Southbound Left	В	14.9	D	28.3	D	26.2	Е	45.6		
Southbound Right	Α	9.8	В	11.4	А	9.8	С	15.8		

Delay - seconds per vehicle





Figure 12 – 2022 Weekday Peak Hour Traffic Volumes With Project

Figure 13 – 2027 Weekday Peak Hour Traffic Volumes With Project





Table 8 and **Table 9** show that delays are expected to occur during peak construction and normal operations at the intersection of Main Street/US 60 with traffic from the proposed mine expansion. Delays are expected to be experienced by the southbound left and southbound right turning movements during peak construction (this volume of traffic only exists for approximately two years during the entire construction period). Traffic volumes are expected to decrease during other times of construction and during operations normal. As a result, only the southbound left turning movement is expected to experience delays during normal operations with the project.

The southbound movement at the intersection of Silver King Mine Road/US 60 is expected to operate at an inadequate LOS with traffic from peak construction. This delay is temporary (lasting approximately two years) and is expected to alleviate to an adequate level after traffic volumes drop on US 60 as construction activities slow and normal operations begin.

Turn Lane Analysis

A key element of this study is to determine if new left and/or right turn lanes are required at the study intersections. *ADOT Traffic Engineering Guidelines and Processes (TGP) 245* - *Turn Lane Warrants* provides warrants for the inclusion of left and right turn lanes based on speed limit, through traffic volume and turning traffic volume during the peak hour. When needed, turn lanes remove the slowing turning traffic from the through traffic stream, improving capacity and reducing rear-end accidents. **Table 10** shows the locations that were evaluated for turn lanes based on expected traffic volumes during peak construction and normal operations.

Intersection	Direction	Turn Treatment Analyzed	Turn Treatments Warranted? Peak Construction	Turn Treatments Warranted? Normal Operations
Silver King Mine Deed/US 60	Eastbound	Left Turn Lane	Existing	Existing
Silver King Mine Road/US 60	Westbound	Right Turn Lane	No	No
Main Street/US 60	Eastbound	Left Turn Lane	Existing	Existing
Main Street/US 60	Westbound	Right Turn Lane	Yes	No
Smaltantarra Daad/Main Streat	Eastbound	Left Turn Lane	Yes	Yes
Sineitertown Road/Iviain Street	Westbound	Right Turn Lane	No	No

 Table 10 – Turn Lane Warrants

Table 10 shows that an eastbound left turn lane is warranted at the intersection of N Smeltertown Road/Main Street during peak construction and during normal operations. It is recommended that this turn lane be constructed.



A westbound right turn lane is warranted at the intersection of Main Street/US 60 during peak construction. However, this same turn lane is not warranted during normal operations. Benefits of this turn lane are limited and it is recommended that a westbound right turn lane at Main Street/US 60 not be constructed. Although briefly warranted during construction activities, a turn lane is not expected to improve operations at the intersection and would be largely unutilized during normal operations.

Queue storage requirements for the recommended and warranted turn lane was calculated utilizing Synchro 10 95th percentile calculations. Typically, an average vehicle length of 25 feet is assumed.

 Table 11 shows the calculated queue lengths for the warranted and recommended eastbound left turn lane at the intersection of N Smeltertown Road/Main Street during peak construction and normal operations.

Intersection	Left Turn Storage				Left Turn Storage			
	NB	SB	EB	WB	NB	SB	EB	WB
Smeltertown Road/Main Street								
Turning Volume (vph)			664				10	
Vehicles in Queue			5.5				0.7	
S _{calculated} =			138				18	
S _{rounded} =			150				25	

Table 11 – Calculated Queue Lengths

S - storage in feet, vph - vehicles per hour

As shown in **Table 11**, the anticipated eastbound left turning queue at the intersection of N Smeltertown Road/Main Street is 150 feet during peak construction and 25 feet during normal operations.

Traffic Signal Warrant Analysis

Traffic signal warrant analyses were performed at the intersections of Silver King Mine Road/US 60 and Main Street/US 60 to determine if a traffic signal is needed as a mitigation measure during peak construction and/or normal operations.

The intersection of Main Street/US 60 was analyzed based on existing traffic volumes as well as future traffic volumes without and with the project.

The *Manual on Uniform Traffic Control Devices (MUTCD)*, Federal Highway Administration, 2009, lists nine warrants that are used to determine if a traffic signal should be considered for installation at an intersection. A traffic signal may be warranted if one or more of the warrants are satisfied. Warrants #1 (Eight Hour Volume) and #2 (Four Hour Vehicular Volume) were used to evaluate the need to signalize the intersection. Based on



existing conditions, availability of information, and applicability, the remaining warrants (#3, #4, #5, #6, #7, #8, and #9) do not apply to the given conditions.

Warrant #1 (Eight Hour Volume) is satisfied when for at least eight (8) hours of an average day, specific traffic volume levels are met for both the major and minor streets (Condition A – Minimum Vehicular Volume). The MUTCD states these volumes depend on the vehicles per hour (vph) combined for both approaches of the major street, and for the highest volume approach on the minor street. The values vary depending on the number of approach lanes and the 85th percentile speed of the roadways.

Warrant #1 also applies to operating conditions where the major street traffic levels are sufficiently high that traffic entering or crossing from a minor street suffers excessive delay (Condition B – Interruption of Continuous Traffic). Once again, the warrant is satisfied when for each of any of the same eight (8) hours of an average day, specific traffic volume levels are met for both the major and minor streets.

Warrant #2 (Four Hour Volume) is met when, for any four hours of the average day on both the major and minor streets, the hourly approach volumes are above the plotted curve contained in the MUTCD (see Appendix).

Tables 12 and **13** shows the results of the warrant analyses at the study intersections based on traffic volumes in the existing conditions, 2022 without traffic from project, 2027 without traffic form the project, 2022 during peak construction, and 2027 during normal operations. Complete traffic signal warrant calculations can be found in the Appendix.

	Warrant Number										
Silver King Mine Road/US 60	1	l		2	4	5		7	0	0	
	Condition A	Condition B		3	4	5	0		0	9	
Existing	No	No	No	*	*	*	*	*	*	*	
Hours Met	0	0	0	*	*	*	*	*	*	*	
2022 Without	No	No	No	*	*	*	*	*	*	*	
Hours Met	0	0	0	*	*	*	*	*	*	*	
2027 Without	No	No	No	*	*	*	*	*	*	*	
Hours Met	0	0	0	*	*	*	*	*	*	*	
2022 With (Peak Construction)	No	No	No	*	*	*	*	*	*	*	
Hours Met	0	0	0	*	*	*	*	*	*	*	
2027 With (Normal Operations)	No	No	No	*	*	*	*	*	*	*	
			110	~	*	*	*	~	*	*	
Hours Met	0	0	0	*	*	*	*	*	*	*	

Table 12 – Traffic Signal Warrant Analysis (Silver King Mine Road/US 60)

* Warrant Not Evaluated



	Warrant Number										
Main Street/US 60	1	2	3	4	-	6	7	0	0		
	Condition A	Condition B		5	4	3	0	/	0	9	
Existing	No	No	No	*	*	*	*	*	*	*	
Hours Met	0	0	0	*	*	*	*	*	*	*	
2022 Without	No	No	No	*	*	*	*	*	*	*	
Hours Met	0	0	0	*	*	*	*	*	*	*	
2027 Without	No	No	No	*	*	*	*	*	*	*	
Hours Met	0	3	0	*	*	*	*	*	*	*	
2022 With (Peak Construction)	No	No	No	*	*	*	*	*	*	*	
Hours Met	1	2	1	*	*	*	*	*	*	*	
2027 With (Normal Operations)	No	No	No	*	*	*	*	*	*	*	
Hours Met	1	3	1	*	*	*	*	*	*	*	

Table 13 – Traffic Signal Warrant Analysis (Main Street/US 60)

* Warrant Not Evaluated

Tables 12 and **13** show that traffic signal warrants #1 (Conditions A and B) and #2 (Four Hour Volume) are not expected to be met at the intersections of Silver King Mine Road/US 60 or Main Street/US 60 during peak construction or during normal operations.

Crash Summary

Crash data on US 60, at Silver King Mine Road and Main Street, was obtained from ADOT's Traffic Records Section and reviewed as a part of this traffic analysis to determine if any trends can be observed. Records for the most recent five-year period were reviewed (2014 to 2018).

Results of the crash analysis are shown in Tables 14 and 15.

Year	Angle	Left Turn	Rear- End	Sideswipe	Single Vehicle	Head On	Other	Fatal	Injury	Totals	
2014	0	0	0	0	0	0	0	0	0	0	
2015	0	0	0	0	0	0	0	0	0	0	
2016	0	0	0	0	0	0	0	0	0	0	
2017	0	0	0	0	0	0	0	0	0	0	
2018	0	0	0	0	0	0	0	0	0	0	
5-Year Total	0	0	0	0	0	0	0	0	0	0	

Table 14 – Silver King Mine Road/US 60 Crash Summary



					~ .						
Year	Angle	Left Turn	Rear- End	Sideswipe	Single Vehicle	Head On	Other	Fatal	Injury	Totals	
2014			1					0	0	1	
2015		1						0	0	1	
2016								0	0	0	
2017								0	0	0	
2018								0	0	0	
5-Year Total	0	1	1	0	0	0	0	0	0	2	

Table 15 – Main Street/US 60 Crash Summary

No crashes have been reported at the intersection of Silver King Mine Road/US 60 in the last five years for which data is available (2014-2018). One crash was reported at Main Street/US 60 in 2014 and one was reported in 2015. No crashes were reported at this intersection in 2016, 2017 or 2018.

The available crash data does not reveal any crash patterns or trends at the study intersections.

An expanded summary of the crash data can be found in the Appendix.

The mine expansion is expected to increase traffic at the study intersections during peak construction and during normal operations. Any traffic increase has the potential to increase crashes; however, the mine expansion is not expected to significantly influence crash patterns at the study intersections or elsewhere in the Town of Superior. The number of trips expected to be generated by the mine expansion are in line with any relatively small 'strip-mall' or shopping center: developments that are routinely analyzed and discussed with no expectations of town-wide, regional safety implications. While mitigation measures may be necessary at specific intersections or for specific movements (e.g. an eastbound left turn lane at N Smeltertown Road/Main Street) US 60 is operating well below capacity and it is expected that this traffic can be accommodated within the existing roadway system.

Mine-related cut-through traffic in the Town of Superior is expected to be minimal. US-60 to Main Street is a faster route to the mine entrance on N Smeltertown Road when compared to cutting through Superior. Mine-related traffic traveling to the West Plant during construction and normal operations will utilize Main Street between US-60 and N Smeltertown Road. This 0.02 mile section of Main Street represents less than 1% of the total roadway network within Superior (approximately 24.2 miles).



Mitigation

The southbound movement at the intersection of Silver King Mine Road/US 60 is expected to operate at an inadequate LOS with traffic from peak construction. This delay is expected to alleviate to an adequate level after construction ends and traffic volumes drop during normal operations. These temporary delays are not expected to impact through traffic on US 60 and adequate storage is available on Silver King Mine Road for queuing southbound vehicles. No mitigation measures are recommended.

During peak construction the southbound turning movements at the intersection of Main Street/US 60 are expected to experience delays during the peak hours. The only way to mitigate these delays, for two hours per day, is a traffic signal. A traffic signal is not recommended. Traffic analyses must also take into account the remaining twenty-two hours of the day. During these 'off peak' times, significantly less vehicles are expected to be traveling to/from Main Street via US 60. Any benefits experienced during the peak hours will be outweighed by the negative impacts to US 60 during the remainder of the day. Moreover, these delays are based on the absolute peak construction activities. During most of the construction period, traffic volumes are expected to be much less than analyzed in this document. These factors, combined with the fact that these delays will alleviate to adequate levels once typical mine operations begin, make a traffic signal an excessive mitigation measure that will result in a negative impact on the traveling public during non-peak daytime and nighttime hours due to unnecessary disruption in traffic flow.

Conclusion

All movements at the intersection of Main Street/US 60 and N Smeltertown Road/Main currently operate at an adequate LOS. These intersections are expected to continue operating adequately during peak construction and normal operations without traffic from the proposed mine expansion.

The southbound movement at the intersection of Silver King Mine Road/US 60 is expected to operate at an inadequate LOS with traffic from peak construction. This delay is temporary (approximately 2 years) and is expected to alleviate to an adequate level after construction ends and traffic volumes drop during normal operations. These temporary delays are not expected to impact through traffic on US 60 and adequate storage is available on Silver King Mine Road for queuing southbound vehicles. No mitigation measures are recommended.

During peak construction the southbound turning movements at the intersection of Main Street/US 60 are expected to experience delays during the peak hours. The only way to mitigate these delays, for two hours per day, is a traffic signal. A traffic signal is not recommended. Traffic analyses must also take into account the remaining twenty-two hours of the day. During these 'off peak' times, significantly less vehicles are expected to be traveling to/from Main Street via US 60. Any benefits experienced during the peak hours will be outweighed by the negative impacts to US 60 during the remainder of the day.



Moreover, these delays are based on the absolute peak construction activities. During most of the construction period, traffic volumes are expected to be much less than analyzed in this document. These factors, combined with the fact that these delays will alleviate to adequate levels once typical mine operations begin, make a traffic signal an excessive mitigation measure that will result in a negative impact on the traveling public during nonpeak daytime and nighttime hours due to unnecessary disruption in traffic flow.

Traffic signal warrants #1 (Conditions A and B) and #2 (Four Hour Volume) are not met at the intersections of Silver King Mine Road/US 60 or Main Street/US 60 during peak construction or during normal operations.

The available crash data from the intersections of Silver King Mine Road/Main Street and Main Street/US 60 does not reveal any crash patterns or trends. The increase in traffic generated by the mine during peak construction and normal operations is not expected to have a significant impact on the crash patterns in the study area or at the study intersections.

Figure 14 shows the proposed lane configuration and traffic control at the project intersections during peak construction and normal operations.

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Figure 14 – Proposed Lane Configurations and Traffic Control With Project (Peak Construction)





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APPENDIX

Traffic Counts

Capacity Calculations

Turn Lane Calculations

Traffic Signal Warrant Analysis

Crash Data



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APPENDIX

Traffic Counts











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APPENDIX

Capacity Calculations
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Intersection						
Int Delay, s/veh	1.2					
Movement	EDI	EDT	\//DT		C/V/I	C/V/D
wovernent	LDL	LDI	VVDI	WDR	SVVL	JAK
Lane Configurations	<u>۲</u>	- 11	_ ≜ î≽		- ሽ	1
Traffic Vol, veh/h	43	245	305	2	1	38
Future Vol, veh/h	43	245	305	2	1	38
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	125	0
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	80	90	90	90	80	80
Heavy Vehicles, %	2	2	4	2	2	2
Mvmt Flow	54	272	339	2	1	48
Peak Hour Factor Heavy Vehicles, % Mvmt Flow	80 2 54	90 2 272	90 4 339	90 2 2	80 2 1	80 2 48

Major/Minor	Major1	Ma	ajor2	Ν	1inor2		
Conflicting Flow All	341	0	-	0	584	171	
Stage 1	-	-	-	-	340	-	
Stage 2	-	-	-	-	244	-	
Critical Hdwy	4.14	-	-	-	6.84	6.94	
Critical Hdwy Stg 1	-	-	-	-	5.84	-	
Critical Hdwy Stg 2	-	-	-	-	5.84	-	
Follow-up Hdwy	2.22	-	-	-	3.52	3.32	
Pot Cap-1 Maneuver	1215	-	-	-	443	843	
Stage 1	-	-	-	-	692	-	
Stage 2	-	-	-	-	774	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1215	-	-	-	424	843	
Mov Cap-2 Maneuver	-	-	-	-	424	-	
Stage 1	-	-	-	-	662	-	
Stage 2	-	-	-	-	774	-	
Annroach	FR		W/R		SW		
HCM Control Delay	1 ?		0		9.6		
LCM LOS	1.3		0		- 7 .0		
					A		

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBRSWLn1	SWLn2
Capacity (veh/h)	1215	-	-	- 424	843
HCM Lane V/C Ratio	0.044	-	-	- 0.003	0.056
HCM Control Delay (s)	8.1	-	-	- 13.5	9.5
HCM Lane LOS	А	-	-	- B	А
HCM 95th %tile Q(veh)	0.1	-	-	- 0	0.2

Int Delay, s/veh	0.1						
Movement	SBL	SBR	NEL	NET	SWT	SWR	
Lane Configurations	۰¥			- 4	f		
Traffic Vol, veh/h	1	0	0	51	39	1	
Future Vol, veh/h	1	0	0	51	39	1	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage	,# 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	80	80	80	80	80	80	
Heavy Vehicles, %	2	2	2	15	7	2	
Mvmt Flow	1	0	0	64	49	1	

Major/Minor	Minor2	l	Major1	Maj	or2		
Conflicting Flow All	114	50	50	0	-	0	
Stage 1	50	-	-	-	-	-	
Stage 2	64	-	-	-	-	-	
Critical Hdwy	6.42	6.22	4.12	-	-	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	2.218	-	-	-	
Pot Cap-1 Maneuver	882	1018	1557	-	-	-	
Stage 1	972	-	-	-	-	-	
Stage 2	959	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	882	1018	1557	-	-	-	
Mov Cap-2 Maneuver	882	-	-	-	-	-	
Stage 1	972	-	-	-	-	-	
Stage 2	959	-	-	-	-	-	

Approach	SB	NE	SW
HCM Control Delay, s	9.1	0	0
HCM LOS	А		

Minor Lane/Major Mvmt	NEL	NET S	BLn1	SWT	SWR	
Capacity (veh/h)	1557	-	882	-	-	
HCM Lane V/C Ratio	-	-	0.001	-	-	
HCM Control Delay (s)	0	-	9.1	-	-	
HCM Lane LOS	А	-	А	-	-	
HCM 95th %tile Q(veh)	0	-	0	-	-	

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ľ	∱ î,		5	∱î ∌			÷			\$	
Traffic Vol, veh/h	3	281	3	4	346	6	6	0	6	4	0	2
Future Vol, veh/h	3	281	3	4	346	6	6	0	6	4	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	350	-	-	225	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	312	3	4	384	7	7	0	7	4	0	2

Major/Minor	Major1		Ν	Major2		1	Minor1		Ν	/linor2			
Conflicting Flow All	391	0	0	315	0	0	520	719	158	558	717	196	
Stage 1	-	-	-	-	-	-	320	320	-	396	396	-	
Stage 2	-	-	-	-	-	-	200	399	-	162	321	-	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32	
Pot Cap-1 Maneuver	1164	-	-	1242	-	-	439	353	859	412	354	812	
Stage 1	-	-	-	-	-	-	666	651	-	601	602	-	
Stage 2	-	-	-	-	-	-	783	601	-	824	650	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1164	-	-	1242	-	-	436	351	859	407	352	812	
Mov Cap-2 Maneuver	-	-	-	-	-	-	436	351	-	407	352	-	
Stage 1	-	-	-	-	-	-	664	649	-	599	600	-	
Stage 2	-	-	-	-	-	-	778	599	-	815	648	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0.1			0.1			11.4			12.5			
HCM LOS							В			В			
Minor Lane/Major Mvm	nt N	VBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1				

Minor Lane/Major NVMt	INREUT	FRF	FRI	FRK MRF	WRI	MRK 2RFUI	
Capacity (veh/h)	578	1164	-	- 1242	-	- 488	
HCM Lane V/C Ratio	0.023	0.003	-	- 0.004	-	- 0.014	
HCM Control Delay (s)	11.4	8.1	-	- 7.9	-	- 12.5	
HCM Lane LOS	В	А	-	- A	-	- B	
HCM 95th %tile Q(veh)	0.1	0	-	- 0	-	- 0	

Intersection

Int Delay, s/veh	1.1					
Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	<u>ار</u>	^	- † 14		۲,	1
Traffic Vol, veh/h	56	488	559	2	6	50
Future Vol, veh/h	56	488	559	2	6	50
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	125	0
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	80	90	90	90	80	80
Heavy Vehicles, %	2	2	4	2	2	2
Mvmt Flow	70	542	621	2	8	63

Major/Minor	Major1	Maj	or2	Ν	/linor2	
Conflicting Flow All	623	0	-	0	1033	312
Stage 1	-	-	-	-	622	-
Stage 2	-	-	-	-	411	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	954	-	-	-	228	684
Stage 1	-	-	-	-	498	-
Stage 2	-	-	-	-	638	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	· 954	-	-	-	211	684
Mov Cap-2 Maneuver	•	-	-	-	211	-
Stage 1	-	-	-	-	462	-
Stage 2	-	-	-	-	638	-
Annroach	FR	1	NR		SW	
HCM Control Dolay	<u></u>		0		12.1	

now control Delay, s	 0	12.1	
HCM LOS		В	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBRSWLn1	SWLn2
Capacity (veh/h)	954	-	-	- 211	684
HCM Lane V/C Ratio	0.073	-	-	- 0.036	0.091
HCM Control Delay (s)	9.1	-	-	- 22.7	10.8
HCM Lane LOS	А	-	-	- C	В
HCM 95th %tile Q(veh)	0.2	-	-	- 0.1	0.3

Int Delay, s/veh	0.2								
Movement	SBL	SBR	NEL	NET	SWT	SWR			
Lane Configurations	Y			÷.	et				
Traffic Vol, veh/h	2	0	0	51	49	0			
Future Vol, veh/h	2	0	0	51	49	0			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	-	None	-	None	-	None			
Storage Length	0	-	-	-	-	-			
Veh in Median Storage,	# 0	-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	80	80	80	80	80	80			
Heavy Vehicles, %	2	2	2	15	7	2			
Mvmt Flow	3	0	0	64	61	0			

Major/Minor	Minor2		Major1	Maj	jor2		
Conflicting Flow All	125	61	61	0	-	0	
Stage 1	61	-	-	-	-	-	
Stage 2	64	-	-	-	-	-	
Critical Hdwy	6.42	6.22	4.12	-	-	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	2.218	-	-	-	
Pot Cap-1 Maneuver	870	1004	1542	-	-	-	
Stage 1	962	-	-	-	-	-	
Stage 2	959	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	870	1004	1542	-	-	-	
Mov Cap-2 Maneuver	870	-	-	-	-	-	
Stage 1	962	-	-	-	-	-	
Stage 2	959	-	-	-	-	-	

Approach	SB	NE	SW	
HCM Control Delay, s	9.2	0	0	
HCM LOS	А			

Minor Lane/Major Mvmt	NEL	NET S	SBLn1	SWT	SWR
Capacity (veh/h)	1542	-	870	-	-
HCM Lane V/C Ratio	-	-	0.003	-	-
HCM Control Delay (s)	0	-	9.2	-	-
HCM Lane LOS	А	-	А	-	-
HCM 95th %tile Q(veh)	0	-	0	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	↑ ĵ≽		1	∱î ≽			÷			¢	
Traffic Vol, veh/h	1	514	4	3	595	0	3	0	1	2	2	3
Future Vol, veh/h	1	514	4	3	595	0	3	0	1	2	2	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	350	-	-	225	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	571	4	3	661	0	3	0	1	2	2	3

Major/Minor	Major1		N	lajor2		Ν	1inor1		Ν	/linor2			
Conflicting Flow All	661	0	0	575	0	0	913	1242	288	955	1244	331	
Stage 1	-	-	-	-	-	-	575	575	-	667	667	-	
Stage 2	-	-	-	-	-	-	338	667	-	288	577	-	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32	
Pot Cap-1 Maneuver	923	-	-	994	-	-	228	173	709	213	173	665	
Stage 1	-	-	-	-	-	-	470	501	-	414	455	-	
Stage 2	-	-	-	-	-	-	650	455	-	695	500	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	923	-	-	994	-	-	224	172	709	212	172	665	
Mov Cap-2 Maneuver	-	-	-	-	-	-	224	172	-	212	172	-	
Stage 1	-	-	-	-	-	-	470	500	-	414	454	-	
Stage 2	-	-	-	-	-	-	642	454	-	693	500	-	
Approach	FB			WB			NR			SB			
HCM Control Delay s		-		0			18.6			18.5			
HCM LOS	0			0			10.0 C			10.0 C			
							Ŭ			Ū			

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1		
Capacity (veh/h)	270	923	-	-	994	-	-	274		
HCM Lane V/C Ratio	0.016	0.001	-	-	0.003	-	-	0.028		
HCM Control Delay (s)	18.6	8.9	-	-	8.6	-	-	18.5		
HCM Lane LOS	С	А	-	-	А	-	-	С		
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1		

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ľ	↑ ĵ≽		5	∱î ∌			÷			\$	
Traffic Vol, veh/h	3	292	3	4	360	6	6	0	6	4	0	2
Future Vol, veh/h	3	292	3	4	360	6	6	0	6	4	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	350	-	-	225	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	85	85	80	90	90	80	80	80	80	80	80
Heavy Vehicles, %	2	17	2	2	7	2	2	2	2	2	2	2
Mvmt Flow	4	344	4	5	400	7	8	0	8	5	0	3

Major/Minor	Major1		Ν	Major2			Vinor1		Ν	/linor2			
Conflicting Flow All	407	0	0	348	0	0	564	771	174	594	770	204	
Stage 1	-	-	-	-	-	-	354	354	-	414	414	-	
Stage 2	-	-	-	-	-	-	210	417	-	180	356	-	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32	
Pot Cap-1 Maneuver	1148	-	-	1208	-	-	408	329	839	389	330	803	
Stage 1	-	-	-	-	-	-	636	629	-	586	591	-	
Stage 2	-	-	-	-	-	-	773	590	-	804	628	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1148	-	-	1208	-	-	404	327	839	383	328	803	
Mov Cap-2 Maneuver	-	-	-	-	-	-	404	327	-	383	328	-	
Stage 1	-	-	-	-	-	-	634	627	-	584	589	-	
Stage 2	-	-	-	-	-	-	767	588	-	794	626	-	
Approach	FB			WB			NB			SB			
HCM Control Delay, s	0.1			0.1			11.8			12.9			
HCM LOS							В			В			
Minor Lane/Maior Myn	nt	NBI n1	FBI	FBT	FBR	WBI	WBT	WBR	SBI n1				

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1	
Capacity (veh/h)	545	1148	-	-	1208	-	-	464	
HCM Lane V/C Ratio	0.028	0.003	-	-	0.004	-	-	0.016	
HCM Control Delay (s)	11.8	8.1	-	-	8	-	-	12.9	
HCM Lane LOS	В	А	-	-	А	-	-	В	
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0	

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Intersection						
Int Delay, s/veh	1.2					
Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	۲.	^	∱ ₿		۲.	1
Traffic Vol, veh/h	44	255	317	2	1	40
Future Vol, veh/h	44	255	317	2	1	40
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	125	0
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	80	85	85	85	80	80
Heavy Vehicles, %	2	2	4	2	2	2
Mvmt Flow	55	300	373	2	1	50

Major/Minor	Major1	Ma	jor2	Ν	1inor2		
Conflicting Flow All	375	0	-	0	634	188	
Stage 1	-	-	-	-	374	-	
Stage 2	-	-	-	-	260	-	
Critical Hdwy	4.14	-	-	-	6.84	6.94	
Critical Hdwy Stg 1	-	-	-	-	5.84	-	
Critical Hdwy Stg 2	-	-	-	-	5.84	-	
Follow-up Hdwy	2.22	-	-	-	3.52	3.32	
Pot Cap-1 Maneuver	1180	-	-	-	411	822	
Stage 1	-	-	-	-	666	-	
Stage 2	-	-	-	-	760	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1180	-	-	-	392	822	
Mov Cap-2 Maneuver	-	-	-	-	392	-	
Stage 1	-	-	-	-	635	-	
Stage 2	-	-	-	-	760	-	
Approach	FB		WB		SW		
HCM Control Delay s	1.3		0		9.8		
HCM LOS			Ū		A		

Vinor Lane/Major Mvmt	EBL	EBT	WBT	WBRSWLn1	SWLn2
Capacity (veh/h)	1180	-	-	- 392	822
HCM Lane V/C Ratio	0.047	-	-	- 0.003	0.061
HCM Control Delay (s)	8.2	-	-	- 14.2	9.7
HCM Lane LOS	А	-	-	- B	А
HCM 95th %tile Q(veh)	0.1	-	-	- 0	0.2

Int Delay, s/veh	0.1					
Movement	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations	۰¥			- 4	el 👘	
Traffic Vol, veh/h	1	0	0	53	41	1
Future Vol, veh/h	1	0	0	53	41	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	2	2	2	15	7	2
Mvmt Flow	1	0	0	66	51	1

Major/Minor	Minor2	I	Major1	M	ajor2	
Conflicting Flow All	118	52	52	0	-	0
Stage 1	52	-	-	-	-	-
Stage 2	66	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	878	1016	1554	-	-	-
Stage 1	970	-	-	-	-	-
Stage 2	957	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	878	1016	1554	-	-	-
Mov Cap-2 Maneuver	878	-	-	-	-	-
Stage 1	970	-	-	-	-	-
Stage 2	957	-	-	-	-	-
Ammanah	CD				CIM	

Approach	SB	NE	SW	
HCM Control Delay, s	9.1	0	0	
HCM LOS	А			

Minor Lane/Major Mvmt	NEL	NET SE	BLn1	SWT	SWR	
Capacity (veh/h)	1554	-	878	-	-	
HCM Lane V/C Ratio	-	- 0	.001	-	-	
HCM Control Delay (s)	0	-	9.1	-	-	
HCM Lane LOS	А	-	А	-	-	
HCM 95th %tile Q(veh)	0	-	0	-	-	

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ľ	_ ^ ↑₽		۲.	∱î ≽			\$			\$	
Traffic Vol, veh/h	1	534	4	3	619	0	3	0	1	2	2	3
Future Vol, veh/h	1	534	4	3	619	0	3	0	1	2	2	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	350	-	-	225	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	85	85	80	90	90	80	80	80	80	80	80
Heavy Vehicles, %	2	17	2	2	7	2	2	2	2	2	2	2
Mvmt Flow	1	628	5	4	688	0	4	0	1	3	3	4

Major/Minor	Major1		N	lajor2		Ν	/linor1		N	/linor2			
Conflicting Flow All	688	0	0	633	0	0	987	1329	317	1012	1331	344	
Stage 1	-	-	-	-	-	-	633	633	-	696	696	-	
Stage 2	-	-	-	-	-	-	354	696	-	316	635	-	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32	
Pot Cap-1 Maneuver	902	-	-	946	-	-	202	154	679	193	153	652	
Stage 1	-	-	-	-	-	-	434	472	-	398	441	-	
Stage 2	-	-	-	-	-	-	636	441	-	670	471	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	902	-	-	946	-	-	198	153	679	192	152	652	
Mov Cap-2 Maneuver	-	-	-	-	-	-	198	153	-	192	152	-	
Stage 1	-	-	-	-	-	-	434	472	-	398	439	-	
Stage 2	-	-	-	-	-	-	626	439	-	668	471	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0			0			20.3			20			
HCM LOS							С			С			

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1	
Capacity (veh/h)	241	902	-	-	946	-	-	248	
HCM Lane V/C Ratio	0.021	0.001	-	-	0.004	-	-	0.035	
HCM Control Delay (s)	20.3	9	-	-	8.8	-	-	20	
HCM Lane LOS	С	А	-	-	А	-	-	С	
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1	

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Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	ኘ	^	đβ		5	1
Traffic Vol, veh/h	58	507	582	2	6	52
Future Vol, veh/h	58	507	582	2	6	52
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	125	0
Veh in Median Storage,	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	80	85	85	85	80	80
Heavy Vehicles, %	2	2	4	2	2	2
Mvmt Flow	73	596	685	2	8	65

Major/Minor	Major1	Maj	or2	Ν	/linor2		
Conflicting Flow All	687	0	-	0	1130	344	
Stage 1	-	-	-	-	686	-	
Stage 2	-	-	-	-	444	-	
Critical Hdwy	4.14	-	-	-	6.84	6.94	
Critical Hdwy Stg 1	-	-	-	-	5.84	-	
Critical Hdwy Stg 2	-	-	-	-	5.84	-	
Follow-up Hdwy	2.22	-	-	-	3.52	3.32	
Pot Cap-1 Maneuver	903	-	-	-	197	652	
Stage 1	-	-	-	-	461	-	
Stage 2	-	-	-	-	614	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	· 903	-	-	-	181	652	
Mov Cap-2 Maneuver	· _	-	-	-	181	-	
Stage 1	-	-	-	-	424	-	
Stage 2	-	-	-	-	614	-	
Approach	ED		MD		C/W		

Approach	EB	WB	SW	
HCM Control Delay, s	1	0	12.6	
HCM LOS			В	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBRSWLn1S	WLn2
Capacity (veh/h)	903	-	-	- 181	652
HCM Lane V/C Ratio	0.08	-	-	- 0.041	0.1
HCM Control Delay (s)	9.3	-	-	- 25.7	11.1
HCM Lane LOS	А	-	-	- D	В
HCM 95th %tile Q(veh)	0.3	-	-	- 0.1	0.3

Int Delay, s/veh	0.2						
Movement	SBL	SBR	NEL	NET	SWT	SWR	
Lane Configurations	Y			÷.	et -		
Traffic Vol, veh/h	2	0	0	53	51	0	
Future Vol, veh/h	2	0	0	53	51	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage	,# 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	80	80	80	80	80	80	
Heavy Vehicles, %	2	2	2	15	7	2	
Mvmt Flow	3	0	0	66	64	0	

Major/Minor	Minor2	I	Major1	Ma	ajor2	
Conflicting Flow All	130	64	64	0	-	0
Stage 1	64	-	-	-	-	-
Stage 2	66	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	864	1000	1538	-	-	-
Stage 1	959	-	-	-	-	-
Stage 2	957	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	864	1000	1538	-	-	-
Mov Cap-2 Maneuver	864	-	-	-	-	-
Stage 1	959	-	-	-	-	-
Stage 2	957	-	-	-	-	-
	00				011/	

Approach	SB	NE	SW	
HCM Control Delay, s	9.2	0	0	
HCM LOS	А			

Minor Lane/Major Mvmt	NEL	NET SI	BLn1	SWT	SWR	
Capacity (veh/h)	1538	-	864	-	-	
HCM Lane V/C Ratio	-	- ().003	-	-	
HCM Control Delay (s)	0	-	9.2	-	-	
HCM Lane LOS	А	-	А	-	-	
HCM 95th %tile Q(veh)	0	-	0	-	-	

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	- † 1-		<u>ک</u>	∱î ≽			\$			\$	
Traffic Vol, veh/h	3	323	3	4	397	6	6	0	6	4	0	2
Future Vol, veh/h	3	323	3	4	397	6	6	0	6	4	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	350	-	-	225	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	85	85	80	90	90	80	80	80	80	80	80
Heavy Vehicles, %	2	17	2	2	7	2	2	2	2	2	2	2
Mvmt Flow	4	380	4	5	441	7	8	0	8	5	0	3

Major/Minor	Major1		Ν	Najor2			Vinor1		Ν	/linor2			
Conflicting Flow All	448	0	0	384	0	0	621	848	192	653	847	224	
Stage 1	-	-	-	-	-	-	390	390	-	455	455	-	
Stage 2	-	-	-	-	-	-	231	458	-	198	392	-	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32	
Pot Cap-1 Maneuver	1109	-	-	1171	-	-	372	297	817	352	297	779	
Stage 1	-	-	-	-	-	-	606	606	-	554	567	-	
Stage 2	-	-	-	-	-	-	751	565	-	785	605	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1109	-	-	1171	-	-	369	295	817	347	295	779	
Mov Cap-2 Maneuver	-	-	-	-	-	-	369	295	-	347	295	-	
Stage 1	-	-	-	-	-	-	604	604	-	552	565	-	
Stage 2	-	-	-	-	-	-	745	563	-	775	603	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0.1			0.1			12.3			13.6			
HCM LOS							В			В			
Minor Lane/Major Myn	nt	NRI n1	FRI	FRT	FRD	W/RI	W/RT		SRI n1				

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR WBL	WBT	WBR SBLn1	
Capacity (veh/h)	508	1109	-	- 1171	-	- 426	
HCM Lane V/C Ratio	0.03	0.003	-	- 0.004	-	- 0.018	
HCM Control Delay (s)	12.3	8.3	-	- 8.1	-	- 13.6	
HCM Lane LOS	В	А	-	- A	-	- B	
HCM 95th %tile Q(veh)	0.1	0	-	- 0	-	- 0.1	

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Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	ሻ	^	_ ≜ t}		ኘ	1
Traffic Vol, veh/h	49	281	350	2	1	44
Future Vol, veh/h	49	281	350	2	1	44
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	125	0
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	80	85	90	90	80	80
Heavy Vehicles, %	2	2	4	2	2	2
Mvmt Flow	61	331	389	2	1	55

Major/Minor	Major1	Maj	jor2	N	1inor2		
Conflicting Flow All	391	0	-	0	678	196	
Stage 1	-	-	-	-	390	-	
Stage 2	-	-	-	-	288	-	
Critical Hdwy	4.14	-	-	-	6.84	6.94	
Critical Hdwy Stg 1	-	-	-	-	5.84	-	
Critical Hdwy Stg 2	-	-	-	-	5.84	-	
Follow-up Hdwy	2.22	-	-	-	3.52	3.32	
Pot Cap-1 Maneuver	1164	-	-	-	386	812	
Stage 1	-	-	-	-	653	-	
Stage 2	-	-	-	-	735	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1164	-	-	-	366	812	
Mov Cap-2 Maneuver	-	-	-	-	366	-	
Stage 1	-	-	-	-	619	-	
Stage 2	-	-	-	-	735	-	
Approach	EB		WB		SW		
HCM Control Delay, s	1.3		0		9.9		

HCM LOS	А

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBRSWLn1	SWLn2
Capacity (veh/h)	1164	-	-	- 366	812
HCM Lane V/C Ratio	0.053	-	-	- 0.003	0.068
HCM Control Delay (s)	8.3	-	-	- 14.9	9.8
HCM Lane LOS	А	-	-	- B	А
HCM 95th %tile Q(veh)	0.2	-	-	- 0	0.2

0.1					
SBL	SBR	NEL	NET	SWT	SWR
Y			- द	el 👘	
1	0	0	59	45	1
1	0	0	59	45	1
0	0	0	0	0	0
Stop	Stop	Free	Free	Free	Free
-	None	-	None	-	None
0	-	-	-	-	-
,# 0	-	-	0	0	-
0	-	-	0	0	-
80	80	80	80	80	80
2	2	2	15	7	2
1	0	0	74	56	1
	0.1 SBL 1 1 0 Stop - 0 ,# 0 0 80 2 1	0.1 SBL SBR Y 1 0 1 0 0 0 Stop Stop Stop Stop 0 - None 0 - 80 - 80 - 80 - 80 - 1 0	0.1 SBL SBR NEL Y 1 0 0 1 0 0 1 0 0 0 0 0 Stop Stop Free None - None - 0 - 1 0 0 - 80 -	0.1 SBL SBR NEL NET ↓ 10 00 59 1 00 00 59 1 00 00 59 0 0 0 0 Stop 700 00 Stop 700 00 Stop 700 00 None 0 None 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.1 NEL NET SWT SBL SBR NEL NET SWT Y - - + + 1 0 0 59 45 1 0 0 59 45 0 0 0 59 45 0 0 0 0 0 Stop Free Free Free None - None - 0 - - 0 0 0 - - 0 0 0 - - 0 0 0 - - 0 0 0 - - 0 0 0 - - 0 0 0 0 80 80 80 80 80 2 2 2 15 7 1 0 0 74 56

Major/Minor	Minor2	I	Major1	Maj	or2		
Conflicting Flow All	131	57	57	0	-	0	
Stage 1	57	-	-	-	-	-	
Stage 2	74	-	-	-	-	-	
Critical Hdwy	6.42	6.22	4.12	-	-	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	2.218	-	-	-	
Pot Cap-1 Maneuver	863	1009	1547	-	-	-	
Stage 1	966	-	-	-	-	-	
Stage 2	949	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	863	1009	1547	-	-	-	
Mov Cap-2 Maneuver	863	-	-	-	-	-	
Stage 1	966	-	-	-	-	-	
Stage 2	949	-	-	-	-	-	

Approach	SB	NE	SW
HCM Control Delay, s	9.2	0	0
HCM LOS	А		

Minor Lane/Major Mvmt	NEL	NET SB	_n1	SWT	SWR			
Capacity (veh/h)	1547	- 8	363	-	-			
HCM Lane V/C Ratio	-	- 0.0	001	-	-			
HCM Control Delay (s)	0	-	9.2	-	-			
HCM Lane LOS	А	-	А	-	-			
HCM 95th %tile Q(veh)	0	-	0	-	-			

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	ħ ₽		1	∱î ≽			÷			÷	
Traffic Vol, veh/h	1	590	4	3	683	0	3	0	1	2	2	3
Future Vol, veh/h	1	590	4	3	683	0	3	0	1	2	2	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	350	-	-	225	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	85	85	80	90	90	80	80	80	80	80	80
Heavy Vehicles, %	2	17	2	2	7	2	2	2	2	2	2	2
Mvmt Flow	1	694	5	4	759	0	4	0	1	3	3	4

Major/Minor	Major1		N	lajor2		Ν	/linor1		N	/linor2			
Conflicting Flow All	759	0	0	699	0	0	1088	1466	350	1116	1468	380	
Stage 1	-	-	-	-	-	-	699	699	-	767	767	-	
Stage 2	-	-	-	-	-	-	389	767	-	349	701	-	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32	
Pot Cap-1 Maneuver	848	-	-	893	-	-	170	127	646	162	127	618	
Stage 1	-	-	-	-	-	-	397	440	-	361	410	-	
Stage 2	-	-	-	-	-	-	606	410	-	640	439	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	848	-	-	893	-	-	166	126	646	161	126	618	
Mov Cap-2 Maneuver	-	-	-	-	-	-	166	126	-	161	126	-	
Stage 1	-	-	-	-	-	-	397	440	-	361	408	-	
Stage 2	-	-	-	-	-	-	596	408	-	638	439	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0			0			23.1			22.8			
HCM LOS							С			С			

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	204	848	-	-	893	-	-	211
HCM Lane V/C Ratio	0.025	0.001	-	-	0.004	-	-	0.041
HCM Control Delay (s)	23.1	9.3	-	-	9	-	-	22.8
HCM Lane LOS	С	А	-	-	А	-	-	С
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1

02/06/2020

Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	ኘ	^	́A†≱		٦	1
Traffic Vol, veh/h	64	560	642	2	5	58
Future Vol, veh/h	64	560	642	2	5	58
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	125	0
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	80	85	90	90	80	80
Heavy Vehicles, %	2	2	4	2	2	2
Mvmt Flow	80	659	713	2	6	73

Major/Minor	Major1	Ma	ijor2	N	Ainor2		
Conflicting Flow All	715	0	-	0	1204	358	
Stage 1	-	-	-	-	714	-	
Stage 2	-	-	-	-	490	-	
Critical Hdwy	4.14	-	-	-	6.84	6.94	
Critical Hdwy Stg 1	-	-	-	-	5.84	-	
Critical Hdwy Stg 2	-	-	-	-	5.84	-	
Follow-up Hdwy	2.22	-	-	-	3.52	3.32	
Pot Cap-1 Maneuver	881	-	-	-	177	638	
Stage 1	-	-	-	-	446	-	
Stage 2	-	-	-	-	581	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	[.] 881	-	-	-	161	638	
Mov Cap-2 Maneuver	· -	-	-	-	161	-	
Stage 1	-	-	-	-	405	-	
Stage 2	-	-	-	-	581	-	
Approach	FB		WB		SW		
HCM Control Delay	<u> </u>		0		12.7		
HCM LOS			5		B		

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBRSWLn1	SWLn2
Capacity (veh/h)	881	-	-	- 161	638
HCM Lane V/C Ratio	0.091	-	-	- 0.039	0.114
HCM Control Delay (s)	9.5	-	-	- 28.3	11.4
HCM Lane LOS	А	-	-	- D	В
HCM 95th %tile Q(veh)	0.3	-	-	- 0.1	0.4

MovementSBLSBRNELNETSWTSWRLane ConfigurationsVIITraffic Vol, veh/h20059560Future Vol, veh/h20059560Future Vol, veh/h20059560Conflicting Peds, #/hr000000Sign ControlStopStopFreeFreeFreeRT Channelized-None-None-Storage Length0Veh in Median Storage, #000-Grade, %000-Peak Hour Factor808080808080	Int Delay, s/veh	0.2							
Lane Configurations \checkmark \checkmark \checkmark Traffic Vol, veh/h20059560Future Vol, veh/h20059560Conflicting Peds, #/hr000000Sign ControlStopStopFreeFreeFreeRT Channelized-None-NoneStorage Length0Veh in Median Storage, #000Grade, %000Peak Hour Factor8080808080	Movement	SBL	SBR	NEL	NET	SWT	SWR		
Traffic Vol, veh/h 2 0 0 59 56 0 Future Vol, veh/h 2 0 0 59 56 0 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Stop Free Free Free Free RT Channelized - None - None - None Storage Length 0 - - - - - Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 80 80 80 80 80 80	Lane Configurations	Y			÷.	et 👘			
Future Vol, veh/h 2 0 0 59 56 0 Conflicting Peds, #/hr 0	Traffic Vol, veh/h	2	0	0	59	56	0		
Conflicting Peds, #/hr000000Sign ControlStopStopFreeFreeFreeFreeRT Channelized-None-None-NoneStorage Length0Veh in Median Storage, #000-Grade, %000-Peak Hour Factor808080808080	Future Vol, veh/h	2	0	0	59	56	0		
Sign ControlStopStopFreeFreeFreeFreeRT Channelized-None-None-NoneStorage Length0Veh in Median Storage, #0-00-Grade, %0-00-Peak Hour Factor8080808080	Conflicting Peds, #/hr	0	0	0	0	0	0		
RT Channelized-None-NoneStorage Length0Veh in Median Storage, #0-00Grade, %000Peak Hour Factor8080808080	Sign Control	Stop	Stop	Free	Free	Free	Free		
Storage Length 0 -	RT Channelized	-	None	-	None	-	None		
Veh in Median Storage, # 0 - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 80 80 80 80 80 80	Storage Length	0	-	-	-	-	-		
Grade, % 0 - 0 0 - Peak Hour Factor 80 80 80 80 80	Veh in Median Storage,	# 0	-	-	0	0	-		
Peak Hour Factor 80 80 80 80 80 80	Grade, %	0	-	-	0	0	-		
	Peak Hour Factor	80	80	80	80	80	80		
Heavy Vehicles, % 2 2 2 15 7 2	Heavy Vehicles, %	2	2	2	15	7	2		
Mvmt Flow 3 0 0 74 70 0	Mvmt Flow	3	0	0	74	70	0		

		viajor i	IVID	IJULZ	
144	70	70	0	-	0
70	-	-	-	-	-
74	-	-	-	-	-
6.42	6.22	4.12	-	-	-
5.42	-	-	-	-	-
5.42	-	-	-	-	-
3.518	3.318	2.218	-	-	-
849	993	1531	-	-	-
953	-	-	-	-	-
949	-	-	-	-	-
			-	-	-
r 849	993	1531	-	-	-
r 849	-	-	-	-	-
953	-	-	-	-	-
949	-	-	-	-	-
	144 70 74 6.42 5.42 3.518 849 953 949 r 849 r 849 r 849 953 949	144 70 70 - 6.42 6.22 5.42 - 3.518 3.318 849 993 953 - 949 - r 849 993 r 849 - 953 - - 949 - - 953 - - 949 - - 953 - - 953 - - 953 - - 953 - - 953 - - 953 - - 953 - - 953 - - 949 - - 953 - - 949 - - 949 - - 949 - -	144 70 70 70 - - 74 - - 6.42 6.22 4.12 5.42 - - 5.42 - - 3.518 3.318 2.218 849 993 1531 953 - - 949 - - 953 - - 953 - - 953 - - 953 - - 953 - - 953 - - 953 - - 949 - - 949 - - 949 - - 949 - -	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$

Approach	SB	NE	SW
HCM Control Delay, s	9.3	0	0
HCM LOS	А		

Minor Lane/Major Mvmt	NEL	NETS	SBLn1	SWT	SWR
Capacity (veh/h)	1531	-	849	-	-
HCM Lane V/C Ratio	-	-	0.003	-	-
HCM Control Delay (s)	0	-	9.3	-	-
HCM Lane LOS	А	-	А	-	-
HCM 95th %tile Q(veh)	0	-	0	-	-

Intersection

Movement I	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	_ ≜ tp		٦	∱ î≽			\$			4	
Traffic Vol, veh/h	12	939	3	4	360	9	6	0	6	4	0	2
Future Vol, veh/h	12	939	3	4	360	9	6	0	6	4	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control F	ree	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	350	-	-	225	-	-	-	-	-	-	-	-
Veh in Median Storage, #		0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	90	90	80	90	90	80	80	80	80	80	80
Heavy Vehicles, %	2	17	2	2	7	2	2	2	2	2	2	2
Mvmt Flow	15	1043	3	5	400	10	8	0	8	5	0	3

Major/Minor	Major1		Μ	lajor2		N	Minor1		٨	/linor2			
Conflicting Flow All	410	0	0	1046	0	0	1285	1495	523	967	1491	205	
Stage 1	-	-	-	-	-	-	1075	1075	-	415	415	-	
Stage 2	-	-	-	-	-	-	210	420	-	552	1076	-	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32	
Pot Cap-1 Maneuver	1145	-	-	661	-	-	122	122	499	209	123	802	
Stage 1	-	-	-	-	-	-	234	294	-	585	591	-	
Stage 2	-	-	-	-	-	-	773	588	-	486	294	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1145	-	-	661	-	-	120	119	499	203	120	802	
Mov Cap-2 Maneuver	-	-	-	-	-	-	120	119	-	203	120	-	
Stage 1	-	-	-	-	-	-	231	290	-	577	586	-	
Stage 2	-	-	-	-	-	-	765	583	-	472	290	-	
Approach	ED			\//D			ND			CD			

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0.1	0.1	25.2	18.7	
HCM LOS			D	С	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	193	1145	-	-	661	-	-	270
HCM Lane V/C Ratio	0.078	0.013	-	-	0.008	-	-	0.028
HCM Control Delay (s)	25.2	8.2	-	-	10.5	-	-	18.7
HCM Lane LOS	D	А	-	-	В	-	-	С
HCM 95th %tile Q(veh)	0.2	0	-	-	0	-	-	0.1

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Intersection						
Int Delay, s/veh	34.4					
•						
Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	5	- 11	_ ≜î ≽		5	1
Traffic Vol, veh/h	655	291	320	108	31	40
Future Vol, veh/h	655	291	320	108	31	40
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None

•						
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	125	0
Veh in Median Storag	e, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	80	90
Heavy Vehicles, %	2	2	4	2	2	2
Mvmt Flow	728	323	356	120	39	44

Major/Minor	Major1	Ν	/lajor2	ľ	Minor2					
Conflicting Flow All	476	0	-	0	2034	238				
Stage 1	-	-	-	-	416	-				
Stage 2	-	-	-	-	1618	-				
Critical Hdwy	4.14	-	-	-	6.84	6.94				
Critical Hdwy Stg 1	-	-	-	-	5.84	-				
Critical Hdwy Stg 2	-	-	-	-	5.84	-				
Follow-up Hdwy	2.22	-	-	-	3.52	3.32				
Pot Cap-1 Maneuver	1082	-	-	-	49	763				
Stage 1	-	-	-	-	634	-				
Stage 2	-	-	-	-	148	-				
Platoon blocked, %		-	-	-						
Mov Cap-1 Maneuver	1082	-	-	-	~ 16	763				
Mov Cap-2 Maneuver	-	-	-	-	~ 16	-				
Stage 1	-	-	-	-	207	-				
Stage 2	-	-	-	-	148	-				
Approach	EB		WB		SW					
HCM Control Delay, s	10.3		0	\$	536.1					
HCM LOS					F					
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBRS	WLn1S	SWLn2			
Capacity (veh/h)		1082	-	-	-	16	763			
HCM Lane V/C Ratio		0.673	-	-	-	2.422	0.058			
HCM Control Delay (s)	14.9	-	-	\$ 1	139.5	10			
HCM Lane LOS	/	В	-	-	-	F	В			
HCM 95th %tile Q(veh	ı)	5.5	-	-	-	5.5	0.2			
Notes										
~: Volume exceeds ca	pacity	\$: De	lay exc	eeds 30)0s +	: Com	outation No	t Defined	*: All major volume in platoon	
Minor Lane/Major Mvr Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s HCM Lane LOS HCM 95th %tile Q(veh Notes ~: Volume exceeds ca	nt) I) Ipacity	EBL 1082 0.673 14.9 B 5.5 \$: Del	EBT - - - - -	WBT - - - - - -	WBRS - - - - - - -	WLn15 16 2.422 1139.5 F 5.5 +: Comp	WLn2 763 0.058 10 B 0.2 Dutation No	t Defined	*: All major volume in platoon	

8.6						
SBL	SBR	NEL	NET	SWT	SWR	
Y			÷	et P		
1	30	717	53	41	1	
1	30	717	53	41	1	
0	0	0	0	0	0	
Stop	Stop	Free	Free	Free	Free	
-	None	-	None	-	None	
0	-	-	-	-	-	
,# 0	-	-	0	0	-	
0	-	-	0	0	-	
90	90	90	90	85	85	
2	2	2	15	7	2	
1	33	797	59	48	1	
	8.6 SBL 1 1 0 Stop - 0 ,# 0 0 90 2 1	8.6 SBL SBR ↓ 1 30 1 30 0 0 Stop Stop Stop Stop - None 0 ,# 0 90 90 2 2 1 33	8.6 SBL SBR NEL Y 1 30 717 1 30 717 0 0 0 Stop Stop Free - None - 0 - 0 - 0 - 90 90 90 90 90 2 2 2 1 33 797	8.6 SBL SBR NEL NET Y	8.6 SBL SBR NEL NET SWT Y - 4 1 1 30 717 53 41 1 30 717 53 41 0 0 0 0 0 Stop Stop Free Free Free None - None - 0 - - 0 0 0 - - 0 0 90 - - 0 0 90 90 90 90 85 2 2 2 15 7 1 33 797 59 48	8.6 SBL SBR NEL NET SWT SWR Y

Major/Minor	Minor2		Major1	Ma	ajor2		
Conflicting Flow All	1702	49	49	0	-	0	
Stage 1	49	-	-	-	-	-	
Stage 2	1653	-	-	-	-	-	
Critical Hdwy	6.42	6.22	4.12	-	-	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	2.218	-	-	-	
Pot Cap-1 Maneuver	101	1020	1558	-	-	-	
Stage 1	973	-	-	-	-	-	
Stage 2	171	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	48	1020	1558	-	-	-	
Mov Cap-2 Maneuver	48	-	-	-	-	-	
Stage 1	458	-	-	-	-	-	
Stage 2	171	-	-	-	-	-	
•					0.47		

Approach	SB	NE	SW	
HCM Control Delay, s	11.2	9	0	
HCMLOS	В			

Minor Lane/Major Mvmt	NEL	NET SBLn1	SWT	SWR
Capacity (veh/h)	1558	- 617	-	-
HCM Lane V/C Ratio	0.511	- 0.056	-	-
HCM Control Delay (s)	9.7	0 11.2	-	-
HCM Lane LOS	А	A B	-	-
HCM 95th %tile Q(veh)	3	- 0.2	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	∱ î≽		۲	∱ î≽			\$			4	
Traffic Vol, veh/h	1	534	4	3	1266	0	3	0	1	5	2	11
Future Vol, veh/h	1	534	4	3	1266	0	3	0	1	5	2	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	350	-	-	225	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	90	90	80	90	90	80	80	80	80	80	80
Heavy Vehicles, %	2	17	2	2	7	2	2	2	2	2	2	2
Mvmt Flow	1	593	4	4	1407	0	4	0	1	6	3	14

Major/Minor	Major1		N	lajor2		N	/linor1		Ν	/linor2			
Conflicting Flow All	1407	0	0	597	0	0	1310	2012	299	1714	2014	704	
Stage 1	-	-	-	-	-	-	597	597	-	1415	1415	-	
Stage 2	-	-	-	-	-	-	713	1415	-	299	599	-	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32	
Pot Cap-1 Maneuver	481	-	-	976	-	-	117	58	697	58	58	379	
Stage 1	-	-	-	-	-	-	456	490	-	144	202	-	
Stage 2	-	-	-	-	-	-	389	202	-	685	489	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	481	-	-	976	-	-	109	58	697	58	58	379	
Mov Cap-2 Maneuver	· -	-	-	-	-	-	109	58	-	58	58	-	
Stage 1	-	-	-	-	-	-	455	489	-	144	201	-	
Stage 2	-	-	-	-	-	-	369	201	-	682	488	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0	0	32.1	41.8	
HCM LOS			D	E	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR \$	SBLn1
Capacity (veh/h)	138	481	-	-	976	-	-	120
HCM Lane V/C Ratio	0.036	0.003	-	-	0.004	-	-	0.188
HCM Control Delay (s)	32.1	12.5	-	-	8.7	-	-	41.8
HCM Lane LOS	D	В	-	-	А	-	-	Е
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.7

41.4					
EBL	EBT	WBT	WBR	SWL	SWR
- ሽ	- 11	_ ≜ î≽		<u>۲</u>	1
58	510	618	32	112	663
58	510	618	32	112	663
0	0	0	0	0	0
Free	Free	Free	Free	Stop	Stop
-	None	-	None	-	None
150	-	-	-	125	0
, # -	0	0	-	0	-
-	0	0	-	0	-
90	90	90	90	80	90
2	2	4	2	2	2
64	567	687	36	140	737
	41.4 EBL 58 58 0 Free - 150 ,# - 90 2 64	41.4 EBL EBT 58 510 58 510 0 0 Free Free 150 - # - 0 4 - 0 90 90 2 2 64 567	41.4 EBL EBT WBT ★★ ★★ ★★ 58 510 618 58 510 618 58 510 618 0 0 0 Free Free Free 150 - - 150 - 0 0 0 0 90 90 90 92 2 4 64 567 687	41.4 WBT WBR EBL EBT WBT WBR ↑ ↑↑ ↑↓ 58 510 618 32 58 510 618 32 58 510 618 32 0 0 0 0 Free Free Free Free 150 - None - 150 - 0 0 - # 0 0 - 90 90 90 90 90 2 2 4 2 64 567 687 368	41.4 EBL EBT WBT WBR SWL ↑ ↑↑ ↑↑ ↑ ↑ 58 510 618 32 112 58 510 618 32 112 58 510 618 32 112 0 0 0 0 0 Free Free Free Free Stop 150 - - 125 # 0 0 - 0 90 90 90 90 80 22 2 4 2 2 64 567 687 36 140

Major/Minor	Major1	Ν	lajor2	Ν	/linor2				
Conflicting Flow All	723	0	-	0	1117	362			
Stage 1	-	-	-	-	705	-			
Stage 2	-	-	-	-	412	-			
Critical Hdwy	4.14	-	-	-	6.84	6.94			
Critical Hdwy Stg 1	-	-	-	-	5.84	-			
Critical Hdwy Stg 2	-	-	-	-	5.84	-			
Follow-up Hdwy	2.22	-	-	-	3.52	3.32			
Pot Cap-1 Maneuver	875	-	-	-	201	~ 635			
Stage 1	-	-	-	-	451	-			
Stage 2	-	-	-	-	637	-			
Platoon blocked, %		-	-	-					
Mov Cap-1 Maneuver	875	-	-	-	186	~ 635			
Mov Cap-2 Maneuver	-	-	-	-	186	-			
Stage 1	-	-	-	-	418	-			
Stage 2	-	-	-	-	637	-			
Approach	EB		WB		SW				
HCM Control Delay, s	1		0		104.7				
HCM LOS					F				
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBRS	WLn1S	SWLn2		
Capacity (veh/h)		875	-	-	-	186	635		
HCM Lane V/C Ratio		0.074	-	-	-	0.753	1.16		
HCM Control Delay (s)	9.4	-	-	-	67	111.9		
HCM Lane LOS	,	А	-	-	-	F	F		
HCM 95th %tile Q(veh	ו)	0.2	-	-	-	4.9	24.1		
Notes									
~: Volume exceeds ca	pacity	\$: Del	ay exc	eeds 30)0s -	+: Com	outation Not Defined	*: All major volume in platoon	

Int Delay, s/veh	17.8					
Movement	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations	Y			<u>କ</u> ୍	el 👘	
Traffic Vol, veh/h	2	717	30	53	51	0
Future Vol, veh/h	2	717	30	53	51	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	85	85
Heavy Vehicles, %	2	2	2	15	7	2
Mvmt Flow	2	797	33	59	60	0

Major/Minor	Minor2		Major1	Maj	or2			
Conflicting Flow All	185	60	60	0	-	0		
Stage 1	60	-	-	-	-	-		
Stage 2	125	-	-	-	-	-		
Critical Hdwy	6.42	6.22	4.12	-	-	-		
Critical Hdwy Stg 1	5.42	-	-	-	-	-		
Critical Hdwy Stg 2	5.42	-	-	-	-	-		
Follow-up Hdwy	3.518	3.318	2.218	-	-	-		
Pot Cap-1 Maneuver	804	1005	1544	-	-	-		
Stage 1	963	-	-	-	-	-		
Stage 2	901	-	-	-	-	-		
Platoon blocked, %				-	-	-		
Mov Cap-1 Maneuver	786	1005	1544	-	-	-		
Mov Cap-2 Maneuver	786	-	-	-	-	-		
Stage 1	942	-	-	-	-	-		
Stage 2	901	-	-	-	-	-		

Approach	SB	NE	SW
HCM Control Delay, s	20.9	2.7	0
HCM LOS	С		

Minor Lane/Major Mvmt	NEL	NET SBLn1	SWT	SWR
Capacity (veh/h)	1544	- 1004	-	-
HCM Lane V/C Ratio	0.022	- 0.796	-	-
HCM Control Delay (s)	7.4	0 20.9	-	-
HCM Lane LOS	А	A C	-	-
HCM 95th %tile Q(veh)	0.1	- 8.7	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	∱ î,		1	∱ î≽			\$			\$	
Traffic Vol, veh/h	11	626	3	4	397	9	6	0	6	4	0	2
Future Vol, veh/h	11	626	3	4	397	9	6	0	6	4	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	350	-	-	225	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	90	90	80	90	90	80	80	80	80	80	80
Heavy Vehicles, %	2	17	2	2	7	2	2	2	2	2	2	2
Mvmt Flow	14	696	3	5	441	10	8	0	8	5	0	3

Major/Minor	Major1		Ν	lajor2		Ν	/linor1		Ν	/linor2			
Conflicting Flow All	451	0	0	699	0	0	957	1187	350	832	1183	226	
Stage 1	-	-	-	-	-	-	726	726	-	456	456	-	
Stage 2	-	-	-	-	-	-	231	461	-	376	727	-	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32	
Pot Cap-1 Maneuver	1106	-	-	893	-	-	212	187	646	262	188	777	
Stage 1	-	-	-	-	-	-	382	428	-	554	567	-	
Stage 2	-	-	-	-	-	-	751	564	-	617	427	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	1106	-	-	893	-	-	208	183	646	255	184	777	
Mov Cap-2 Maneuver	-	-	-	-	-	-	208	183	-	255	184	-	
Stage 1	-	-	-	-	-	-	377	422	-	547	564	-	
Stage 2	-	-	-	-	-	-	744	561	-	602	421	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	0.2			0.1			17			16.2			

HCM LOS						С		C	
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR SBLn1		
Capacity (veh/h)	315	1106	-	-	893	-	- 329		
HCM Lane V/C Ratio	0.048	0.012	-	-	0.006	-	- 0.023		

HCM Control Delay (s)	17	8.3	-	-	9.1	-	-	16.2
HCM Lane LOS	С	А	-	-	А	-	-	С
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1

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Intersection						
Int Delay, s/veh	2.1					
M	EDI	EDT			014/	
Movement	EBL	ERI	WBI	WBR	SWL	SWR
Lane Configurations	- ሽ	- 11	_ ≜ β		- ሽ	1
Traffic Vol, veh/h	182	451	353	25	1	44
Future Vol, veh/h	182	451	353	25	1	44
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	125	0
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	90	90	90	80	85
Heavy Vehicles, %	2	2	4	2	2	2
Mvmt Flow	214	501	392	28	1	52

Major/Minor	Major1	Ma	ajor2	ľ	/linor2	
Conflicting Flow All	420	0	-	0	1085	210
Stage 1	-	-	-	-	406	-
Stage 2	-	-	-	-	679	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1136	-	-	-	211	796
Stage 1	-	-	-	-	641	-
Stage 2	-	-	-	-	465	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1136	-	-	-	171	796
Mov Cap-2 Maneuver	-	-	-	-	171	-
Stage 1	-	-	-	-	520	-
Stage 2	-	-	-	-	465	-
Approach	EB		WB		SW	
HCM Control Delay, s	2.7		0		10.2	
HCM LOS					В	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBRSWLn1	SWLn2	
Capacity (veh/h)	1136	-	-	- 171	796	
HCM Lane V/C Ratio	0.188	-	-	- 0.007	0.065	
HCM Control Delay (s)	8.9	-	-	- 26.2	9.8	
HCM Lane LOS	А	-	-	- D	Α	
HCM 95th %tile Q(veh)	0.7	-	-	- 0	0.2	

Int Delay, s/veh	4.6							
Movement	SBL	SBR	NEL	NET	SWT	SWR		
Lane Configurations	Y			÷.	et 👘			
Traffic Vol, veh/h	1	0	156	59	45	1		
Future Vol, veh/h	1	0	156	59	45	1		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	-	-	-	-	-		
Veh in Median Storage,	# 0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	85	85	85	85	85	85		
Heavy Vehicles, %	2	2	2	15	7	2		
Mvmt Flow	1	0	184	69	53	1		

Major/Minor	Minor2		Major1	Ma	ijor2	
Conflicting Flow All	491	54	54	0	-	0
Stage 1	54	-	-	-	-	-
Stage 2	437	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	537	1013	1551	-	-	-
Stage 1	969	-	-	-	-	-
Stage 2	651	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	471	1013	1551	-	-	-
Mov Cap-2 Maneuver	471	-	-	-	-	-
Stage 1	850	-	-	-	-	-
Stage 2	651	-	-	-	-	-

Approach	SB	NE	SW
HCM Control Delay, s	12.7	5.5	0
HCM LOS	В		

Minor Lane/Major Mvmt	NEL	NET S	BLn1	SWT	SWR
Capacity (veh/h)	1551	-	471	-	-
HCM Lane V/C Ratio	0.118	-	0.002	-	-
HCM Control Delay (s)	7.6	0	12.7	-	-
HCM Lane LOS	А	А	В	-	-
HCM 95th %tile Q(veh)	0.4	-	0	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ľ	↑ ĵ₽		1	∱î ≽			¢			¢	
Traffic Vol, veh/h	1	590	4	3	986	0	3	0	1	5	2	11
Future Vol, veh/h	1	590	4	3	986	0	3	0	1	5	2	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	350	-	-	225	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	90	90	80	90	90	80	80	80	80	80	80
Heavy Vehicles, %	2	17	2	2	7	2	2	2	2	2	2	2
Mvmt Flow	1	656	4	4	1096	0	4	0	1	6	3	14

Major/Minor	Major1		Ν	lajor2		ľ	Minor1		ľ	/linor2			
Conflicting Flow All	1096	0	0	660	0	0	1218	1764	330	1434	1766	548	
Stage 1	-	-	-	-	-	-	660	660	-	1104	1104	-	
Stage 2	-	-	-	-	-	-	558	1104	-	330	662	-	
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-	
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32	
Pot Cap-1 Maneuver	633	-	-	924	-	-	136	83	666	94	83	480	
Stage 1	-	-	-	-	-	-	418	458	-	225	285	-	
Stage 2	-	-	-	-	-	-	482	285	-	657	457	-	
Platoon blocked, %		-	-		-	-							
Mov Cap-1 Maneuver	633	-	-	924	-	-	129	83	666	93	83	480	
Mov Cap-2 Maneuver	· _	-	-	-	-	-	129	83	-	93	83	-	
Stage 1	-	-	-	-	-	-	417	457	-	225	284	-	
Stage 2	-	-	-	-	-	-	462	284	-	655	456	-	
A 1										0.5			

Approach	EB	WB	NB	SB	
HCM Control Delay, s	0	0	27.9	28	
HCM LOS			D	D	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1
Capacity (veh/h)	162	633	-	-	924	-	-	179
HCM Lane V/C Ratio	0.031	0.002	-	-	0.004	-	-	0.126
HCM Control Delay (s)	27.9	10.7	-	-	8.9	-	-	28
HCM Lane LOS	D	В	-	-	А	-	-	D
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.4

Intersection						
Int Delay, s/veh	3.2					
Movement	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	۲.	- 11	∱ î,		۲.	1
Traffic Vol, veh/h	64	563	812	2	29	191
Future Vol, veh/h	64	563	812	2	29	191
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	125	0
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	85	90	90	90	80	85
Heavy Vehicles, %	2	2	4	2	2	2
Mymt Flow	75	626	902	2	36	225

Major/Minor	Major1	Ma	jor2	Ν	/linor2	
Conflicting Flow All	904	0	-	0	1366	452
Stage 1	-	-	-	-	903	-
Stage 2	-	-	-	-	463	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	748	-	-	-	138	555
Stage 1	-	-	-	-	356	-
Stage 2	-	-	-	-	600	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	748	-	-	-	124	555
Mov Cap-2 Maneuver	· _	-	-	-	124	-
Stage 1	-	-	-	-	320	-
Stage 2	-	-	-	-	600	-
A I					0147	

Approach	EB	WB	SW	
HCM Control Delay, s	1.1	0	19.9	
HCM LOS			С	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBRSWLn1	SWLn2	
Capacity (veh/h)	748	-	-	- 124	555	
HCM Lane V/C Ratio	0.101	-	-	- 0.292	0.405	
HCM Control Delay (s)	10.4	-	-	- 45.6	15.8	
HCM Lane LOS	В	-	-	- E	С	
HCM 95th %tile Q(veh)	0.3	-	-	- 1.1	1.9	

5.4					
SBL	SBR	NEL	NET	SWT	SWR
۰¥			- 4	el 👘	
2	156	0	59	56	0
2	156	0	59	56	0
0	0	0	0	0	0
Stop	Stop	Free	Free	Free	Free
-	None	-	None	-	None
0	-	-	-	-	-
,# 0	-	-	0	0	-
0	-	-	0	0	-
85	85	85	85	85	85
2	2	2	15	7	2
2	184	0	69	66	0
	5.4 SBL 2 2 0 Stop - 0 ,# 0 0 85 2 2 2	5.4 SBL SBR 2 156 2 156 2 156 0 0 Stop Stop Stop Stop 4 0 - 5 4 0 - 85 85 2 2 184	5.4 SBR NEL SBL SBR 0 2 156 0 2 156 0 2 156 0 2 156 0 2 156 0 0 0 0 Stop Stop Free None - - 0 - - 0 - - 0 - - 85 85 85 2 2 2 2 184 0	5.4 SBR NEL NET № 156 0 59 2 156 0 59 2 156 0 59 2 156 0 59 0 0 0 0 Stop Free Free Free None - None 0 0 - - 0 0 - - 0 85 85 85 85 2 2 2 15 2 184 0 69	5.4 NEL NET SWT Y - ↓ <t< td=""></t<>

Major/Minor	Minor2	I	Major1	Ma	ajor2	
Conflicting Flow All	135	66	66	0	-	0
Stage 1	66	-	-	-	-	-
Stage 2	69	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	859	998	1536	-	-	-
Stage 1	957	-	-	-	-	-
Stage 2	954	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	859	998	1536	-	-	-
Mov Cap-2 Maneuver	859	-	-	-	-	-
Stage 1	957	-	-	-	-	-
Stage 2	954	-	-	-	-	-
Approach	CD				C/M/	

Approach	SB	NE	SW	
HCM Control Delay, s	9.4	0	0	
HCMLOS	А			

Minor Lane/Major Mvmt	NEL	NET SBLn1	SWT	SWR
Capacity (veh/h)	1536	- 996	-	-
HCM Lane V/C Ratio	-	- 0.187	-	-
HCM Control Delay (s)	0	- 9.4	-	-
HCM Lane LOS	А	- A	-	-
HCM 95th %tile Q(veh)	0	- 0.7	-	-



TRAFFIC IMPACT ANALYIS - ADDENDUM #1 RESOLUTION COPPER MINE PROJECT SUPERIOR, ARIZONA

APPENDIX

Turn Lane Calculations

04/09/2020

Intersection Int Delay, s/veh 8.6 Movement SBL SBR NEL NET SWT SWR Y Lane Configurations ÷. 14 Traffic Vol, veh/h 53 30 717 41 1 30 717 53 Future Vol, veh/h 1 41 1 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Stop Free Free Free Free RT Channelized - None - None - None 0 - - - - -Storage Length Veh in Median Storage, # 0 - - 0 0 -Grade, % 0 --0 0 -90 90 90 Peak Hour Factor 90 85 85 Heavy Vehicles, % 22 15 2 7 2 33 797 59 Mvmt Flow 1 48 1 Major/Minor Major2 Minor2 Major1 Conflicting Flow All 1702 49 49 0 -0 Stage 1 49 -----Stage 2 1653 - - -- -Critical Hdwy 6.42 6.22 4.12 ---Critical Hdwy Stg 1 5.42 - ----Critical Hdwy Stg 2 5.42 - ---_ 3.518 3.318 2.218 Follow-up Hdwy -- -Pot Cap-1 Maneuver 101 1020 1558 ---Stage 1 973 - - - - -Stage 2 171 - - -- -Platoon blocked, % ---48 1020 1558 - - -Mov Cap-1 Maneuver Mov Cap-2 Maneuver 48 . . . - -458 Stage 1 - - -- -Stage 2 171 SW Approach SB NE HCM Control Delay, s 11.2 0 9 HCM LOS В Minor Lane/Major Mvmt NEL NET SBLn1 SWT SWR Capacity (veh/h) 1558 - 617 --HCM Lane V/C Ratio 0.511 - 0.056 - -HCM Control Delay (s) 0 11.2 9.7 - -HCM Lane LOS A А В - -3

HCM 6th TWSC 30: Main Street & Lonetree Road

Intersection						
Int Delay, s/veh	17.8					
Movement	SBL	SBR	NEL	NET	SWT	SWR
Lane Configurations	۰¥			÷	et 👘	
Traffic Vol, veh/h	2	717	30	53	51	0
Future Vol, veh/h	2	717	30	53	51	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	85	85
Heavy Vehicles, %	2	2	2	15	7	2
Mvmt Flow	2	797	33	59	60	0
Major/Minor I	Minor2		Major1	1	Major2	
Conflicting Flow All	185	60	60	0	-	0
Stage 1	60	-	-	-	-	-
Stage 2	125	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	804	1005	1544	-	-	-
Stage 1	963	-	-	-	-	-
Stage 2	901	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	786	1005	1544	-	-	-
Mov Cap-2 Maneuver	786	-	-	-	-	-
Stage 1	942	-	-	-	-	-
Stage 2	901	-	-	-	-	-
Approach	SB		NE		SW	
HCM Control Delay, s	20.9		2.7		0	
HCM LOS	С					
Minor Lane/Major Mvm	t	NEL	NET	SBLn1	SWT	SWR
Capacity (veh/h)		1544	-	1004	-	-
HCM Lane V/C Ratio		0.022	-	0.796	-	-
HCM Control Delay (s)		7.4	0	20.9	-	-
HCM Lane LOS		A	A	С	-	-
HCM 95th %tile Q(veh)		0.1) -	8.7	-	-

- -More vehicles queue in AM.

0.2

2022 With - AM Peak Hour

HCM 95th %tile Q(veh)

Synchro 10 Report Page 3 2022 With - PM Peak Hour

2022 With Project (Peak Construction)

Y

1

1

0

0

85

2

1

Minor2

491

SBL SBR NEL NET SWT SWR

Stop Stop Free Free Free Free

-

-

-

85

2

0 184

Major1

54

0 156

0 156

- None - None

-

-

-

85

2

54

0 0 0 0

4

59

59

-

0

0

85

69

0

15

Þ

45

45

0

-

0

0

85

53

Major2

7

-

- None

1

0

-

-

-

85

2

1

0

Intersection Int Delay, s/veh

Movement

Lane Configurations

Conflicting Peds, #/hr

Veh in Median Storage, # 0

Traffic Vol, veh/h

Future Vol, veh/h

Sign Control

RT Channelized

Storage Length

Peak Hour Factor

Heavy Vehicles, %

Conflicting Flow All

Grade, %

Mvmt Flow

Major/Minor

04/09/2020

HCM 6th TWSC 30: Main Street & Lonetree Road

	5.4					
Int Delay, s/ven	5.4					
Movement	SBL	SBR	NEL	NET	SWT	SW
Lane Configurations	- Y			- କୀ	- îs	
Traffic Vol, veh/h	2	156	0	59	56	
Future Vol, veh/h	2	156	0	59	56	
Conflicting Peds, #/hr	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Fre
RT Channelized	-	None	-	None	-	Nor
Storage Length	0	-	-	-	-	
Veh in Median Storage	,# 0	-	-	0	0	
Grade, %	0	-	-	0	0	
Peak Hour Factor	85	85	85	85	85	8
Heavy Vehicles, %	2	2	2	15	7	
Mvmt Flow	2	184	0	69	66	
Major/Minor I	Minor2		Major1		Major2	
Conflicting Flow All	135	66	66	0	-	
Stage 1	66	-	-	-	-	
Stage 2	69	-	-	-	-	
Critical Hdwy	6.42	6.22	4.12	-	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	
Follow-up Hdwy	3.518	3.318	2.218	-	-	
Pot Cap-1 Maneuver	859	998	1536	-	-	
Stage 1	957	-	-	-	-	
Stage 2	954	-	-	-	-	
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	859	998	1536	-	-	
Mov Cap-2 Maneuver	859	-	-	-	-	
Stage 1	957	-	-	-	-	
Stage 2	954	-	-	-	-	
Approach	SB		NF		SW	
HCM Control Delay	94		0		0	
HCM LOS	A		0		0	
Minor Long/Maier M	1			001-4	C) A/T	OW
Ninor Lane/Major MVm	I	INEL 4520	NEL	SBLNT	5001	511
Capacity (ven/n)		1536	-	996	-	
HUM Cantral Datas (1)		-	-	0.18/	-	
HOW Long LOC		0		9.4	-	
nuvu ane LUS		A	<u> </u>	А	-	

Stage 1	54	-	-	-	-	-			
Stage 2	437	-	-	-	-	-			
Critical Hdwy	6.42	6.22	4.12	-	-	-			
Critical Hdwy Stg 1	5.42	-	-	-	-	-			
Critical Hdwy Stg 2	5.42	-	-	-	-	-			
Follow-up Hdwy	3.518	3.318	2.218	-	-	-			
Pot Cap-1 Maneuver	537	1013	1551	-	-	-			
Stage 1	969	-	-	-	-	-			
Stage 2	651	-	-	-	-	-			
Platoon blocked, %				-	-	-			
Mov Cap-1 Maneuver	471	1013	1551	-	-	-			
Mov Cap-2 Maneuver	471	-	-	-	-	-			
Stage 1	850	-	-	-	-	-			
Stage 2	651	-	-	-	-	-			
Annroach	SB		NE		SW				
HCM Control Dolay	12.7		5.5		011				
HCM LOS	12.1 R		5.5		0				
	D								
Minor Lane/Major Mvm	t	NEL	NETS	SBLn1	SWT	SWR			
Capacity (veh/h)		1551	-	471	-	-			
HCM Lane V/C Ratio		0.118	-	0.002	-	-			
HCM Control Delay (s)		7.6	0	12.7	-	-			
HCM Lane LOS		A	A	В	-	-			
HCM 95th %tile Q(veh)		0.4) -	0	-	-			
		$\overline{}$	<u></u> М	ore ve	ehicle	es que	eue in AM.		

2027 With - AM Peak Hour

Synchro 10 Report Page 3 2027 With - PM Peak Hour

2027 With Project (Normal Operations)



TRAFFIC IMPACT ANALYIS - ADDENDUM #1 RESOLUTION COPPER MINE PROJECT SUPERIOR, ARIZONA

APPENDIX

Traffic Signal Warrant Analysis



Enter Traffic Volumes:

Automated Traffic Counts

Street: US 60

Location: Main Street City/State: Superior, AZ Project #: 20010 Date: 11/18/2016 Day of Week: Friday

Data Source: 24-hour approach



- Westbound

- Total Vehicles

24-Hour Volume: 12,093

Time	Eastbound		Westbound		The	Eastbour	Eastbound		Westbound	
	Vehicles	Peds	Vehicles	Peds	1 ime	Vehicles	Peds	Vehicles	Peds	
12:00 AM					12:00 PM					
12:15 AM					12:15 PM					
12:30 AM					12:30 PM					
12:45 AM					12:45 PM					
1:00 AM	26		22		1:00 PM	327		372		
1:15 AM					1:15 PM					
1:30 AM					1:30 PM					
1:45 AM					1:45 PM					
2:00 AM	34		14		2:00 PM	347		406		
2:15 AM					2:15 PM					
2:30 AM					2:30 PM					
2:45 AM					2:45 PM					
3:00 AM	23		17		3:00 PM	404		488		
3:15 AM					3:15 PM					
3:30 AM					3:30 PM					
3:45 AM					3:45 PM					
4:00 AM	42		34		4:00 PM	438		523		
4:15 AM					4:15 PM					
4:30 AM					4:30 PM					
4:45 AM					4:45 PM					
5:00 AM	134		69		5:00 PM	468		524		
5:15 AM					5:15 PM	100		021		
5:30 AM					5:30 PM					
5:45 AM					5:45 PM					
6:00 AM	288		120		6:00 PM	448		461		
6:15 AM	200		120		6:15 PM	110		101		
6:30 AM					6:30 PM					
6:45 AM					6:45 PM					
7:00 AM	317		187		7:00 PM	437		403		
7:15 AM	517		107		7:15 PM	137		100		
7:30 AM					7:30 PM					
7:45 AM					7:45 PM					
8:00 AM	340		229		8:00 PM	382		203		
8:15 AM	510		220		8:15 PM	002		200		
8:30 AM					8:30 PM					
8:45 AM					8:45 PM					
9:00 AM	310		270		9:00 PM	242		150		
9:15 AM	510		210		9:15 PM					
9:30 AM					9:30 PM					
9:45 AM					9:45 PM					
10:00 AM	301		325		10:00 PM	195		83		
10:15 AM	501		020		10:15 PM					
10:30 AM					10:30 PM					
10:45 AM					10:45 PM					
11:00 AM	353		324		11:00 PM	149		50		
11:15 AM			J2T		11:15 PM					
11:30 AM					11.30 PM					
11:45 AM					11:45 PM					
12.00 PM	353		332		12.00 AM	102		26		
12.00 1 WI	555		552	L]	12.00 AlVI	6.460	I	5.622		
Equipment ID#:				1		0,400	Hour Volume	12.093		
Equipment for. 24-nour volume 12,093										

------ Eastbound
	arric Counts		<u> </u>	<u> </u>					
Street: 1 Location:	Main Street US 60		. Hour	80 70 60					
City/State:	Superior, AZ		icles per	50 40 30					
Project #:	11/10/2016		Veh	10	10-10-1				-8
Dav of Week:	Fridav			100 300	500 700 900	**************************************	0 1700	\$ \$ \$ \$ \$ 1900 2100 2300	
Data Source: 2	24-hour approacl	1				Time of Day			
4-Hour Volume:	748				- Northbound	-B- Southbound	—st-— Tota	al Vehicles	
Time	Northbour	ıd	Southbo	und	Time	Northbou	nd	Southbou	ınd
10.00.134	Vehicles	Peds	Vehicles	Peds		Vehicles	Peds	Vehicles	Ped
12:00 AM					12:00 PM				
12:15 AM					12:59 PM				
12:30 AM					12:30 PM				
12:45 AM			2		12:45 PM			64	
1:00 AM			э	┼───┤	1:00 PM	+		01	<u> </u>
1.15 AW					1.39 PM 1.30 DM				
1.30 ANI 1.45 AM					1.50 PM				
2.00 AM			4		2.00 PM			64	
2:15 AM			7	<u> </u>	2.50 PM			04	
2:30 AM					2:30 PM				
2:45 AM					2:45 PM				
3:00 AM			1		3:00 PM			53	
3:15 AM					3:59 PM				
3:30 AM					3:30 PM				
3:45 AM					3:45 PM				
4:00 AM			2		4:00 PM			67	
4:15 AM					4:59 PM				
4:30 AM					4:30 PM				
4:45 AM					12:00 AM				
5:00 AM			11		5:00 PM			63	
5:15 AM					5:59 PM				
5:30 AM					5:30 PM				
5:45 AM					5:45 PM				
6:00 AM			13		6:00 PM			55	
6:15 AM					6:59 PM				
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM			16	───┤	7:00 PM			29	
7:15 AM					7:59 PM				
7:30 AM					7:30 PM				
/:45 AM			40		/:45 PM			04	
8:00 AM			40	<u>├</u>	8:00 PM			24	<u> </u>
8:15 AM					8:39 PM				
8.45 AM					8-45 DM				
9.00 AM			37		0.40 PM			14	
9:15 AM			51	<u> </u>	9.50 PM	1		14	
9:30 AM					9:30 PM				
9:45 AM					9:45 PM				
10:00 AM			43		10:00 PM			17	
10:15 AM			-		10:59 PM				
10:30 AM					10:30 PM				
10:45 AM					10:45 PM				
11:00 AM			51		11:00 PM			14	
11:15 AM					11:59 PM				1
11:30 AM					11:30 PM				
11:45 AM					11:45 PM				
12:00 PM			50		12:00 AM			15	L
									_

TRAFFIC SURVEY - COUNT ANALYSIS 2009 MUTCD WARRANTS

	2020 With	out Project						
	County:		_	District No.:				
	City:	Superior	Population: 4,000	_	Survey Date:	11/18/2016		
#	Route #	Name		Control	Section	85% Speed		
Major		US 60				45		
Minor		Main Stree	et			25		

Warrant 1: Eight- Hour Volumes Condition A

Number	r of Lanes		Major Both Ap	Street proaches	Minor High Volum	Street e Approach
Major	Street	Minor	Req	, uired	Requ	uired
wajor	Number of LanesMajor StreetMinor StreetVajorStreetBoth ApproachesHigh Volume ApproachesVajorStreetUrbanRequired115003501502 or more16004201502 or more2 or more60042020012 or more500350200	Rural*				
	1	1	500	350	150	105
2 or	more	1	600	420	150	105
2 or	more	2 or more	600	420	200	140
	1	2 or more	500	350	200	140

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

	Warrant 1						
					Cr	iteria	
	Ti	me	Vol	lume	Major	Minor	
	Begin	End	Major	Minor	>= 420	>= 140	Both Meet
	12:00 AM	1:00 AM	47.62702	3.2472965	N	N	N
	1:00 AM	2:00 AM	47.62702	4.3297286	N	Ν	N
	2:00 AM	3:00 AM	40.04999	1.0824322	N	Ν	N
	3:00 AM	4:00 AM	75.77025	2.1648643	N	Ν	N
	4:00 AM	5:00 AM	203.4972	10.824322	N	Ν	N
	5:00 AM	6:00 AM	408.0769	12.989186	N	Ν	N
	6:00 AM	7:00 AM	504.4134	16.236482	Y	Ν	N
	7:00 AM	8:00 AM	569.3593	40.04999	Y	Ν	N
	8:00 AM	9:00 AM	579.1012	36.802693	Y	Ν	N
	9:00 AM	10:00 AM	625.6458	43.297286	Y	Ν	N
	10:00 AM	11:00 AM	676.5201	50.874312	Y	Ν	N
	11:00 AM	12:00 PM	685.1796	49.791879	Y	Ν	N
	12:00 PM	1:00 PM	699.2512	60.616201	Y	Ν	N
	1:00 PM	2:00 PM	753.3728	63.863497	Y	N	N
	2:00 PM	3:00 PM	891.9241	53.039176	Y	N	N
	3:00 PM	4:00 PM	961.1998	67.110794	Y	Ν	N
	4:00 PM	5:00 PM	991.5079	62.781065	Y	Ν	N
	5:00 PM	6:00 PM	909.243	55.20404	Y	Ν	N
	6:00 PM	7:00 PM	839.9674	29.225668	Y	Ν	N
	7:00 PM	8:00 PM	585.5958	23.813508	Y	Ν	N
	8:00 PM	9:00 PM	392.9229	14.071618	N	Ν	N
	9:00 PM	10:00 PM	278.1851	17.318915	N	Ν	N
	10:00 PM	11:00 PM	199.1675	14.071618	N	Ν	N
	11:00 PM	12:00 AM	127.727	15.15405	N	N	N
			Total numb	er of hours, b	ooth the major(both	
			approaches	s) and minor(high volume ap	proach) met:	0
Condition	A is not sat	tisfied			Ho	ours Required:	8
Warrant 1	not satisfie	d.					

Warrant 1: Eight- Hour Volumes Condition B

Number	of Lanes		Major Both App	Street proaches	Minor High Volum	Street e Approach	
Major	Street	Minor	Req	uired	Required		
wajoi	Vajor Street	Street	Urban	Rural*	Urban	Rural*	
1		1	750	525	75	53	
2 or r	nore	1	900	630	75	53	
2 or r	nore	2 or more	900	630	100	70	
1		2 or more	750	525	100	70	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 2						
				Cri	teria	
T	ime	Vol	ume	Major	Minor	
Begin	End	Major	Minor	>= 630	>=70	Both Meet
12:00 AM	1:00 AM	47.62702	3.2472965	N	N	N
1:00 AM	2:00 AM	47.62702	4.3297286	Ν	Ν	N
2:00 AM	3:00 AM	40.04999	1.0824322	Ν	Ν	N
3:00 AM	4:00 AM	75.77025	2.1648643	Ν	Ν	N
4:00 AM	5:00 AM	203.4972	10.824322	Ν	Ν	N
5:00 AM	6:00 AM	408.0769	12.989186	Ν	Ν	Ν
6:00 AM	7:00 AM	504.4134	16.236482	Ν	Ν	N
7:00 AM	8:00 AM	569.3593	40.04999	Ν	Ν	Ν
8:00 AM	9:00 AM	579.1012	36.802693	Ν	Ν	N
9:00 AM	10:00 AM	625.6458	43.297286	Ν	Ν	Ν
10:00 AM	11:00 AM	676.5201	50.874312	Y	Ν	Ν
11:00 AM	12:00 PM	685.1796	49.791879	Y	Ν	Ν
12:00 PM	1:00 PM	699.2512	60.616201	Y	Ν	Ν
1:00 PM	2:00 PM	753.3728	63.863497	Y	Ν	Ν
2:00 PM	3:00 PM	891.9241	53.039176	Y	Ν	Ν
3:00 PM	4:00 PM	961.1998	67.110794	Y	Ν	Ν
4:00 PM	5:00 PM	991.5079	62.781065	Y	Ν	Ν
5:00 PM	6:00 PM	909.243	55.20404	Y	Ν	Ν
6:00 PM	7:00 PM	839.9674	29.225668	Y	Ν	N
7:00 PM	8:00 PM	585.5958	23.813508	Ν	Ν	N
8:00 PM	9:00 PM	392.9229	14.071618	Ν	Ν	N
9:00 PM	10:00 PM	278.1851	17.318915	Ν	Ν	N
10:00 PM	11:00 PM	199.1675	14.071618	Ν	Ν	N
11:00 PM	12:00 AM	127.727	15.15405	N	N	Ν

Total number of hours, both the major(both

approaches) and minor(high volume approach) met:

Hours Required: 8

0

Condition B is not satisfied Warrant 1 not satisfied.

Warrant 2: Four Hour Vehicular Volumes

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of an average day. The traffic volumes required are based on curves (Figure 4C-1) shown in the MUTCD.

* The required traffic volumes for Warrant 2 do not meet for any one hour.

Warrant 2 is not satisfied

Warrant 3, Condition A- Peak Hour Delay

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehiclehours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - Peak Hour Volume

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This w is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is N/A.

Warrant 4: Pedestrian Volume							
Required*	Existing						

100 or more for each of any four hours

OR

190 or more during any one hour

* For predominant pedestrian crossing speeds less than 3.5 ft/sec, the pedestrian volume may be reduced as much as 50 percent.

Gap Requirements

YES	NO	Is the nearest signal located more than 300 feet away?
YES	NO	For traffic flow which is not platooned, are there less than 60 gaps per hour of adequate
		length for the pedestrians to cross the street?

Warrant 4 is N/A.

Warrant 5: School Crossing

YES NO Is the number of adequate gaps in traffic stream during the period when the children are using the crossing less than the number of minutes in the same period?

Warrant 5 is N/A.

Warrant 6: Co	oordinate	e Systems
YES YES	NO NO	Are the adjacent signals in a signal system? Would the resultant spacing be 1000 feet or more?
Warrant 6 is	s N/A.	
Warrant 7: Cr	ash Exp	erience
YES YES	NO NO	Is 80% or more of one of Warrants #1, #2, or #3 met? Have there been more than five accidents susceptible to correction by a traffic signal in 12 months?
Warrant 7 is	s N/A.	
Warrant 8: Ro	badway N	Network
YES	NO	Does the major street having an existing or immediately projected entering volume of > 1000 vehicles per hour of a typical weekday?
YES	NO	Do 5-year projected traffic volumes meet Warrants 1, 2, or 3?
YES	NO	Is there an entering traffic volume of at least 1000 vehicles per hour for each of any 5 hours on a Saturday or Sunday?
Warrant 8 is	s N/A.	

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none

Warrant 2 - Four Hour Vehicular Volumes

85th % speed: > 40 mph Population: < 10,000 45 Major Street Lanes: 2 Minor Street Lanes: 2

Use Figure: 4C-2 2&2

Pank	Major Street	Minor Street		Figure 4C-1	1	Figure 4C-2		
	Volume	Volume	1&1	2&1	2&2	1&1	2&1	2&2
1	127.7269949	15.1540502	-	-	-	-	-	N
2	47.62701504	3.24729648	-	-	-	-	-	N
3	47.62701504	4.32972864	-	-	-	-	-	N
4	40.04998992	1.08243216	-	-	-	-	-	N
5	75.7702512	2.16486432	-	-	-	-	-	N
6	203.4972461	10.8243216	-	-	-	-	-	N
7	408.0769243	12.9891859	-	-	-	-	-	N
8	504.4133866	16.2364824	-	-	-	-	-	N
9	569.3593162	40.0499899	-	-	-	-	-	N
10	579.1012056	36.8026934	-	-	-	-	-	N
11	625.6457885	43.2972864	-	-	-	-	-	N
12	676.5201	50.8743115	-	-	-	-	-	N
13	685.1795573	49.7918794	-	-	-	-	-	N
14	699.2511754	60.616201	-	-	-	-	-	N
15	753.3727834	63.8634974	-	-	-	-	-	N
16	891.9240998	53.0391758	-	-	-	-	-	N
17	961.1997581	67.1107939	-	-	-	-	-	N
18	991.5078586	62.7810653	-	-	-	-	-	N
19	909.2430144	55.2040402	-	-	-	-	-	N
20	839.9673562	29.2256683	-	-	-	-	-	N
21	585.5957986	23.8135075	-	-	-	-	-	N
22	392.9228741	14.0716181	-	-	-	-	-	N
23	278.1850651	17.3189146	-	-	-	-	-	N
24	199.1675174	14.0716181	-	-	-	-	-	Ν
			0	0	0	0	0	0
Warrant 2 is not	Warrant 2 is not satisfied. N N		N	Ν	N	Ν		

Warrant 2 Figure 4C-2 Four Hour Volume Warrant (population<10,000 or >40 mph on major street)



* If data point is outside graph boundaries, it is plotted at the maximum shown value(s).

Major Street-Total of Both Approaches-vph



Enter Traffic Volumes:

Automated Tra	affic Counts		17 11 101 1 1 0 T						
Street: Location:	US 60 Main Street		1,20 1,00 H 80 H 60						
City/State: Project #: Date: Day of Week:	Superior, AZ 20010 11/18/2016 Friday		40 Achicles	0 0 0 100 300	500 700 900	1100 1300 150	0 1700	1900 2100 2300	
Data Source:	24-hour approach	1		. Follow		Time of Day	T - 1 - 1 - 1 - 1		
24-Hour Volume:	12,581		_	Eastbound		<u> </u>	Total Vehicles		
Time	Eastboun Vehicles	d Peds	Westbou Vehicles	nd Peds	Time	Eastbour Vehicles	nd Peds	Westbour Vehicles	nd Peds
12:00 AM					12:00 PM				
12:15 AM					12:15 PM				
12:30 AM					12:30 PM				
12:45 AM					12:45 PM				
1:00 AM	27		23		1:00 PM	340		387	
1:15 AM					1:15 PM				
1:30 AM					1:30 PM				
1:45 AM	<u></u>				1:45 PM			100	
2:00 AM	35		15		2:00 PM	361		422	
2:15 AM					2:15 PM				
2:30 AM 2:45 AM					2:30 PM 2:45 PM				
2.45 AM 3.00 AM	24		18		2.43 FM 3:00 PM	420		508	
3:15 AM	27		10		3:15 PM	420		500	
3:30 AM					3:30 PM				
3:45 AM					3:45 PM				
4:00 AM	44		35		4:00 PM	456		544	
4:15 AM					4:15 PM				
4:30 AM					4:30 PM				
4:45 AM					4:45 PM				
5:00 AM	140		72		5:00 PM	487		545	
5:15 AM					5:15 PM				
5:30 AM					5:30 PM				
5:45 AM					5:45 PM				
6:00 AM	300		125		6:00 PM	466		480	
6:15 AM					6:15 PM				
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM	330		195	 	7:00 PM	455		419	
7:15 AM					7:15 PM				
/:30 AM					/:30 PM				
/:43 AM 8:00 AM	251		220		/:45 PM 8:00 DM	202		212	
0.00 ANI 8·15 ΔΜ	334		239	<u> </u>	0.00 FW	390		212	
8.30 AM					8.30 PM				
8:45 AM					8:45 PM				
9:00 AM	322		280		9:00 PM	252		157	
9:15 AM			~ ~		9:15 PM				
9:30 AM					9:30 PM				
9:45 AM					9:45 PM				
10:00 AM	313		338		10:00 PM	203		87	
10:15 AM					10:15 PM				
10:30 AM					10:30 PM				
10:45 AM					10:45 PM				
11:00 AM	367		337		11:00 PM	155		52	
11:15 AM					11:15 PM				
11:30 AM					11:30 PM				
11:45 AM					11:45 PM				
12:00 PM	367		346		12:00 AM	106		27	
				1		6,721	II	5,861	
Equipment ID#:				1		24-	Hour Volume	12,581	

-	~								
Street: 1 Location: 1	Main Street US 60		Hour	80 70 60					
City/State: S	Superior, AZ		nicles per	50 40 30					
Date:	11/18/2016		Veh	10					
Dav of Week:]	Fridav			100 300	Image: wide wide wide wide wide wide wide wide	1100 1300 1500	1700 19	00 2100 2300	
Data Source: 2	24-hour approac	h				Time of Day			
-Hour Volume:	778				- Northbound -		—≜— Total V	ehicles	
Time	Northbou	nd	Southbou	ind	Time	Northbound		Southbour	nd
Thic	Vehicles	Peds	Vehicles	Peds		Vehicles	Peds	Vehicles	Pe
12:00 AM					12:00 PM				
12:15 AM					12:59 PM				
12:30 AM					12:30 PM				
12:45 AM					12:45 PM				
1:00 AM		├ ───┤	3	├ ───┤	1:00 PM	<u> </u>		63	
1:15 AM					1:59 PM				
1:30 AM					1:30 PM				
1:45 AM			_		1:45 PM				
2:00 AM			5	───┤	2:00 PM	<u>├</u>		66	
2:15 AM					2:59 PM				
2:30 AM					2:30 PM				
2:45 AM			4		2:45 PM				
3:00 AM			1		3:00 PM			55	
3:15 AM					3:59 PM				
3:30 AM					3:30 PM				
5:45 AM			2		5:43 PM			70	
4:00 AM			2		4:00 PM			70	
4.13 AM					4.30 PM				
4:45 AM					12:00 AM				
5:00 AM			11		5:00 PM			65	
5:15 AM					5:59 PM				
5:30 AM					5:30 PM				
5:45 AM					5:45 PM				
6:00 AM			14		6:00 PM			57	
6:15 AM					6:59 PM				
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM			17		7:00 PM			30	
7:15 AM					7:59 PM		T		
7:30 AM					7:30 PM				
7:45 AM					7:45 PM				
8:00 AM		ļ ļ	42		8:00 PM	ļ		25	
8:15 AM					8:59 PM				
8:30 AM					8:30 PM				
8:45 AM					8:45 PM				
9:00 AM		├	38	<u> </u>	9:00 PM	<u>├</u> ────		15	
9:15 AM					9:59 PM				
9:30 AM					9:30 PM				
9:45 AM			45		9:45 PM			40	
10:00 AM		<u>├</u>	45	┼───┤	10:00 PM	┼───┼─		٦ð	
10:15 AM					10:39 PM				
10:50 AW					10:50 PM				
10:45 AW			50		10:45 PM			16	
11:00 AW		├	33	╂────┨	11:00 PM	├		10	
11.13 AIVI					11:37 PW 11:20 DM				
11.30 AIVI					11.30 PIVI 11.45 DM				
12:00 PM			52		11.43 FIVI 12.00 AM			16	
12.001111		1	52	1 1	12.00 AIVI	1 1	1	10	

TRAFFIC SURVEY - COUNT ANALYSIS 2009 MUTCD WARRANTS

	2022 Witho	out Project				
	County:		_		District No.:	
	City:	Superior	Population: 4,00	0	Survey Date:	11/18/2016
	Route #	Name		Control	Section	85% Speed
Major		US 60				45
Minor		Main Stree	et			25

Warrant 1: Eight- Hour Volumes

Condition A

Numbe	r of Lanes		Major Both Ap	Street proaches	Minor High Volum	Street e Approach
Major	Street	Minor	Req	uired	Requ	lired
wajoi	Slieel	Street	Urban	Rural*	Urban	Rural*
	1	1	500	350	150	105
2 or	more	1	600	420	150	105
2 or	more	2 or more	600	420	200	140
	1	2 or more	500	350	200	140

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

	Warrant 1						
					Cri	teria	
	Ti	me	Vol	lume	Major	Minor	
	Begin	End	Major	Minor	>= 420	>= 140	Both Meet
	12:00 AM	1:00 AM	49.55115	3.3784873	N	Ν	N
	1:00 AM	2:00 AM	49.55115	4.5046497	Ν	Ν	N
	2:00 AM	3:00 AM	41.66801	1.1261624	N	Ν	N
	3:00 AM	4:00 AM	78.83137	2.2523248	Ν	Ν	N
	4:00 AM	5:00 AM	211.7185	11.261624	Ν	Ν	N
	5:00 AM	6:00 AM	424.5632	13.513949	Y	Ν	N
	6:00 AM	7:00 AM	524.7917	16.892436	Y	Ν	N
	7:00 AM	8:00 AM	592.3614	41.66801	Y	Ν	N
	8:00 AM	9:00 AM	602.4969	38.289522	Y	Ν	N
	9:00 AM	10:00 AM	650.9219	45.046497	Y	Ν	N
	10:00 AM	11:00 AM	703.8515	52.929634	Y	Ν	N
	11:00 AM	12:00 PM	712.8608	51.803471	Y	Ν	N
	12:00 PM	1:00 PM	727.5009	63.065095	Y	Ν	N
	1:00 PM	2:00 PM	783.809	66.443583	Y	Ν	N
	2:00 PM	3:00 PM	927.9578	55.181959	Y	Ν	N
	3:00 PM	4:00 PM	1000.032	69.82207	Y	Ν	N
	4:00 PM	5:00 PM	1031.565	65.31742	Y	Ν	N
	5:00 PM	6:00 PM	945.9764	57.434283	Y	Ν	N
	6:00 PM	7:00 PM	873.902	30.406385	Y	Ν	N
	7:00 PM	8:00 PM	609.2539	24.775573	Y	Ν	N
	8:00 PM	9:00 PM	408.797	14.640111	Ν	Ν	N
	9:00 PM	10:00 PM	289.4237	18.018599	Ν	Ν	N
	10:00 PM	11:00 PM	207.2139	14.640111	Ν	Ν	N
	11:00 PM	12:00 AM	132.8872	15.766274	Ν	Ν	N
			Total numb	er of hours, b	oth the major(b	oth	
			approaches	s) and minor(high volume ap	proach) met:	0
Condition	A is not sat	isfied			Ho	ours Required:	8
Warrant 1	not satisfie	d.				-	

Warrant 1: Eight- Hour Volumes Condition B

Numbe	er of Lanes		Major Both App	Street proaches	Minor High Volum	Street e Approach
Major	Stroot	Minor	Req	uired	Requ	uired
iviajui	Sileei	Street	Urban	Rural*	Urban	Rural*
	1	1	750	525	75	53
2 o	r more	1	900	630	75	53
2 o	r more	2 or more	900	630	100	70
	1	2 or more	750	525	100	70

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 2						
				Cri	teria	
Т	ime	Vo	lume	Major	Minor	
Begin	End	Major	Minor	>= 630	>=70	Both Meet
12:00 AM	1:00 AM	49.55115	3.3784873	Ν	N	N
1:00 AM	2:00 AM	49.55115	4.5046497	Ν	Ν	N
2:00 AM	3:00 AM	41.66801	1.1261624	Ν	Ν	N
3:00 AM	4:00 AM	78.83137	2.2523248	Ν	Ν	N
4:00 AM	5:00 AM	211.7185	11.261624	Ν	Ν	N
5:00 AM	6:00 AM	424.5632	13.513949	Ν	Ν	N
6:00 AM	7:00 AM	524.7917	16.892436	Ν	Ν	N
7:00 AM	8:00 AM	592.3614	41.66801	Ν	Ν	N
8:00 AM	9:00 AM	602.4969	38.289522	Ν	Ν	N
9:00 AM	10:00 AM	650.9219	45.046497	Y	Ν	N
10:00 AM	11:00 AM	703.8515	52.929634	Y	Ν	N
11:00 AM	12:00 PM	712.8608	51.803471	Y	Ν	N
12:00 PM	1:00 PM	727.5009	63.065095	Y	Ν	N
1:00 PM	2:00 PM	783.809	66.443583	Y	Ν	N
2:00 PM	3:00 PM	927.9578	55.181959	Y	Ν	N
3:00 PM	4:00 PM	1000.032	69.82207	Y	Ν	N
4:00 PM	5:00 PM	1031.565	65.31742	Y	Ν	N
5:00 PM	6:00 PM	945.9764	57.434283	Y	Ν	N
6:00 PM	7:00 PM	873.902	30.406385	Y	Ν	N
7:00 PM	8:00 PM	609.2539	24.775573	Ν	Ν	N
8:00 PM	9:00 PM	408.797	14.640111	Ν	Ν	N
9:00 PM	10:00 PM	289.4237	18.018599	Ν	Ν	N
10:00 PM	11:00 PM	207.2139	14.640111	N	Ν	N
11:00 PM	12:00 AM	132.8872	15.766274	Ν	Ν	N

Total number of hours, both the major(both approaches) and minor(high volume approach) met:

0

8

Hours Required:

Condition B is not satisfied Warrant 1 not satisfied.

Warrant 2: Four Hour Vehicular Volumes

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of an average day. The traffic volumes required are based on curves (Figure 4C-1) shown in the MUTCD.

* The required traffic volumes for Warrant 2 do not meet for any one hour.

Warrant 2 is not satisfied

Warrant 3, Condition A- Peak Hour Delay

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - Peak Hour Volume

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This wa is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is N/A.

Warrant 4: Pede	estrian Vo	blume
	Requ	ired* Existing
100 or more fo 190 or more du	r each of O uring any	any four hours R one hour
* For predomina 50 percent.	nt pedest	rian crossing speeds less than 3.5 ft/sec, the pedestrian volume may be reduced as much as
Gap Requirem	ents	
YES	NO	Is the nearest signal located more than 300 feet away?
YES	NO	For traffic flow which is not platooned, are there less than 60 gaps per hour of adequate length for the pedestrians to cross the street?
Warrant 4 is N	I/A.	

Warrant 5: School Crossing

YES NO Is the number of adequate gaps in traffic stream during the period when the children are using the crossing less than the number of minutes in the same period?

Warrant 5 is N/A.

Warrant 6: C	oordinate	e Systems	
YES	NO	Are the adjacent signals in a signal system?	
YES	NO	Would the resultant spacing be 1000 feet or more?	
Warrant 6	is N/A.		

Warrant 7: Crash Experience

YES	NO	Is 80% or more of one of Warrants #1, #2, or #3 met?
YES	NO	Have there been more than five accidents susceptible to correction by a traffic signal in 12
		months?

Warrant 7 is N/A.

Warrant 8: Roadway Network

YES	NO	Does the major street having an existing or immediately projected entering volume of > 1000 vehicles per hour of a typical weekday?
YES	NO	Do 5-year projected traffic volumes meet Warrants 1, 2, or 3?
YES Warrant 8 is	NO 8 N/A	Is there an entering traffic volume of at least 1000 vehicles per hour for each of any 5 hours on a Saturday or Sunday?

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none

85th % speed: > 40 mph Population: < 10,000

Major Street Lanes: 2 Minor Street Lanes: 2

Use Figure: 4C-2 2&2

Pank	Major Street	Minor Street		Figure 4C-1			Figure 4C-2	2
Γατικ	Volume	Volume	1&1	2&1	2&2	1&1	2&1	2&2
1	132.8871655	15.7662739	-	-	-	-	-	N
2	49.55114645	3.37848726	-	- 1	-	-	-	N
3	49.55114645	4.50464968	-	- 1	-	-	-	N
4	41.66800951	1.12616242	-	- 1	-	-	-	N
5	78.83136935	2.25232484	-	-	-	-	-	N
6	211.7185348	11.2616242	-	-	-	-	-	N
7	424.5632321	13.513949	-	- 1	-	-	-	N
8	524.7916874	16.8924363	-	- 1	-	-	-	N
9	592.3614325	41.6680095	-	-	-	-	-	N
10	602.4968943	38.2895223	-	- 1	-	-	-	N
11	650.9218783	45.0464968	-	-	-	-	-	N
12	703.851512	52.9296337	-	-	-	-	-	N
13	712.8608114	51.8034713	-	-	-	-	-	N
14	727.5009228	63.0650955	-	-	-	-	-	N
15	783.8090438	66.4435827	-	-	-	-	-	N
16	927.9578335	55.1819585	-	-	-	-	-	N
17	1000.032228	69.82207	-	-	-	-	-	N
18	1031.564776	65.3174203	-	-	-	-	-	N
19	945.9764322	57.4342834	-	-	-	-	-	N
20	873.9020373	30.4063853	-	-	-	-	-	N
21	609.2538688	24.7755732	-	-	-	-	-	N
22	408.7969582	14.6401115	-	-	-	-	-	N
23	289.4237418	18.0185987	-	-	-	-	-	N
24	207.2138851	14.6401115	-	-	-	-	-	N
			0	0	0	0	0	0
Warrant 2 is not	satisfied.		Ν	N	Ν	Ν	Ν	N

 Warrant 2

 Figure 4C-2
 Four Hour Volume Warrant (population<10,000 or >40 mph on major street)



* If data point is outside graph boundaries, it is plotted at the maximum shown value(s).

Major Street-Total of Both Approaches-vph



Enter Traffic Volumes:

Automated Tra	affic Counts	_ •							
Street: Location: 1	US 60 Main Street		1,20 Inon 1,00 Inon 80 Inon 60						
City/State: S Project #: 2	Superior, AZ 20010		40 Achicles 20						
Date: 1 Day of Week: 1 Data Source: 2	Friday 24-hour approach	L		100 300	500 700 900	1100 1300 150 Time of Day	00 1700	1900 2100 2300)
24-Hour Volume:	13,891		-	Eastbound		<u></u> `	Total Vehicles		
Time	Eastboun Vehicles	d Peds	Westbou Vehicles	nd Peds	Time	Eastbour Vehicles	nd Peds	Westbour	nd Peds
12:00 AM	v chicles	T cus	Venicies		12:00 PM	Venicies	1 003	Venicies	T cus
12:15 AM					12:00 PM				
12:10 AM					12:30 PM				
12:45 AM					12:45 PM				
1:00 AM	30		25		1:00 PM	375		428	
1:15 AM					1:15 PM			0	
1:30 AM					1:30 PM				
1:45 AM					1:45 PM				
2:00 AM	39		16		2:00 PM	399		466	
2:15 AM					2:15 PM				
2:30 AM					2:30 PM				
2:45 AM					2:45 PM				
3:00 AM	26		20		3:00 PM	464		561	
3:15 AM					3:15 PM				
3:30 AM					3:30 PM				
3:45 AM					3:45 PM				
4:00 AM	48		39		4:00 PM	504		601	
4:15 AM					4:15 PM				
4:30 AM					4:30 PM				
4:45 AM					4:45 PM				
5:00 AM	154		80		5:00 PM	537		602	
5:15 AM					5:15 PM				
5:30 AM					5:30 PM				
5:45 AM					5:45 PM				
6:00 AM	331		138		6:00 PM	515		530	
6:15 AM					6:15 PM				
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM	364		215	ļ	7:00 PM	502		463	
7:15 AM					7:15 PM				
7:30 AM					7:30 PM				
7:45 AM			0 5 i		7:45 PM				
8:00 AM	390		264	<u> </u>	8:00 PM	439		234	
8:15 AM					8:15 PM				
8:30 AM					8:30 PM				
0:00 AM	256		210		0.00 PM	270		170	
9.00 AW	330		510	 	0.15 DM	213		113	
9.13 AIVI 0.30 AM					9.13 MVI 0.30 DM				
9.45 AM					9.30 F M 9.45 PM				
10.00 AM	346		373		10.00 PM	224		96	
10:15 AM	570		0,0		10.00 I M				
10:30 AM					10:30 PM				
10:45 AM					10:45 PM				
11:00 AM	405		372		11:00 PM	172		57	
11:15 AM					11:15 PM				
11:30 AM					11:30 PM				
11:45 AM					11:45 PM				
12:00 PM	405		382		12:00 AM	117		30	
	,			·1		7,420		6,471	
Equipment ID#:						24-	Hour Volume	13,891	

Straat	Main Street			90					
Location:	US 60		Hour	80 70					
Citv/State:	Superior, AZ		es ber	60 50 40					
Project #:			ehicl	30 20					
Date:	11/18/2016		5						- ↓
Day of Week:	Friday			100 300	500 700 900	1100 1300 1500	0 1700	1900 2100 2300	
Data Source:	24-hour approac	h				Time of Day			
Hour Volume:	859				- Northbound ·		—≜— Tota	al Vehicles	
Time	Northbou	nd	Southbou	und	Time	Northbour	nd	Southbou	nd
10.00.435	Vehicles	Peds	Vehicles	Peds		Vehicles	Peds	Vehicles	Pe
12:00 AM					12:00 PM				
12:15 AM					12:59 PM				
12:30 AM					12:30 PM				
12:45 AM					12:45 PM			70	
1:00 AM			4	┼───┤	1:00 PM			/0	
1.1.5 AIVI 1.30 AM					1.39 PW 1.30 DM				
1.30 AIVI 1.45 AM					1.50 FIVI 1.45 DM				
2.00 AM			F		1.43 MVI 2.00 DM			72	
2:15 AM			5	<u> </u>	2.00 F WI 2.59 PM			13	
2:30 AM					2:30 PM				
2:45 AM					2:45 PM				
3:00 AM			1		3:00 PM			61	
3:15 AM			•		3:59 PM			01	
3:30 AM					3:30 PM				
3:45 AM					3:45 PM				
4:00 AM			2		4.00 PM			77	
4:15 AM					4:59 PM				
4:30 AM					4:30 PM				
4:45 AM					12:00 AM				
5:00 AM			12		5:00 PM			72	
5:15 AM					5:59 PM				
5:30 AM					5:30 PM				
5:45 AM					5:45 PM				
6:00 AM			15		6:00 PM			63	
6:15 AM					6:59 PM				
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM			19	ļ	7:00 PM			34	
7:15 AM					7:59 PM				
7:30 AM					7:30 PM				
7:45 AM					7:45 PM				
8:00 AM			46	<u> </u>	8:00 PM			27	
8:15 AM					8:59 PM				
8:30 AM					8:30 PM				
8:45 AM			40		8:45 PM			40	
9:00 AM			42	┼───┤	9:00 PM			10	
9:15 AM					9:59 PM				
9.30 AM					9:50 PM				
2.4.2 AIVI			50		7.43 FIVI			20	
10.00 AW			50	+	10.50 PM			20	
10.13 AW					10.37 FWI 10.30 DM				
10.30 AIVI					10.30 PM 10.45 DM				
10.45 AIVI			58		10.45 PM 11.00 DM			16	
11.00 AW			50	+	11.50 PM				
11:30 AM					11.37 PM				
11:45 AM					11:45 PM				
12:00 PM			57		12:00 AM			17	
		-							

TRAFFIC SURVEY - COUNT ANALYSIS 2009 MUTCD WARRANTS

	2027 With					
	County:				District No.:	
	City:	Superior	Population: 4,000	_	Survey Date:	11/18/2016
	Route #	Name		Control	Section	85% Spee
Major		US 60				45
Minor		Main Stre	et			25

Warrant 1: Eight- Hour Volumes

Condition A

Number of Lanes			Major Both Ap	Street proaches	Minor Street High Volume Approach		
Major	Street	Minor	Req	uired	Requ	lired	
Major	Slieel	Street	Urban	Rural*	Urban	Rural*	
	1	1	500	350	150	105	
2 or more		1	600	420	150	105	
2 or more		2 or more	600	420	200	140	
1		2 or more	500	350	200	140	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

	Warrant 1											
					Cri	teria						
	Ti	me	Vo	lume	Major	Minor						
	Begin	End	Major	Minor	>= 420	>= 140	Both Meet					
	12:00 AM	1:00 AM	54.70847	3.7301229	N	N	N					
	1:00 AM	2:00 AM	54.70847	4.9734972	N	Ν	N					
	2:00 AM	3:00 AM	46.00485	1.2433743	N	Ν	N					
	3:00 AM	4:00 AM	87.0362	2.4867486	N	Ν	N					
	4:00 AM	5:00 AM	233.7544	12.433743	Ν	Ν	N					
	5:00 AM	6:00 AM	468.7521	14.920492	Y	Ν	N					
	6:00 AM	7:00 AM	579.4124	18.650615	Y	Ν	N					
	7:00 AM	8:00 AM	654.0149	46.004849	Y	Ν	N					
	8:00 AM	9:00 AM	665.2053	42.274726	Y	Ν	N					
	9:00 AM	10:00 AM	718.6704	49.734972	Y	Ν	N					
	10:00 AM	11:00 AM	777.1089	58.438592	Y	Ν	N					
	11:00 AM	12:00 PM	787.0559	57.195218	Y	Ν	N					
	12:00 PM	1:00 PM	803.2198	69.628961	Y	Ν	N					
	1:00 PM	2:00 PM	865.3885	73.359084	Y	Ν	N					
	2:00 PM	3:00 PM	1024.54	60.925341	Y	Ν	N					
	3:00 PM	4:00 PM	1104.116	77.089207	Y	Ν	N					
	4:00 PM	5:00 PM	1138.931	72.11571	Y	Ν	N					
	5:00 PM	6:00 PM	1044.434	63.41209	Y	Ν	N					
	6:00 PM	7:00 PM	964.8585	33.571106	Y	Ν	N					
	7:00 PM	8:00 PM	672.6655	27.354235	Y	Ν	N					
	8:00 PM	9:00 PM	451.3449	16.163866	Y	Ν	N					
	9:00 PM	10:00 PM	319.5472	19.893989	N	Ν	N					
	10:00 PM	11:00 PM	228.7809	16.163866	Ν	Ν	N					
	11:00 PM	12:00 AM	146.7182	17.40724	Ν	Ν	N					
			Total numb	er of hours, b	oth the major(b	oth						
			approaches	s) and minor(high volume ap	proach) met:	0					
Condition	A is not sat	isfied			Ho	ours Required:	8					
Warrant 1	not satisfie	d.										

Warrant 1: Eight- Hour Volumes Condition B

Number of Lanes			Major Both App	Street proaches	Minor Street High Volume Approach		
Major	Stroot	Minor	Req	uired	Requ	lired	
	Sileei	Street	Urban	Rural*	Urban	Rural*	
	1	1	750	525	75	53	
2 or more		1	900	630	75	53	
2 or more		2 or more	900	630	100	70	
	1	2 or more	750	525	100	70	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 2	Warrant 2									
				Cri	teria					
Т	ime	Volume		Major	Minor					
Begin	End	Major	Minor	>= 630	>=70	Both Meet				
12:00 AM	1:00 AM	54.70847	3.7301229	Ν	Ν	N				
1:00 AM	2:00 AM	54.70847	4.9734972	Ν	Ν	N				
2:00 AM	3:00 AM	46.00485	1.2433743	Ν	Ν	N				
3:00 AM	4:00 AM	87.0362	2.4867486	Ν	Ν	N				
4:00 AM	5:00 AM	233.7544	12.433743	Ν	Ν	N				
5:00 AM	6:00 AM	468.7521	14.920492	Ν	Ν	N				
6:00 AM	7:00 AM	579.4124	18.650615	Ν	Ν	N				
7:00 AM	8:00 AM	654.0149	46.004849	Y	Ν	N				
8:00 AM	9:00 AM	665.2053	42.274726	Y	Ν	N				
9:00 AM	10:00 AM	718.6704	49.734972	Y	Ν	N				
10:00 AM	11:00 AM	777.1089	58.438592	Y	Ν	N				
11:00 AM	12:00 PM	787.0559	57.195218	Y	Ν	N				
12:00 PM	1:00 PM	803.2198	69.628961	Y	Ν	N				
1:00 PM	2:00 PM	865.3885	73.359084	Y	Y	Y				
2:00 PM	3:00 PM	1024.54	60.925341	Y	Ν	N				
3:00 PM	4:00 PM	1104.116	77.089207	Y	Y	Y				
4:00 PM	5:00 PM	1138.931	72.11571	Y	Y	Y				
5:00 PM	6:00 PM	1044.434	63.41209	Y	Ν	N				
6:00 PM	7:00 PM	964.8585	33.571106	Y	Ν	N				
7:00 PM	8:00 PM	672.6655	27.354235	Y	Ν	N				
8:00 PM	9:00 PM	451.3449	16.163866	Ν	Ν	N				
9:00 PM	10:00 PM	319.5472	19.893989	Ν	Ν	N				
10:00 PM	11:00 PM	228.7809	16.163866	Ν	Ν	N				
11:00 PM	12:00 AM	146.7182	17.40724	Ν	Ν	N				

Total number of hours, both the major(both approaches) and minor(high volume approach) met:

3

8

Hours Required:

Condition B is not satisfied Warrant 1 not satisfied.

Warrant 2: Four Hour Vehicular Volumes

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of ar average day. The traffic volumes required are based on curves (Figure 4C-1) shown in the MUTCD.

* The required traffic volumes for Warrant 2 do not meet for any one hour.

Warrant 2 is not satisfied

Warrant 3, Condition A- Peak Hour Delay

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - Peak Hour Volume

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is N/A.

Warrant 4: Pe	edestrian	Volume
	Re	quired* Existing
100 or more 190 or more	e for each o e during an	of any four hours OR by one hour
* For predom 50 percent.	inant pede	estrian crossing speeds less than 3.5 ft/sec, the pedestrian volume may be reduced as much as
Gap Requir	ements	
YES	NO	Is the nearest signal located more than 300 feet away?
YES	NO	For traffic flow which is not platooned, are there less than 60 gaps per hour of adequate length for the pedestrians to cross the street?
Warrant 4 i	is N/A.	

Warrant 5: Se	chool Cro	ssing
YES	NO	Is the number of adequate gaps in traffic stream during the period when the children are using the crossing less than the number of minutes in the same period?
Warrant 5	is N/A.	
Warrant 6: C	oordinate	Systems
YES	NO	Are the adjacent signals in a signal system?
YES	NO	Would the resultant spacing be 1000 feet or more?
Warrant 6	is N/A.	
Warrant 7: C	rash Expe	rience
YES	NO	Is 80% or more of one of Warrants #1, #2, or #3 met?
YES	NO	Have there been more than five accidents susceptible to correction by a traffic signal in 12 months?

Warrant 7 is N/A.

Warrant 8: Roadway Network

YES	NO	Does the major street having an existing or immediately projected entering volume of > 1000 vehicles per hour of a typical weekday?
YES	NO	Do 5-year projected traffic volumes meet Warrants 1, 2, or 3?
YES	NO	Is there an entering traffic volume of at least 1000 vehicles per hour for each of any 5 hours on a Saturday or Sunday?
Warrant 8 i	s N/A.	

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none

85th % speed: > 40 mph Population: < 10,000

Major Street Lanes: 2 Minor Street Lanes: 2

Use Figure: 4C-2 2&2

Pank	Major Street Minor Street			Figure 4C-1			Figure 4C-2		
Ndlik	Volume	Volume	1&1	2&1	2&2	1&1	2&1	2&2	
1	146.7181684	17.4072403	-	-	-	-	-	N	
2	54.70846957	3.73012293	-	-	-	-	-	N	
3	54.70846957	4.97349723	-	-	-	-	-	N	
4	46.00484941	1.24337431	-	-	-	-	-	N	
5	87.03620159	2.48674862	-	-	-	-	-	N	
6	233.75437	12.4337431	-	-	-	-	-	N	
7	468.7521143	14.9204917	-	-	-	-	-	N	
8	579.4124277	18.6506146	-	-	-	-	-	N	
9	654.0148862	46.0048494	-	-	-	-	-	N	
10	665.205255	42.2747265	-	-	-	-	-	N	
11	718.6703503	49.7349723	-	-	-	-	-	N	
12	777.1089427	58.4385925	-	-	-	-	-	N	
13	787.0559372	57.1952182	-	-	-	-	-	N	
14	803.2198032	69.6289613	-	-	-	-	-	N	
15	865.3885186	73.3590842	-	-	-	-	-	N	
16	1024.54043	60.9253411	-	-	-	-	-	N	
17	1104.116386	77.0892071	-	-	-	-	-	N	
18	1138.930866	72.1157099	-	-	-	-	-	N	
19	1044.434419	63.4120897	-	-	-	-	-	N	
20	964.8584633	33.5711063	-	-	-	-	-	N	
21	672.6655008	27.3542348	-	-	-	-	-	N	
22	451.3448739	16.163866	-	-	-	-	-	N	
23	319.5471973	19.8939889	-	-	-	-	-	N	
24	228.7808727	16.163866	-	-	-	-	-	N	
			0	0	0	0	0	0	
Warrant 2 is not	ant 2 is not satisfied. N N N N N				N	Ν			

 Warrant 2

 Figure 4C-2
 Four Hour Volume Warrant (population<10,000 or >40 mph on major street)



* If data point is outside graph boundaries, it is plotted at the maximum shown value(s).

Major Street-Total of Both Approaches-vph



Enter Traffic Volumes:

Automated Traffic Counts

Street: US 60

Location: Main Street City/State: Superior, AZ Project #: 20010 Date: 11/18/2016 Day of Week: Friday

Data Source: 24-hour approach



24-Hour Volume: 13,406

There	Eastbour	nd	Westbour	nd	Time	Eastbound		Westbound	
Time	Vehicles	Peds	Vehicles	Peds	Time	Vehicles	Peds	Vehicles	Peds
12:00 AM					12:00 PM				
12:15 AM					12:15 PM				
12:30 AM					12:30 PM				
12:45 AM					12:45 PM				
1:00 AM	27		23		1:00 PM	340		387	
1:15 AM					1:15 PM				
1:30 AM					1:30 PM				
1:45 AM					1:45 PM				
2:00 AM	35		15		2:00 PM	361		422	
2:15 AM					2:15 PM				
2:30 AM					2:30 PM				
2:45 AM					2:45 PM				
3:00 AM	24		18		3:00 PM	420		508	
3:15 AM					3:15 PM				
3:30 AM					3:30 PM				
3:45 AM					3:45 PM				
4:00 AM	44		35		4:00 PM	456		544	
4:15 AM					4:15 PM				
4:30 AM					4:30 PM				
4:45 AM					4:45 PM				
5:00 AM	140		72		5:00 PM	490		611	
5:15 AM					5:15 PM				
5:30 AM					5:30 PM				
5:45 AM					5:45 PM				
6:00 AM	300		125		6:00 PM	466		480	
6:15 AM					6:15 PM				
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM	330		195		7:00 PM	455		419	
7:15 AM	550		170		7:15 PM	100			
7:30 AM					7:30 PM				
7:45 AM					7:45 PM				
8:00 AM	1001		348		8:00 PM	398		212	
8:15 AM	1001		040		8:15 PM	000		212	
8-30 AM					8:30 PM				
8:45 AM					8:45 PM				
9:00 AM	322		280		9:00 PM	252		157	
9-15 AM	522		200		Q-15 DM	202		107	
9.13 AW					9.10 FW				
9.45 AM					9.45 DM				
10:00 AM	313		338		10.00 PM	203		87	
10.00 AW	515		000		10.15 DM	200		01	
10:15 AIVI 10:20 AM					10:15 PM				
10.30 AIVI					10.30 FW				
10.43 AIVI	267		227		10.45 FW	155		50	
11:00 AM	307		221		11:00 PM	100		IJZ	
11:13 AM					11:15 PM				
11:50 AM					11:30 PM				
11:40 AM	2/7		246		11:40 PM	106		27	
12:00 PM	30/		340		12:00 AM	106	I	21	
Emilia ID#						/,3/1	Hour V-l	6,036	
Equipment ID#:						24-	nour volume	13,400	

Street: N	Main Street			900					-
Location: U	J S 60		Hour	800 700			Å		
			per l	600 500					-
City/State: S	Superior, AZ		cles	400 300					-
Project #:	1/10/2017		Vehi	200					
Date: I	1/18/2016 Friday			0 0 0 0 0	500 700 900	1100 1300 150	0 1700 1	<u>● ● ● ● ●</u> 900 2100 2300	-6
Data Source: 2	4-hour approact	h				Time of Day			
		-			- Nothbourd	Southbound	Tota	Vahialas	
4-Hour Volume:	1,525				Normbound	-e- Soumbound	Tota	vencies	
	Northbour	nd	Southbo	und	T .	Northbou	nd	Southbou	nd
Time	Vehicles	Peds	Vehicles	Peds	Time	Vehicles	Peds	Vehicles	Ped
12:00 AM					12:00 PM				
12:15 AM					12:59 PM				
12:30 AM					12:30 PM				
12:45 AM					12:45 PM				1
1:00 AM			3		1:00 PM			63	
1:15 AM					1:59 PM				ł
1:30 AM					1:30 PM				ł
1:45 AM			-		1:45 PM			00	ł
2:00 AM			5	┼───┤	2:00 PM			66	
2:15 AM					2:39 PM				ł
2:50 AIVI 2:45 AM					2:50 PM				ł
2:45 AM 2:00 AM			1		2:45 PM 2:00 PM			55	
3:15 AM			1		3:59 PM			55	
3:30 AM					3:30 PM				1
3:45 AM					3:45 PM				1
4:00 AM			2		4:00 PM			70	
4:15 AM					4:59 PM			-	
4:30 AM					4:30 PM				
4:45 AM					12:00 AM				
5:00 AM			11		5:00 PM			782	
5:15 AM					5:59 PM				
5:30 AM					5:30 PM				1
5:45 AM					5:45 PM				
6:00 AM			14		6:00 PM			57	l
6:15 AM					6:59 PM				
0:30 AM					6:30 PM				ł
0:45 AM			17		0:45 PM			30	ł
7:00 AM			17	┼───┤	7:00 PM	1		30	
7:30 AM					7.30 PM				ł
7:45 AM					7:45 PM				ł
8:00 AM			72		8:00 PM			25	ł
8:15 AM				1	8:59 PM			-	
8:30 AM					8:30 PM				ł
8:45 AM					8:45 PM				ł
9:00 AM			38		9:00 PM			15	L
9:15 AM					9:59 PM				
9:30 AM					9:30 PM				ł
9:45 AM					9:45 PM				ł
10:00 AM			45		10:00 PM			18	
10:15 AM					10:59 PM				ł
10:30 AM					10:30 PM				ł
10:45 AM			50		10:45 PM			4-	ł
11:00 AM			53	┼───┤	11:00 PM			15	
11:15 AM					11:39 PM				ł
11:50 AM					11:30 PM 11:45 PM				ł
12:00 PM			52		12:00 AM			16	ł
									-

TRAFFIC SURVEY - COUNT ANALYSIS 2009 MUTCD WARRANTS

	2022 Wit						
	County:					District No.:	
	City:	Superior	Population:	4,000	_	Survey Date:	11/18/2016
	Route #	Name			Control	Section	85% Spee
Major		US 60					45
Minor		Main Stre	et				25

Warrant 1: Eight- Hour Volumes

Condition A

Number of Lanes			Major Both Ap	⁻ Street proaches	Minor Street High Volume Approach		
Major	Street	Minor	Req	uired	Required		
	Slieel	Street	Urban	Rural*	Urban	Rural*	
1		1	500	350	150	105	
2 or more		1	600	420	150	105	
2 or more		2 or more	600	420	200	140	
1		2 or more	500	350	200	140	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

	Warrant 1						
					(Criteria	
	Ti	me	Vo	lume	Major	Minor	
	Begin	End	Major	Minor	>= 420	>= 140	Both Meet
	12:00 AM	1:00 AM	49.55115	3.3784873	N	N	N
	1:00 AM	2:00 AM	49.55115	4.5046497	N	Ν	N
	2:00 AM	3:00 AM	41.66801	1.1261624	N	Ν	N
	3:00 AM	4:00 AM	78.83137	2.2523248	N	Ν	N
	4:00 AM	5:00 AM	211.7185	11.261624	N	Ν	N
	5:00 AM	6:00 AM	424.5632	13.513949	Y	Ν	N
	6:00 AM	7:00 AM	524.7917	16.892436	Y	Ν	N
	7:00 AM	8:00 AM	1348.361	71.66801	Y	Ν	N
	8:00 AM	9:00 AM	602.4969	38.289522	Y	Ν	N
	9:00 AM	10:00 AM	650.9219	45.046497	Y	Ν	N
	10:00 AM	11:00 AM	703.8515	52.929634	Y	Ν	N
	11:00 AM	12:00 PM	712.8608	51.803471	Y	Ν	Ν
	12:00 PM	1:00 PM	727.5009	63.065095	Y	Ν	N
	1:00 PM	2:00 PM	783.809	66.443583	Y	Ν	Ν
	2:00 PM	3:00 PM	927.9578	55.181959	Y	Ν	N
	3:00 PM	4:00 PM	1000.032	69.82207	Y	Ν	N
	4:00 PM	5:00 PM	1100.565	782.31742	Y	Y	Y
	5:00 PM	6:00 PM	945.9764	57.434283	Y	Ν	Ν
	6:00 PM	7:00 PM	873.902	30.406385	Y	Ν	Ν
	7:00 PM	8:00 PM	609.2539	24.775573	Y	Ν	Ν
	8:00 PM	9:00 PM	408.797	14.640111	Ν	Ν	Ν
	9:00 PM	10:00 PM	289.4237	18.018599	Ν	Ν	N
	10:00 PM	11:00 PM	207.2139	14.640111	Ν	Ν	N
	11:00 PM	12:00 AM	132.8872	15.766274	Ν	Ν	Ν
			Total numb	er of hours, b	oth the major	r(both	
			approaches	s) and minor(l	high volume a	approach) met:	1
Condition	A is not sat	isfied			I	Hours Required:	8
Warrant 1	not satisfie	d.					

Warrant 1: Eight- Hour Volumes Condition B

Numbe	r of Lanes		Major Both App	Street proaches	Minor Street High Volume Approach		
Major St	Street	Minor	Req	uired	Required		
	Sileei	Street	Urban	Rural*	Urban	Rural*	
1		1	750	525	75	53	
2 or more		1	900	630	75	53	
2 or more		2 or more	900	630	100	70	
1		2 or more	750	525	100	70	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 2						
				Cri	teria	
Т	ime	Vol	lume	Major	Minor	
Begin	End	Major	Minor	>= 630	>=70	Both Meet
12:00 AM	1:00 AM	49.55115	3.3784873	Ν	Ν	N
1:00 AM	2:00 AM	49.55115	4.5046497	Ν	Ν	N
2:00 AM	3:00 AM	41.66801	1.1261624	Ν	Ν	N
3:00 AM	4:00 AM	78.83137	2.2523248	Ν	Ν	N
4:00 AM	5:00 AM	211.7185	11.261624	Ν	Ν	N
5:00 AM	6:00 AM	424.5632	13.513949	Ν	Ν	N
6:00 AM	7:00 AM	524.7917	16.892436	Ν	Ν	N
7:00 AM	8:00 AM	1348.361	71.66801	Y	Y	Y
8:00 AM	9:00 AM	602.4969	38.289522	Ν	Ν	N
9:00 AM	10:00 AM	650.9219	45.046497	Y	Ν	N
10:00 AM	11:00 AM	703.8515	52.929634	Y	Ν	N
11:00 AM	12:00 PM	712.8608	51.803471	Y	Ν	N
12:00 PM	1:00 PM	727.5009	63.065095	Y	Ν	N
1:00 PM	2:00 PM	783.809	66.443583	Y	Ν	N
2:00 PM	3:00 PM	927.9578	55.181959	Y	Ν	N
3:00 PM	4:00 PM	1000.032	69.82207	Y	Ν	N
4:00 PM	5:00 PM	1100.565	782.31742	Y	Y	Y
5:00 PM	6:00 PM	945.9764	57.434283	Y	Ν	N
6:00 PM	7:00 PM	873.902	30.406385	Y	Ν	N
7:00 PM	8:00 PM	609.2539	24.775573	Ν	Ν	N
8:00 PM	9:00 PM	408.797	14.640111	Ν	Ν	N
9:00 PM	10:00 PM	289.4237	18.018599	Ν	Ν	N
10:00 PM	11:00 PM	207.2139	14.640111	Ν	Ν	N
11:00 PM	12:00 AM	132.8872	15.766274	Ν	Ν	N

Total number of hours, both the major(both approaches) and minor(high volume approach) met:

Hours Required:

2

8

Condition B is not satisfied Warrant 1 not satisfied.

Warrant 2: Four Hour Vehicular Volumes

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of an average day. The traffic volumes required are based on curves (Figure 4C-1) shown in the MUTCD.

* The required traffic is only present for one hour.

Warrant 2 is not satisfied

Warrant 3, Condition A- Peak Hour Delay

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - Peak Hour Volume

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is N/A.

Warrant 4: Pede	Varrant 4: Pedestrian Volume								
	Requ	uired* Existing							
100 or more fo 190 or more du	r each of O uring any	any four hours R one hour							
* For predomina 50 percent.	nt pedest	trian crossing speeds less than 3.5 ft/sec, the pedestrian volume may be reduced as much as							
Gap Requirem	ents								
YES YES	NO NO	Is the nearest signal located more than 300 feet away? For traffic flow which is not platooned, are there less than 60 gaps per hour of adequate length for the pedestrians to cross the street?							

Warrant 4 is N/A.

Warrant 5: S	chool Cro	ssing
YES	NO	Is the number of adequate gaps in traffic stream during the period when the children are using the crossing less than the number of minutes in the same period?
Warrant 5	is N/A.	
Warrant 6: C	oordinate	Systems
YES	NO	Are the adjacent signals in a signal system?
YES	NO	Would the resultant spacing be 1000 feet or more?
Warrant 6	is N/A.	
Warrant 7: C	rash Expe	rience
YES	NO	Is 80% or more of one of Warrants #1, #2, or #3 met?
YES	NO	Have there been more than five accidents susceptible to correction by a traffic signal in 12 months?

Warrant 7 is N/A.

Warrant 8: Roadway Network

YES	NO	Does the major street having an existing or immediately projected entering volume of > 1000 vehicles per hour of a typical weekday?
YES	NO	Do 5-year projected traffic volumes meet Warrants 1, 2, or 3?
YES	NO	Is there an entering traffic volume of at least 1000 vehicles per hour for each of any 5 hours on a Saturday or Sunday?
Warrant 8 i	s N/A.	

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none



Enter Traffic Volumes:

Automated Traffic Counts

Street: US 60

Location: Main Street City/State: Superior, AZ Project #: 20010 Date: 11/18/2016 Day of Week: Friday

Data Source: 24-hour approach



- Westbound

- Total Vehicles

24-Hour Volume: 14,393

T .	Eastbour	nd	Westbou	nd	T .	Eastbour	nd	Westbound	
Time	Vehicles	Peds	Vehicles	Peds	Time	Vehicles	Peds	Vehicles	Peds
12:00 AM					12:00 PM				
12:15 AM					12:15 PM				
12:30 AM					12:30 PM				
12:45 AM					12:45 PM				
1:00 AM	30		25		1:00 PM	375		428	
1:15 AM					1:15 PM				
1:30 AM					1:30 PM				
1:45 AM					1:45 PM				
2:00 AM	39		16		2:00 PM	399		466	
2:15 AM					2:15 PM				
2:30 AM					2:30 PM				
2:45 AM					2:45 PM				
3:00 AM	26		20		3:00 PM	464		561	
3:15 AM					3:15 PM				
3:30 AM					3:30 PM				
3:45 AM					3.45 PM				
4:00 AM	48		39		4:00 PM	504		601	
4:15 AM					4:15 PM	004		001	
4:30 AM					4:30 PM				
4:45 AM					4:45 PM				
5:00 AM	154		80		5:00 PM	540		772	
5:15 AM	134		00		5:15 PM	540		112	
5:20 AM					5:20 PM				
5.45 AM					5.45 DM				
5:45 AM	221		120		5:45 PM	515		520	
6:00 AM	331		130		6:15 PM	515		550	
6.20 AM					6.20 DM				
6:50 AM					6:30 PM				
0:43 AM	264		215		0:45 PM	502		462	
7:00 AM	504		213		7:00 PM	302		403	
7:13 AM					7:13 PM				
7:50 AM					7:50 PM				
/:45 AM	(0)		200		7:45 PM	420		224	
8:00 AM	093		290		8:00 PM	439		234	
8:15 AM					8:15 PM				
8:50 AM					8:30 PM				
0:43 AM	256		210		0:40 PM	270		170	
9:00 AM	330		510		9:00 PM	219		173	
9:15 AM					9:15 PM				
9:50 AM					9:50 PM				
9:43 AW	216		370		9:45 FIVI	224		06	
10.00 AW	340		513		10.00 FW	224		JU	
10:15 AM					10:15 PM				
10:50 AM					10:50 PM				
10:45 AM	405		272		10:45 PM	170		57	
11:00 AM	405		372	<u> </u>	11:00 PM	1/2		/د	
11:15 AM					11:15 PM				
11:30 AM					11:30 PM				
11:45 AM	407		000		11:45 PM	447		00	
12:00 PM	405		382		12:00 AM	117		30	
P						1,726	H V 1	6,667	
Equipment ID#:				l		24-	nour volume	14,393	

------ Eastbound

Street: M Location: U City/State: S Project #:	Main Street								
Location: U City/State: S Project #:	18 60			250					
City/State: S Project #:	500		Hour	200					_
Project #:	Superior AZ		se ber	150			/		
			ahicle	50					_
Date: 1	1/18/2016		5	0					P
Day of Week: F	Friday	h.		100 300	500 700 900	1100 1300 150	0 1700 1	.900 2100 2300	
Data Source. 2	4-nour approach					Time of Day			
4-Hour Volume:	1,015				Northbound		- tota	I venicles	
Time	Northbour	nd	Southbou	ind	Time	Northbou	nd	Southbou	nd
Thic	Vehicles	Peds	Vehicles	Peds	Thic	Vehicles	Peds	Vehicles	Ped
12:00 AM					12:00 PM				
12:15 AM					12:59 PM				
12:50 AM					12:30 PM				
1:00 AM			4		12.73 FW			70	
1:15 AM			r	<u>†</u> ──┤│	1:59 PM	1			
1:30 AM					1:30 PM				
1:45 AM					1:45 PM				
2:00 AM			5		2:00 PM			73	
2:15 AM					2:59 PM				
2:30 AM					2:30 PM				
2:45 AM					2:45 PM				
3:00 AM			1		3:00 PM			61	
3:15 AM					3:59 PM				
3:30 AM					3:30 PM				
3:45 AM			0		3:45 PM				
4:00 AM			2		4:00 PM			77	
4:15 AM					4:59 PM				
4:50 AM					4:50 PM				
5:00 AM			12		5:00 PM			228	
5:15 AM					5:59 PM				
5:30 AM					5:30 PM				
5:45 AM					5:45 PM				
6:00 AM			15		6:00 PM	ļ		63	
6:15 AM		Т			6:59 PM				
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM			19	├───┤ │	7:00 PM	+		34	
/:15 AM					7:59 PM				
/:30 AM					7:30 PM				
8:00 AM			46		8.00 PM			27	
8:15 AM			Ψ	┼───┤ │	8:59 PM	1		21	
8:30 AM					8:30 PM				
8:45 AM					8:45 PM				
9:00 AM			42		9:00 PM			16	
9:15 AM					9:59 PM				
9:30 AM					9:30 PM				
9:45 AM					9:45 PM				
10:00 AM			50	├ ───┤ │	10:00 PM			20	
10:15 AM					10:59 PM				
10:30 AM					10:30 PM				
10:45 AM			50		10:45 PM			16	
11:00 AM			58	┼───┤ │	11:00 PM	1		ю	
11:30 AM					11.37 PM				
11:45 AM					11:45 PM				
12:00 PM			57		12:00 AM			17	
				<u> </u>		0		1.015	

TRAFFIC SURVEY - COUNT ANALYSIS 2009 MUTCD WARRANTS

	2027 Wit	h Project					
	County:					District No.:	
	City:	Superior	Population:	4,000	_	Survey Date:	11/18/2016
	Route #	Name			Control	Section	85% Spee
Major		US 60					45
Minor		Main Stre	et			_	25

Warrant 1: Eight- Hour Volumes

Condition A

Numbe	r of Lanes		Major Both Ap	Street proaches	Minor Street High Volume Approact		
Major	Street	Minor	Req	uired	Required		
	Slieel	Street	Urban	Rural*	Urban	Rural*	
1		1	500	350	150	105	
2 or more		1	600	420	150	105	
2 or more		2 or more	600	420	200	140	
1		2 or more	500	350	200	140	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

	Warrant 1						
					(Criteria	
	Ti	me	Vol	lume	Major	Minor	
	Begin	End	Major	Minor	>= 420	>= 140	Both Meet
	12:00 AM	1:00 AM	54.70847	3.7301229	N	Ν	N
	1:00 AM	2:00 AM	54.70847	4.9734972	N	Ν	N
	2:00 AM	3:00 AM	46.00485	1.2433743	N	Ν	N
	3:00 AM	4:00 AM	87.0362	2.4867486	N	Ν	N
	4:00 AM	5:00 AM	233.7544	12.433743	N	Ν	N
	5:00 AM	6:00 AM	468.7521	14.920492	Y	Ν	N
	6:00 AM	7:00 AM	579.4124	18.650615	Y	Ν	N
	7:00 AM	8:00 AM	983.0149	46.004849	Y	Ν	N
	8:00 AM	9:00 AM	665.2053	42.274726	Y	Ν	N
	9:00 AM	10:00 AM	718.6704	49.734972	Y	Ν	N
	10:00 AM	11:00 AM	777.1089	58.438592	Y	Ν	N
	11:00 AM	12:00 PM	787.0559	57.195218	Y	Ν	N
	12:00 PM	1:00 PM	803.2198	69.628961	Y	Ν	N
	1:00 PM	2:00 PM	865.3885	73.359084	Y	Ν	N
	2:00 PM	3:00 PM	1024.54	60.925341	Y	Ν	N
	3:00 PM	4:00 PM	1104.116	77.089207	Y	Ν	N
	4:00 PM	5:00 PM	1311.931	228.11571	Y	Y	Y
	5:00 PM	6:00 PM	1044.434	63.41209	Y	Ν	N
	6:00 PM	7:00 PM	964.8585	33.571106	Y	Ν	N
	7:00 PM	8:00 PM	672.6655	27.354235	Y	Ν	N
	8:00 PM	9:00 PM	451.3449	16.163866	Y	Ν	N
	9:00 PM	10:00 PM	319.5472	19.893989	N	Ν	N
	10:00 PM	11:00 PM	228.7809	16.163866	N	Ν	N
	11:00 PM	12:00 AM	146.7182	17.40724	N	Ν	N
			Total numb	er of hours, b	oth the major	r(both	
			approaches	s) and minor(l	high volume a	approach) met:	1
Condition	A is not sat	isfied			I	Hours Required:	8
Warrant 1	not satisfie	d.					
Warrant 1: Eight- Hour Volumes Condition B

Number of Lanes			Major Both Apr	Street proaches	Minor Street High Volume Approach		
Major Street	Street	Minor	Req	uired	Required		
	Sileei	Street	Urban	Rural*	Urban	Rural*	
1		1	750	525	75	53	
2 or more		1	900	630	75	53	
2 or more		2 or more	900	630	100	70	
1		2 or more	750	525	100	70	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 2						
			Cri	teria		
Т	ime	Volume		Major	Minor	
Begin	End	Major	Minor	>= 630	>=70	Both Meet
12:00 AM	1:00 AM	54.70847	3.7301229	N	Ν	N
1:00 AM	2:00 AM	54.70847	4.9734972	Ν	Ν	N
2:00 AM	3:00 AM	46.00485	1.2433743	Ν	Ν	N
3:00 AM	4:00 AM	87.0362	2.4867486	Ν	Ν	N
4:00 AM	5:00 AM	233.7544	12.433743	Ν	Ν	N
5:00 AM	6:00 AM	468.7521	14.920492	Ν	Ν	N
6:00 AM	7:00 AM	579.4124	18.650615	Ν	Ν	N
7:00 AM	8:00 AM	983.0149	46.004849	Y	Ν	N
8:00 AM	9:00 AM	665.2053	42.274726	Y	Ν	N
9:00 AM	10:00 AM	718.6704	49.734972	Y	Ν	N
10:00 AM	11:00 AM	777.1089	58.438592	Y	Ν	N
11:00 AM	12:00 PM	787.0559	57.195218	Y	Ν	N
12:00 PM	1:00 PM	803.2198	69.628961	Y	Ν	N
1:00 PM	2:00 PM	865.3885	73.359084	Y	Y	Y
2:00 PM	3:00 PM	1024.54	60.925341	Y	Ν	N
3:00 PM	4:00 PM	1104.116	77.089207	Y	Y	Y
4:00 PM	5:00 PM	1311.931	228.11571	Y	Y	Y
5:00 PM	6:00 PM	1044.434	63.41209	Y	Ν	N
6:00 PM	7:00 PM	964.8585	33.571106	Y	Ν	N
7:00 PM	8:00 PM	672.6655	27.354235	Y	Ν	N
8:00 PM	9:00 PM	451.3449	16.163866	Ν	Ν	N
9:00 PM	10:00 PM	319.5472	19.893989	Ν	Ν	N
10:00 PM	11:00 PM	228.7809	16.163866	N	Ν	N
11:00 PM	12:00 AM	146.7182	17.40724	Ν	Ν	N

Total number of hours, both the major(both

approaches) and minor(high volume approach) met:

Hours Required:

3

8

Condition B is not satisfied *Warrant 1 not satisfied.*

Warrant 2: Four Hour Vehicular Volumes

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of an average day. The traffic volumes required are based on curves (Figure 4C-1) shown in the MUTCD.

* The required traffic is only present for one hour.

Warrant 2 is not satisfied

Warrant 3, Condition A- Peak Hour Delay

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - Peak Hour Volume

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is N/A.

Required* Existing 100 or more for each of any four hours
100 or more for each of any four hours
OR 190 or more during any one hour
* For predominant pedestrian crossing speeds less than 3.5 ft/sec, the pedestrian volume may be reduced as much a 50 percent.
Gap Requirements
YES NO Is the nearest signal located more than 300 feet away? YES NO For traffic flow which is not platooned, are there less than 60 gaps per hour of adequate length for the pedestrians to cross the street?

Warrant 4 is N/A.

Warrant 5: S	chool Cro	ssing			
YES NO Is the number of adequate gaps in traffic stream during the period when the childre the crossing less than the number of minutes in the same period?					
Warrant 5	is N/A.				
Warrant 6: C	oordinate	Systems			
YES	NO	Are the adjacent signals in a signal system?			
YES	NO	Would the resultant spacing be 1000 feet or more?			
Warrant 6	is N/A.				
Warrant 7: C	rash Expe	rience			
YES	NO	Is 80% or more of one of Warrants #1, #2, or #3 met?			
YES	NO	Have there been more than five accidents susceptible to correction by a traffic signal in 12 months?			

Warrant 7 is N/A.

Warrant 8: Roadway Network

YES	NO	Does the major street having an existing or immediately projected entering volume of > 1000 vehicles per hour of a typical weekday?
YES	NO	Do 5-year projected traffic volumes meet Warrants 1, 2, or 3?
YES	NO	Is there an entering traffic volume of at least 1000 vehicles per hour for each of any 5 hours on a Saturday or Sunday?
Warrant 8 i	s N/A.	

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none



Enter Traffic Volumes:



Equipment ID#:

24-Hour Volume

itomated Tra	affic Counts		,						
Street:	Silver King Mine US 60	e Road	four	12 10					
City/State:	Superior, AZ		les per H	8 6					
Project #:	•		ehicl	4					
Date:	11/18/2016		>	0					10
Day of Week:	Friday			100 300	500 700 900	1100 1300 150	0 1700 ⁻	900 2100 2300	
Data Source:	24-hour approac	h				Time of Day			
4-Hour Volume:	152				Northbound	Southbound		Vehicles	
Time	Northbou Vehicles	nd Peds	Southbou Vehicles	nd Peds	Time	Northbou Vehicles	nd Peds	Southbou Vehicles	ind Pedi
12:00 AM					12:00 PM				
12:15 AM					12:59 PM				
12:30 AM					12:30 PM				
12:45 AM					12:45 PM				
1:00 AM	0		0		1:00 PM	4		7	
1:15 AM					1:59 PM				
1:30 AM					1:30 PM				
1:45 AM	<u>,</u>		0		1:45 PM			<u> </u>	
2:00 AM	U		U	┨────┤	2:00 PM	4		6	
2:15 AM 2:30 AM					2:39 PM				
2:50 AM					2:50 PM				
3:00 AM	0		0		2:45 PM	3		6	
3:15 AM	0		Ŭ		3:59 PM	Ŭ		Ū	
3:30 AM					3:30 PM				
3:45 AM					3:45 PM				
4:00 AM	0		1		4:00 PM	4		6	
4:15 AM					4:59 PM				
4:30 AM					4:30 PM				
4:45 AM					12:00 AM				
5:00 AM	1		1		5:00 PM	4		7	
5:15 AM					5:59 PM				
5:30 AM					5:30 PM				
5:45 AM	2		4		5:45 PM	2		6	
6:00 AM	2		4		6:50 PM	3		0	
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM	2		4		7:00 PM	4		7	
7:15 AM					7:59 PM				
7:30 AM					7:30 PM				
7:45 AM					7:45 PM				
8:00 AM	3		5		8:00 PM	3		6	
8:15 AM					8:59 PM				
8:30 AM					8:30 PM				
8:45 AM	<u>,</u>		-		8:45 PM			<u> </u>	
9:00 AM	3		5		9:00 PM	2		3	
9:15 AM					9:59 PM				
9:50 AM					9:50 PM				
10:00 AM	3		5		10:00 PM	2		3	
10:15 AM	5		~	1 1	10:59 PM	<u> </u>		5	<u> </u>
10:30 AM					10:30 PM				
10:45 AM					10:45 PM				
11:00 AM	3		5		11:00 PM	1		2	
11:15 AM				1	11:59 PM				
11:30 AM					11:30 PM				
11:45 AM					11:45 PM				
12:00 PM	3		6		12:00 AM	0		1	
						54		07	

TRAFFIC SURVEY - COUNT ANALYSIS 2009 MUTCD WARRANTS

	2020 With	out Project						
	County:					District No.:		
	City: Superior		Population:	4,000	_	Survey Date:	e: 11/18/2016	
	Route #	Name			Control	Section	85% Speed	
Major		US 60					65	
Minor		Silver Kin	g Mine Road				25	

Warrant 1: Eight- Hour Volumes Condition A

Number of Lanes			Major Both Ap	Street proaches	Minor Street High Volume Approach			
Major Street	Ctract	Minor	Minor Required			Required		
	Street	Street	Urban	Rural*	Urban	Rural*		
1		1	500	350	150	105		
2 or more		1	600	420	150	105		
2 or more		2 or more	600	420	200	140		
1		2 or more	500	350	200	140		
						10.000		

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

	Warrant 1						
						Criteria	
	Ti	me	Volume		Major	Minor	
	Begin	End	Major	Minor	>= 420	>= 105	Both Meet
	12:00 AM	1:00 AM	14.07162	0.1194905	N	N	N
	1:00 AM	2:00 AM	16.23648	0.2219109	N	Ν	N
	2:00 AM	3:00 AM	23.81351	0.3243313	N	Ν	N
	3:00 AM	4:00 AM	41.13242	0.5291722	N	Ν	N
	4:00 AM	5:00 AM	96.33646	1.3485356	N	Ν	N
	5:00 AM	6:00 AM	256.5364	3.9090462	N	Ν	N
	6:00 AM	7:00 AM	379.9337	4.4894286	N	Ν	N
	7:00 AM	8:00 AM	479.5174	4.7966899	Y	Ν	N
	8:00 AM	9:00 AM	623.4809	4.9673906	Y	Ν	N
	9:00 AM	10:00 AM	660.2836	4.8308301	Y	Ν	N
	10:00 AM	11:00 AM	747.9606	5.2575818	Y	Ν	N
	11:00 AM	12:00 PM	756.6201	5.9062445	Y	Ν	N
	12:00 PM	1:00 PM	890.8417	6.6231875	Y	Ν	N
	1:00 PM	2:00 PM	1023.981	6.4183466	Y	Ν	N
	2:00 PM	3:00 PM	980.6835	5.9062445	Y	Ν	N
	3:00 PM	4:00 PM	1085.679	6.4866269	Y	Ν	N
	4:00 PM	5:00 PM	1074.855	7.1694298	Y	Ν	N
	5:00 PM	6:00 PM	817.2363	6.1964357	Y	Ν	N
	6:00 PM	7:00 PM	815.0714	7.1011495	Y	Ν	N
	7:00 PM	8:00 PM	607.2444	5.547773	Y	Ν	N
	8:00 PM	9:00 PM	425.3958	3.4993645	Y	Ν	N
	9:00 PM	10:00 PM	358.285	2.8677719	N	Ν	N
	10:00 PM	11:00 PM	247.877	2.1166888	N	Ν	N
	11:00 PM	12:00 AM	104.9959	0.7852233	N	N	N
			Total numb	er of hours, b	ooth the major(l	ooth	
			approaches	s) and minor(high volume ap	proach) met:	0
Condition	A is not sat	tisfied			Ho	ours Required:	8
Warrant 1	arrant 1 not satisfied.						

Warrant 1: Eight- Hour Volumes Condition B

Number of Lanes			Major Both App	Street proaches	Minor Street High Volume Approach		
Major Street	Street	Minor	Req	uired	Required		
	Slieel	Street	Urban	Rural*	Urban	Rural*	
1		1	750	525	75	53	
2 or more		1	900	630	75	53	
2 or more		2 or more	900	630	100	70	
1		2 or more	750	525	100	70	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 2						
				Cri	teria	
T	ime	Vol	ume	Major	Minor	
Begin	End	Major	Minor	>= 630	> = 53	Both Meet
12:00 AM	1:00 AM	14.07162	0.1194905	N	Ν	N
1:00 AM	2:00 AM	16.23648	0.2219109	Ν	Ν	N
2:00 AM	3:00 AM	23.81351	0.3243313	Ν	Ν	N
3:00 AM	4:00 AM	41.13242	0.5291722	Ν	Ν	N
4:00 AM	5:00 AM	96.33646	1.3485356	Ν	Ν	Ν
5:00 AM	6:00 AM	256.5364	3.9090462	Ν	Ν	Ν
6:00 AM	7:00 AM	379.9337	4.4894286	Ν	Ν	Ν
7:00 AM	8:00 AM	479.5174	4.7966899	Ν	Ν	Ν
8:00 AM	9:00 AM	623.4809	4.9673906	Ν	Ν	Ν
9:00 AM	10:00 AM	660.2836	4.8308301	Y	Ν	Ν
10:00 AM	11:00 AM	747.9606	5.2575818	Y	Ν	Ν
11:00 AM	12:00 PM	756.6201	5.9062445	Y	Ν	Ν
12:00 PM	1:00 PM	890.8417	6.6231875	Y	Ν	Ν
1:00 PM	2:00 PM	1023.981	6.4183466	Y	Ν	Ν
2:00 PM	3:00 PM	980.6835	5.9062445	Y	Ν	Ν
3:00 PM	4:00 PM	1085.679	6.4866269	Y	Ν	Ν
4:00 PM	5:00 PM	1074.855	7.1694298	Y	Ν	N
5:00 PM	6:00 PM	817.2363	6.1964357	Y	Ν	Ν
6:00 PM	7:00 PM	815.0714	7.1011495	Y	Ν	Ν
7:00 PM	8:00 PM	607.2444	5.547773	Ν	Ν	Ν
8:00 PM	9:00 PM	425.3958	3.4993645	Ν	Ν	N
9:00 PM	10:00 PM	358.285	2.8677719	Ν	Ν	N
10:00 PM	11:00 PM	247.877	2.1166888	Ν	Ν	N
11:00 PM	12:00 AM	104.9959	0.7852233	N	N	N

Total number of hours, both the major(both

approaches) and minor(high volume approach) met:

0 Hours Required: 8

Condition B is not satisfied Warrant 1 not satisfied.

Warrant 2: Four Hour Vehicular Volumes

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of an average day. The traffic volumes required are based on curves (Figure 4C-1) shown in the MUTCD.

* The required traffic volumes for Warrant 2 do not meet for any one hour.

Warrant 2 is not satisfied

Warrant 3, Condition A- Peak Hour Delay

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehiclehours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - Peak Hour Volume

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This w is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is N/A.

Warrant	4: Pedestrian	Volume	
	Red	quired*	Existing
100 or	more for each o	of any four hours	
		OR	
190 or	more during an	y one hour	
* For pre as 50	edominant pede percent.	strian crossing speeds	\mathfrak{s} less than 3.5 ft/sec, the pedestrian volume may be reduced as much
Gap R	equirements		
YE	S NO	Is the nearest signa	al located more than 300 feet away?

YES NO For traffic flow which is not platooned, are there less than 60 gaps per hour of adequate length for the pedestrians to cross the street?

Warrant 4 is N/A.

Warrant 5: School Crossing

YES NO Is the number of adequate gaps in traffic stream during the period when the children are using the crossing less than the number of minutes in the same period?

Warrant 5 is N/A.

Warrant 6: Co	ordinate	e Systems
YES YES	NO NO	Are the adjacent signals in a signal system? Would the resultant spacing be 1000 feet or more?
Warrant 6 is	s N/A.	
Warrant 7: Cr	ash Exp	erience
YES	NO	Is 80% or more of one of Warrants #1, #2, or #3 met?
YES	NO	Have there been more than five accidents susceptible to correction by a traffic signal in 12 months?
Warrant 7 is	s N/A.	
Warrant 8: Ro	adway N	letwork
YES	NO	Does the major street having an existing or immediately projected entering volume of > 1000 vehicles per hour of a typical weekday?
YES	NO	Do 5-year projected traffic volumes meet Warrants 1, 2, or 3?
YES	NO	Is there an entering traffic volume of at least 1000 vehicles per hour for each of any 5 hours on a Saturday or Sunday?
Warrant 8 is	s N/A.	

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none

TRAFFIC SIGNAL WARRANT EVALUATION

This review is based on the methodology presented in the 2009 revision of the Manual on Uniform Traffic Control Devices (MUTCD), Please refer to Chapter 4C of the current manual.

Warrant 1, Condition A- MINIMUM VEHICULAR VOLUME

The installation of a traffic signal may be necessary to control an intersection with large volumes of conflicting traffic. The required traffic volumes must be present for at least 8 hours (not an average) of an average day. The minimum volumes vary according to the number of lanes on the intersecting streets, the speed of the traffic on the main street, and the community size.

Number of Lanes		Major Str	Major Street - Both Approaches			t - High Volur	ne Approach	Major & Minor
Number of Lanes							Street Volumes	
Major	Minor	Required Total H		Total Hours	Required		Total Hours	Total Hours
Street	Street	Urban Rural* Satisfied		Satisfied	Urban	Rural*	Satisfied	Both Satisfied
1	1	500	350	-	150	105	-	-
2 or more	1	600	420	14	150	105	0	8
2 or more	2 or more	600	420	-	200	140	-	-
1	2 or more	500	350	-	200	140	-	-

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 1- Condition A is not satisfied

Warrant 1, Condition B - INTERRUPTION OF CONTINUOUS TRAFFIC

On major streets with high traffic volume, it may be necessary to use traffic signal control to provide an adequate number of gaps in traffic to allow vehicles to enter from a side street. The required traffic volumes must be present for at least 8 hours (not an average) of an average day. The minimum volumes vary according to the number of lanes on the intersecting streets, the speed of the traffic on the main street, and the community size.

Number of Lanes		Major Str	eet - Both Ap	proaches	Minor Stree	t - High Volur	Major & Minor Street Volumes	
Major	Minor	Required		Total Hours	Required		Total Hours	Total Hours
Street	Street	Urban Rural* Satisfied		Urban	Rural*	Satisfied	Both Satisfied	
1	1	750	525	-	75	53	-	-
2 or more	1	900	630	10	75	53	0	0
2 or more	2 or more	900	630	-	100	70	-	-
1	2 or more	750	525	-	100	70	-	-

Warrant 1- Condition B is not satisfied

To satisfy Warrant 1, either Condition A or B must be must be satisfied. Warrant 1 is not satisfied

Warrant 2 - FOUR HOUR VOLUMES

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of an average day. The traffic volumes required are based on curves (Figure 4C-2) shown in the MUTCD.

* These traffic volumes are not known.

Warrant 2 is not satisfied

Warrant 3, Condition A- PEAK HOUR DELAY

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - PEAK HOUR VOLUME

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This warrant is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is not applicable.

Warrant 4 - MINIMUM PEDESTRIAN VOLUME

This warrant is similar to Warrant 2, but is intended to identify locations where additional gaps are needed to provide safe pedestrian crossing of a major street. A signal installed solely for pedestrians should use a fully actuated controller and, if in a signal system, be coordinated with that system. A signal installed only under this warrant shall include pedestrian signals. When installed at a midblock location, additional restrictions may apply (See section 4C-5):

100 or more for each of fours hours; or 190 or more during any one hour.

The necessary pedestrian volumes are not present or have not been collected.

Warrant 4 is not applicable.

Warrant 5 - SCHOOL CROSSING

An established school crossing may require signal protection if an engineering study reveals that there is less than one adequate gap per minute during the period of crossing usage. The restrictions on signals installed under this warrant are similar to those of Warrant 3, above.

Warrant 5 is not applicable.

Warrant 6 - Coordinate Systems

A traffic signal may occasionally be used to maintain vehicle grouping in a coordinated system. Such a signal should not be within 1,000 ft of adjacent signalized intersections in the system.

Warrant 6 is not applicable.

Warrant 7 - CRASH EXPERIENCE

Many traffic signals are installed on the premise of reducing accidents; however, it must be recognized that signals may actually increase some types of accidents. The result is often contrary to the intended goal. Four conditions must be met before a signal is installed solely to reduce accidents:

- (1) less restrictive solutions have been tried and enforced with unsatisfactory results.
- (2) There has been five or more accidents of types preventable by traffic signals (i.e. right angle) in the last 12 months;
- (3) volume of vehicular and pedstrian traffic not less than 80% of Warrants 1, 2, or 3.
- (4) traffic progression would not be seriously disrupted, and

A signal installed solely under this warrant should be traffic actuated.

* The number of preventable accidents in the past 12 months is le

* None of the Warrant 7 volume requirements are met.

Warrant 7 is not applicable.

Warrant 8 - ROADWAY NETWORK

Traffic signal control may be used to encourage concentration and organization of vehicles on the major street networks. Such a signal may be installed at the intersection of two major street routes as defined by section 4C-9 of the TMUTCD if the intersection has a total existing, or immediately projected, entering volume of at least 1000 vehicles:

1) during the peak hour of a typical weekday and has five year projected volumes, based on an engineering study, which meet one or more of Warrant 1, 2, and 3 during an average weekday; or

2) for each of any five hours of a Saturday and/or Sunday.

Warrant 8 is not applicable.

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none

85th % speed: > 40 mph Population: < 10,000

Major Street Lanes: 2 Minor Street Lanes: 1

Use Figure: 4C-2 2&1

Pank	Major Street	Minor Street		Figure 4C-1			Figure 4C-2	2
Rallk	Volume	Volume	1&1	2&1	2&2	1&1	2&1	2&2
1	104.9959195	0.78522326	-	-	-	-	N	-
2	14.07161808	0.1194905	-	-	-	-	N	-
3	16.2364824	0.22191092	-	-	-	-	N	-
4	23.81350752	0.32433135	-	-	-	-	N	-
5	41.13242208	0.5291722	-	-	-	-	N	-
6	96.33646224	1.3485356	-	-	-	-	N	-
7	256.5364219	3.90904623	-	-	-	-	N	-
8	379.9336882	4.48942864	-	-	-	-	N	-
9	479.5174469	4.79668991	-	-	-	-	N	-
10	623.4809242	4.96739062	-	-	-	-	N	-
11	660.2836176	4.83083005	-	-	-	-	N	-
12	747.9606226	5.25758182	-	-	-	-	N	-
13	756.6200798	5.90624452	-	-	-	-	N	-
14	890.8416677	6.62318749	-	-	-	-	N	-
15	1023.980823	6.41834664	-	-	-	-	N	-
16	980.683537	5.90624452	-	-	-	-	N	-
17	1085.679456	6.48662693	-	-	-	-	N	-
18	1074.855135	7.16942976	-	-	-	-	N	-
19	817.2362808	6.19643572	-	-	-	-	N	-
20	815.0714165	7.10114948	-	-	-	-	N	-
21	607.2444418	5.54777303	-	-	-	-	N	-
22	425.3958389	3.49936453	-	-	-	-	N	-
23	358.285045	2.8677719	-	-	-	-	Ν	-
24	247.8769646	2.11668879	-	-	-	-	Ν	-
		0	0	0	0	0	0	
Warrant 2 is not	satisfied.		Ν	Ν	Ν	Ν	Ν	Ν

Warrant 2 Figure 4C-2 Four Hour Volume Warrant (population<10,000 or >40 mph on major street)



* If data point is outside graph boundaries, it is plotted at the maximum shown value(s).

Major Street-Total of Both Approaches-vph



Enter Traffic Volumes:

Automated Traffic Counts 1,200 Street: US 60 Vehicles per Hour 1,000 Location: Silver King Mine Road 800 600 City/State: Superior, AZ 400 Project #: 20010 200 Date: 11/18/2016 0 [300 500 100 700 900 1100 1300 1500 1700 1900 2100 2300 Day of Week: Friday Time of Day Data Source: 24-hour approach ------------------------Eastbound 24-Hour Volume: 13,034 Eastbound Westbound Eastbound Westbound Time Time Vehicles Peds Vehicles Peds Vehicles Peds Vehicles Peds 12:00 AM 12:00 PM 12:15 AM 12:15 PM 12:30 AM 12:30 PM 12:45 AM 12:45 PM 1:00 AM 8 7 1:00 PM 437 490 1:15 AM 1:15 PM 1:30 AM 1:30 PM 1:45 AM 1:45 PM 2:00 AM 15 2 2:00 PM 423 642 2:15 AM 2:15 PM 2:30 PM 2:30 AM 2:45 AM 2:45 PM 3:00 AM 21 3 3:00 PM 390 631 3:15 AM 3:15 PM 3:30 AM 3:30 PM 3:45 AM 3:45 PM 4:00 AM 35 8 4:00 PM 428 702 4:15 AM 4:15 PM 4:30 AM 4:30 PM 4:45 AM 4:45 PM 5:00 AM 89 11 5:00 PM 473 645 5:15 AM 5:15 PM 5:30 AM 5:30 PM 5:45 PM 5:45 AM 258 9 409 6:00 AM 6:00 PM 441 6:15 PM 6:15 AM 6:30 AM 6:30 PM 6:45 AM 6:45 PM 99 380 7:00 AM 296 7:00 PM 468 7:15 AM 7:15 PM 7:30 AM 7:30 PM 7:45 AM 7:45 PM 8:00 AM 316 182 8:00 PM 366 266

Equipment ID#:				24-Hour Volu	me 13,034	
				6,427	6,607	
12:00 PM	390	398	12:00 AM	52	57	
11:45 AM			11:45 PM			
11:30 AM			11:30 PM			
11:15 AM			11:15 PM			
11:00 AM	347	431	11:00 PM	140	118	
10:45 AM			10:45 PM			
10:30 AM			10:30 PM			
10:15 AM			10:15 PM			
10:00 AM	319	368	10:00 PM	189	184	
9:45 AM			9:45 PM			
9:30 AM			9:30 PM			
9:15 AM			9:15 PM			
9:00 AM	328	321	9:00 PM	231	212	
8:45 AM			8:45 PM			
8:30 AM			8:30 PM			
8:15 AM			8:15 PM			

C 4	Silver Vin - M	o Dood		14					
Street: Location:	US 60	e Koad	ar Hour	14 12 10					
City/State:	Superior, AZ		cles pe	8					
Project #:	11/10/2017		Vehi	2					
Date:	11/18/2010 Emidou			0	500 700 900	1100 1300 150	0 1700	1900 2100 2300	
Day of week. Data Source:	24-hour approa	ch				Time of Day			
-Hour Volume:	158	1			Northbound	-B Southbound	—≜— Tota	al Vehicles	
Time	Northbo	und	Southbou	ind	Time	Northbou	nd	Southbou	ind
12.00.434	Vehicles	Peds	Vehicles	Peds	12.00 PM	Vehicles	Peds	Vehicles	Pe
12:00 AM					12:00 PM				
12:15 AM					12:59 PM				
12:30 AM					12:30 PM				
12:43 AM	0		0		12:43 PM	4		7	
1.00 AW	U		0		1.00 PWI	4		/	
1.13 ΑΨ 1.30 ΔΜ					1.39 FWI 1.30 DM				
1:45 AM					1.30 PM				
2:00 AM	0		0		2:00 PM	4		7	
2:15 AM			.		2:59 PM			· ·	1
2:30 AM					2:30 PM				
2:45 AM					2:45 PM				
3:00 AM	0		0		3:00 PM	3		6	
3:15 AM					3:59 PM				
3:30 AM					3:30 PM				
3:45 AM					3:45 PM				
4:00 AM	0		1		4:00 PM	4		7	
4:15 AM					4:59 PM				
4:30 AM					4:30 PM				
4:45 AM					12:00 AM				
5:00 AM	1		1		5:00 PM	4		7	
5:15 AM					5:59 PM				
5:30 AM					5:30 PM				
5:45 AM	-				5:45 PM				
6:00 AM	2		4		6:00 PM	4		6	
6:15 AM					6:59 PM				
6:30 AM					6:30 PM				
6:45 AM 7:00 AM	2		Б		0:45 PM 7:00 PM	1		7	
7:15 AM	5		5		7:59 PM	4		1	
7:30 AM					7:30 PM				
7:45 AM					7:45 PM				
8:00 AM	3		5		8:00 PM	3		6	
8:15 AM					8:59 PM				
8:30 AM					8:30 PM				
8:45 AM					8:45 PM				
9:00 AM	3		5		9:00 PM	2		4	
9:15 AM					9:59 PM				
9:30 AM					9:30 PM				
9:45 AM					9:45 PM				
10:00 AM	3		5		10:00 PM	2		3	
10:15 AM					10:59 PM				
10:30 AM					10:30 PM				
10:45 AM					10:45 PM				
11:00 AM	3	<u> </u>	5		11:00 PM	1		2	
11:15 AM					11:59 PM				
11:30 AM					11:30 PM				
11:45 AM	<u>^</u>		<u>^</u>		11:45 PM				
12:00 PM	3		6	1 1	12:00 AM	0		I 1	1

TRAFFIC SURVEY - COUNT ANALYSIS

2009 MUTCD WARRANTS

	2022 Without Project							
	County:			4,000	District No.:			
	City:	Superior	Population:			Survey Date:	11/18/2016	
	Route #	Name			Control	Section	85% Speed	
Major		US 60					65	
Minor		Silver Kin	g Mine Road				25	

Warrant 1: Eight- Hour Volumes

Condition A

Number of Lanes			Major Both Ap	Street proaches	Minor Street High Volume Approact		
Major Stree	Street	Minor	Req	uired	Requ	iired	
	Slieel	Street	Urban	Rural*	Urban	Rural*	
	1	1	500	350	150	105	
2 or	more	1	600	420	150	105	
2 or more		2 or more	600	420	200	140	
1		2 or more	500	350	200	140	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

	Warrant 1						
					Cr	iteria	
	Ti	me	Vo	lume	Major	Minor	
	Begin	End	Major	Minor	>= 420	>= 105	Both Meet
	12:00 AM	1:00 AM	14.64011	0.1243179	N	N	N
	1:00 AM	2:00 AM	16.89244	0.2308761	Ν	Ν	N
	2:00 AM	3:00 AM	24.77557	0.3374343	Ν	Ν	N
	3:00 AM	4:00 AM	42.79417	0.5505508	N	Ν	N
	4:00 AM	5:00 AM	100.2285	1.4030164	N	Ν	N
	5:00 AM	6:00 AM	266.9005	4.0669717	N	Ν	N
	6:00 AM	7:00 AM	395.283	4.6708016	N	Ν	N
	7:00 AM	8:00 AM	498.89	4.9904762	Y	Ν	N
	8:00 AM	9:00 AM	648.6696	5.1680732	Y	Ν	N
	9:00 AM	10:00 AM	686.9591	5.0259956	Y	Ν	N
	10:00 AM	11:00 AM	778.1782	5.4699881	Y	Ν	N
	11:00 AM	12:00 PM	787.1875	6.1448568	Y	Ν	N
	12:00 PM	1:00 PM	926.8317	6.8907643	Y	Ν	N
	1:00 PM	2:00 PM	1065.35	6.6776478	Y	Ν	N
	2:00 PM	3:00 PM	1020.303	6.1448568	Y	Ν	N
	3:00 PM	4:00 PM	1129.541	6.7486867	Y	Ν	N
	4:00 PM	5:00 PM	1118.279	7.4590747	Y	Ν	N
	5:00 PM	6:00 PM	850.2526	6.4467717	Y	Ν	N
	6:00 PM	7:00 PM	848.0003	7.3880359	Y	Ν	N
	7:00 PM	8:00 PM	631.7771	5.7719031	Y	Ν	N
	8:00 PM	9:00 PM	442.5818	3.6407389	Y	Ν	N
	9:00 PM	10:00 PM	372.7598	2.9836299	Ν	Ν	N
	10:00 PM	11:00 PM	257.8912	2.202203	N	Ν	N
	11:00 PM	12:00 AM	109.2378	0.8169463	Ν	N	N
			Total numb	er of hours, b	oth the major(l	ooth	
			approaches	s) and minor(high volume ap	proach) met:	0
Condition	A is not sat	isfied			H	ours Required:	8
Warrant 1	not satisfie	d.					

Warrant 1: Eight- Hour Volumes Condition B

Number of Lanes			Major Both Apr	Street proaches	Minor Street High Volume Approach		
Major Stree	Street	Minor	Req	uired	Required		
	Slieel	Street	Urban	Rural*	Urban	Rural*	
1		1	750	525	75	53	
2 or more		1	900	630	75	53	
2 or more		2 or more	900	630	100	70	
1		2 or more	750	525	100	70	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 2						
				Cri	teria	
Т	ime	Vol	lume	Major	Minor	
Begin	End	Major	Minor	>= 630	> = 53	Both Meet
12:00 AM	1:00 AM	14.64011	0.1243179	Ν	Ν	N
1:00 AM	2:00 AM	16.89244	0.2308761	Ν	Ν	N
2:00 AM	3:00 AM	24.77557	0.3374343	Ν	Ν	N
3:00 AM	4:00 AM	42.79417	0.5505508	Ν	Ν	N
4:00 AM	5:00 AM	100.2285	1.4030164	Ν	Ν	N
5:00 AM	6:00 AM	266.9005	4.0669717	Ν	Ν	N
6:00 AM	7:00 AM	395.283	4.6708016	Ν	Ν	N
7:00 AM	8:00 AM	498.89	4.9904762	Ν	Ν	N
8:00 AM	9:00 AM	648.6696	5.1680732	Y	Ν	N
9:00 AM	10:00 AM	686.9591	5.0259956	Y	Ν	N
10:00 AM	11:00 AM	778.1782	5.4699881	Y	Ν	N
11:00 AM	12:00 PM	787.1875	6.1448568	Y	Ν	N
12:00 PM	1:00 PM	926.8317	6.8907643	Y	Ν	N
1:00 PM	2:00 PM	1065.35	6.6776478	Y	Ν	N
2:00 PM	3:00 PM	1020.303	6.1448568	Y	Ν	N
3:00 PM	4:00 PM	1129.541	6.7486867	Y	Ν	N
4:00 PM	5:00 PM	1118.279	7.4590747	Y	Ν	N
5:00 PM	6:00 PM	850.2526	6.4467717	Y	Ν	N
6:00 PM	7:00 PM	848.0003	7.3880359	Y	Ν	N
7:00 PM	8:00 PM	631.7771	5.7719031	Y	Ν	N
8:00 PM	9:00 PM	442.5818	3.6407389	Ν	Ν	N
9:00 PM	10:00 PM	372.7598	2.9836299	Ν	Ν	N
10:00 PM	11:00 PM	257.8912	2.202203	Ν	Ν	N
11:00 PM	12:00 AM	109.2378	0.8169463	Ν	Ν	N

Total number of hours, both the major(both approaches) and minor(high volume approach) met:

0

8

Hours Required:

Condition B is not satisfied Warrant 1 not satisfied.

Warrant 2: Four Hour Vehicular Volumes

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of an average day. The traffic volumes required are based on curves (Figure 4C-1) shown in the MUTCD.

* The required traffic volumes for Warrant 2 do not meet for any one hour.

Warrant 2 is not satisfied

Warrant 3, Condition A- Peak Hour Delay

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - Peak Hour Volume

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This wa is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is N/A.

Warrant 4: Po	edestrian '	Volume	
	Re	equired* Existing	
100 or more 190 or more	e for each o e during an	of any four hours OR ny one hour	
* For predom 50 percent.	inant pede	estrian crossing speeds less than 3.5 ft/sec, the pedestrian volume may be reduced as	much as
Gap Requir	ements		
YES YES	NO NO	Is the nearest signal located more than 300 feet away? For traffic flow which is not platooned, are there less than 60 gaps per hour of adeq length for the pedestrians to cross the street?	uate
Warrant 4	is N/A.		

Warrant 5: School Crossing

YES NO Is the number of adequate gaps in traffic stream during the period when the children are using the crossing less than the number of minutes in the same period?

Warrant 5 is N/A.

Warrant 6: C	oordinate	Systems	
YES	NO	Are the adjacent signals in a signal system?	
YES	NO	Would the resultant spacing be 1000 feet or more?	
Warrant 6	is N/A.		

Warrant 7: Crash Experience

YES	NO	Is 80% or more of one of Warrants #1, #2, or #3 met?
YES	NO	Have there been more than five accidents susceptible to correction by a traffic signal in 12
		months?

Warrant 7 is N/A.

Warrant 8: Roadway Network

YES	NO	Does the major street having an existing or immediately projected entering volume of > 1000 vehicles per hour of a typical weekday?
YES	NO	Do 5-year projected traffic volumes meet Warrants 1, 2, or 3?
YES	NO	Is there an entering traffic volume of at least 1000 vehicles per hour for each of any 5 hours on a Saturday or Sunday?
Warrant 8	is N/A.	

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none

85th % speed: > 40 mph Population: < 10,000

Major Street Lanes: 2 Minor Street Lanes: 1

Use Figure: 4C-2 2&1

Donk	Major Street	Minor Street		Figure 4C-1			Figure 4C-2	2
Rank	Volume	Volume	1&1	2&1	2&2	1&1	2&1	2&2
1	109.2377547	0.81694628	-	-	-	-	N	-
2	14.64011145	0.12431791	-	-	-	-	N	-
3	16.89243629	0.23087612	-	-	-	-	N	-
4	24.77557322	0.33743433	-	-	-	-	N	-
5	42.79417193	0.55055075	-	-	-	-	N	-
6	100.2284553	1.40301644	-	-	-	-	N	-
7	266.9004934	4.06697169	-	-	-	-	N	-
8	395.2830092	4.67080155	-	-	-	-	N	-
9	498.8899517	4.99047618	-	-	-	-	N	-
10	648.6695535	5.1680732	-	-	-	-	N	-
11	686.9590758	5.02599559	-	-	-	-	N	-
12	778.1782317	5.46998813	-	-	-	-	N	-
13	787.1875311	6.1448568	-	-	-	-	N	-
14	926.8316711	6.89076427	-	-	-	-	N	-
15	1065.349649	6.67764785	-	-	-	-	N	-
16	1020.303152	6.1448568	-	-	-	-	N	-
17	1129.540907	6.74868665	-	-	-	-	N	-
18	1118.279282	7.45907472	-	-	-	-	N	-
19	850.2526265	6.44677173	-	-	-	-	N	-
20	848.0003017	7.38803592	-	-	-	-	N	-
21	631.7771172	5.77190306	-	-	-	-	N	-
22	442.5818308	3.64073885	-	-	-	-	N	-
23	372.7597608	2.98362989	-	-	-	-	N	-
24	257.891194	2.20220301	-	-	-	-	N	-
			0	0	0	0	0	0
Warrant 2 is not	satisfied.		Ν	N	N	Ν	N	Ν

Warrant 2 Figure 4C-2 Four Hour Volume Warrant (population<10,000 or >40 mph on major street)



it is plotted at the maximum shown value(s).

Major Street-Total of Both Approaches-vph



Enter Traffic Volumes:

Automated Traffic Counts 1,400 Street: US 60 1,200 Vehicles per Hour Location: Silver King Mine Road 1,000 800 600 City/State: Superior, AZ 400 Project #: 20010 200 Date: 11/18/2016 0 300 100 500 700 900 1100 1300 1500 1700 1900 2100 2300 Day of Week: Friday Time of Day Data Source: 24-hour approach ------ Eastbound ------ Total Vehicles 14,391 24-Hour Volume: Eastbound Westbound Eastbound Westbound Time Time Vehicles Peds Vehicles Peds Vehicles Peds Vehicles Peds 12:00 AM 12:00 PM 12:15 AM 12:15 PM 12:30 AM 12:30 PM 12:45 AM 12:45 PM 1:00 AM 9 7 1:00 PM 482 541 1:15 AM 1:15 PM 1:30 AM 1:30 PM 1:45 AM 1:45 PM 2:00 AM 16 2 2:00 PM 468 709 2:15 AM 2:15 PM 2:30 PM 2:30 AM 2:45 AM 2:45 PM 3:00 AM 24 4 3:00 PM 430 696 3:15 AM 3:15 PM 3:30 AM 3:30 PM 3:45 AM 3:45 PM 4:00 AM 39 9 4:00 PM 472 775 4:15 AM 4:15 PM 4:30 AM 4:30 PM 4:45 AM 4:45 PM 5:00 AM 98 12 5:00 PM 522 712 5:15 AM 5:15 PM 5:30 AM 5:30 PM 5:45 PM 5:45 AM 285 10 6:00 AM 6:00 PM 451 487 6:15 PM 6:15 AM 6:30 AM 6:30 PM 6:45 AM 6:45 PM 327 419 7:00 AM 109 7:00 PM 517 7:15 AM 7:15 PM 7:30 AM 7:30 PM 7:45 AM 7:45 PM 8:00 PM 293 8:00 AM 349 201 404

Equipment ID#:				24-Hour Volu	me 14,391	
				7,096	7,295	
12:00 PM	430	439	12:00 AM	57	63	
11:45 AM			11:45 PM			
11:30 AM			11:30 PM			
11:15 AM			11:15 PM			
11:00 AM	383	476	11:00 PM	154	131	
10:45 AM			10:45 PM			
10:30 AM			10:30 PM			
10:15 AM			10:15 PM			
10:00 AM	352	407	10:00 PM	209	203	
9:45 AM			9:45 PM			
9:30 AM			9:30 PM			
9:15 AM			9:15 PM			
9:00 AM	362	354	9:00 PM	255	234	
8:45 AM			8:45 PM			
8:30 AM			8:30 PM			
8:15 AM			8:15 PM			

Straat	Silver King Min	e Ruad		14					,
Location:	US 60	e Nudu	r Hour	12 10					
City/State:	Superior, AZ		cles pe						
Project #:	11/10/001/		Vehi	2					5
Date:	11/18/2010 Friday			0 0 300	500 700 900	1100 1300 150	0 1700	1900 2100 2300	
Day of week. Data Source:	24-hour approa	ch				Time of Day			
Llour Volumou	174	1			Northbound		à Tota	al Vehicles	
-Hour volume:	1/4 Northbo	ind	Southbor	ind		Northbou	nd	Southbou	ind
Time	Vehicles	Peds	Vehicles	Peds	Time	Vehicles	Peds	Vehicles	Pe
12:00 AM					12:00 PM				
12:15 AM					12:59 PM				
12:30 AM					12:30 PM				
12:45 AM					12:45 PM				
1:00 AM	0		0		1:00 PM	4		8	
1:15 AM					1:59 PM				
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2:45 AM					2:45 PM				
3:00 AM	0		0		3:00 PM	4		7	
3:15 AM					3:59 PM				
3:30 AM					3:30 PM				
3:45 AM					3:45 PM				
4:00 AM	0		1		4:00 PM	4		7	
4:15 AM					4:59 PM				
4:30 AM					4:30 PM				
4:45 AM	_				12:00 AM	_			
5:00 AM	1		2		5:00 PM	5		8	
5:15 AM					5:59 PM				
5:30 AM					5:30 PM				
5:45 AM	2		4		5:45 PM	4		7	
6:15 AM	2		4		6:50 PM	4		1	
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM	3		5		7:00 PM	5		8	
7:15 AM	0		0		7:59 PM	0		0	
7:30 AM					7:30 PM				
7:45 AM					7:45 PM				
8:00 AM	3		6		8:00 PM	4		6	
8:15 AM			-		8:59 PM				
8:30 AM					8:30 PM				
8:45 AM					8:45 PM				
9:00 AM	3		6		9:00 PM	2		4	
9:15 AM					9:59 PM				
9:30 AM					9:30 PM				
9:45 AM					9:45 PM				
10:00 AM	3		6		10:00 PM	2		3	
10:15 AM					10:59 PM				
10:30 AM					10:30 PM				
10:45 AM					10:45 PM				
11:00 AM	3		6		11:00 PM	11		2	
11:15 AM					11:59 PM				
11:30 AM					11:30 PM				
11:45 AM					11:45 PM				
10.00 DM	4	1	7	1 1	12.00 414	1		1	1

TRAFFIC SURVEY - COUNT ANALYSIS

2009 MUTCD WARRANTS

	2027 With	out Project					
	County:					District No.:	
	City:	Superior	Population:	4,000	_	Survey Date:	11/18/2016
	Route #	Name			Control	Section	85% Speed
Major		US 60					65
Minor		Silver Kin	g Mine Road				25

Warrant 1: Eight- Hour Volumes

Condition A

Number of Lanes			Major Both Ap	Street proaches	Minor Street High Volume Approac		
Maian Otraat		Minor	Req	uired	Requ	ired	
Major	Slieel	Street	Urban	Rural*	Urban	Rural*	
	1	1	500	350	150	105	
2 or	more	1	600	420	150	105	
2 or	more	2 or more	600	420	200	140	
	1	2 or more	500	350	200	140	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

	Warrant 1						
					Cri	teria	
	Ti	me	Vol	lume	Major	Minor	
	Begin	End	Major	Minor	>= 420	>= 105	Both Meet
	12:00 AM	1:00 AM	16.16387	0.137257	N	Ν	N
	1:00 AM	2:00 AM	18.65061	0.2549059	N	Ν	N
	2:00 AM	3:00 AM	27.35423	0.3725548	N	Ν	N
	3:00 AM	4:00 AM	47.24822	0.6078525	N	Ν	N
	4:00 AM	5:00 AM	110.6603	1.5490435	N	Ν	N
	5:00 AM	6:00 AM	294.6797	4.4902654	N	Ν	N
	6:00 AM	7:00 AM	436.4244	5.1569423	Y	Ν	N
	7:00 AM	8:00 AM	550.8148	5.509889	Y	Ν	N
	8:00 AM	9:00 AM	716.1836	5.7059704	Y	Ν	N
	9:00 AM	10:00 AM	758.4583	5.5491052	Y	Ν	N
	10:00 AM	11:00 AM	859.1716	6.0393089	Y	N	N
	11:00 AM	12:00 PM	869.1186	6.7844184	Y	N	N
	12:00 PM	1:00 PM	1023.297	7.6079605	Y	N	N
	1:00 PM	2:00 PM	1176.232	7.3726628	Y	N	N
	2:00 PM	3:00 PM	1126.497	6.7844184	Y	N	N
	3:00 PM	4:00 PM	1247.104	7.4510954	Y	N	N
	4:00 PM	5:00 PM	1234.671	8.2354212	Y	N	N
	5:00 PM	6:00 PM	938.7476	7.1177569	Y	N	N
	6:00 PM	7:00 PM	936.2609	8.1569886	Y	N	N
	7:00 PM	8:00 PM	697.533	6.3726474	Y	N	N
	8:00 PM	9:00 PM	488.6461	4.0196699	Y	N	N
	9:00 PM	10:00 PM	411.5569	3.2941685	N	N	N
	10:00 PM	11:00 PM	284.7327	2.4314101	N	N	N
	11:00 PM	12:00 AM	120.6073	0.9019747	N	N	N
			Total numb	er of hours, b	oth the major(b	ooth	
			approaches	s) and minor(high volume ap	proach) met:	0
Condition	A is not sat	isfied			Ho	ours Required:	8

Warrant 1 not satisfied.

Warrant 1: Eight- Hour Volumes Condition B

Number of Lanes			Major Both Apr	Street proaches	Minor Street High Volume Approach		
Major Street	Minor	Req	uired	Requ	lired		
	Slieel	Street	Urban	Rural*	Urban	Rural*	
	1	1	750	525	75	53	
2 o	r more	1	900	630	75	53	
2 o	r more	2 or more	900	630	100	70	
1		2 or more	750	525	100	70	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 2									
	Criteria								
Т	ime	Vol	lume	Major	Minor				
Begin	End	Major	Minor	>= 630	> = 53	Both Meet			
12:00 AM	1:00 AM	16.16387	0.137257	Ν	Ν	N			
1:00 AM	2:00 AM	18.65061	0.2549059	Ν	Ν	N			
2:00 AM	3:00 AM	27.35423	0.3725548	Ν	Ν	N			
3:00 AM	4:00 AM	47.24822	0.6078525	Ν	Ν	N			
4:00 AM	5:00 AM	110.6603	1.5490435	Ν	Ν	N			
5:00 AM	6:00 AM	294.6797	4.4902654	Ν	Ν	N			
6:00 AM	7:00 AM	436.4244	5.1569423	Ν	Ν	N			
7:00 AM	8:00 AM	550.8148	5.509889	Ν	Ν	N			
8:00 AM	9:00 AM	716.1836	5.7059704	Y	Ν	N			
9:00 AM	10:00 AM	758.4583	5.5491052	Y	Ν	N			
10:00 AM	11:00 AM	859.1716	6.0393089	Y	Ν	N			
11:00 AM	12:00 PM	869.1186	6.7844184	Y	Ν	N			
12:00 PM	1:00 PM	1023.297	7.6079605	Y	Ν	N			
1:00 PM	2:00 PM	1176.232	7.3726628	Y	Ν	N			
2:00 PM	3:00 PM	1126.497	6.7844184	Y	Ν	N			
3:00 PM	4:00 PM	1247.104	7.4510954	Y	Ν	N			
4:00 PM	5:00 PM	1234.671	8.2354212	Y	Ν	N			
5:00 PM	6:00 PM	938.7476	7.1177569	Y	Ν	N			
6:00 PM	7:00 PM	936.2609	8.1569886	Y	Ν	N			
7:00 PM	8:00 PM	697.533	6.3726474	Y	Ν	N			
8:00 PM	9:00 PM	488.6461	4.0196699	N	Ν	N			
9:00 PM	10:00 PM	411.5569	3.2941685	Ν	Ν	N			
10:00 PM	11:00 PM	284.7327	2.4314101	Ν	Ν	N			
11:00 PM	12:00 AM	120.6073	0.9019747	Ν	Ν	N			

Total number of hours, both the major(both approaches) and minor(high volume approach) met:

0

8

Hours Required:

Condition B is not satisfied Warrant 1 not satisfied.

Warrant 2: Four Hour Vehicular Volumes

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of an average day. The traffic volumes required are based on curves (Figure 4C-1) shown in the MUTCD.

* The required traffic volumes for Warrant 2 do not meet for any one hour.

Warrant 2 is not satisfied

Warrant 3, Condition A- Peak Hour Delay

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - Peak Hour Volume

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This wa is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is N/A.

Warrant 4: Po	edestrian '	Volume	
	Re	equired* Existing	
100 or more 190 or more	e for each o e during an	of any four hours OR ny one hour	
* For predom 50 percent.	inant pede	estrian crossing speeds less than 3.5 ft/sec, the pedestrian volume may be reduced as	much as
Gap Requir	ements		
YES YES	NO NO	Is the nearest signal located more than 300 feet away? For traffic flow which is not platooned, are there less than 60 gaps per hour of adeq length for the pedestrians to cross the street?	uate
Warrant 4	is N/A.		

Warrant 5: School Crossing

YES NO Is the number of adequate gaps in traffic stream during the period when the children are using the crossing less than the number of minutes in the same period?

Warrant 5 is N/A.

Warrant 6: C	oordinate	Systems	
YES	NO	Are the adjacent signals in a signal system?	
YES	NO	Would the resultant spacing be 1000 feet or more?	
Warrant 6	is N/A.		

Warrant 7: Crash Experience

YES	NO	Is 80% or more of one of Warrants #1, #2, or #3 met?
YES	NO	Have there been more than five accidents susceptible to correction by a traffic signal in 12
		months?

Warrant 7 is N/A.

Warrant 8: Roadway Network

YES	NO	Does the major street having an existing or immediately projected entering volume of > 1000 vehicles per hour of a typical weekday?
YES	NO	Do 5-year projected traffic volumes meet Warrants 1, 2, or 3?
YES	NO	Is there an entering traffic volume of at least 1000 vehicles per hour for each of any 5 hours on a Saturday or Sunday?
Warrant 8	is N/A.	

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none

85th % speed: > 40 mph Population: < 10,000

Major Street Lanes: 2 Minor Street Lanes: 1

Use Figure: 4C-2 2&1

Book	Major Street	Minor Street		Figure 4C-1			-2	
Ralik	Volume	Volume	1&1	2&1	2&2	1&1	2&1	2&2
1	120.6073079	0.9019747	-	-	-	-	N	-
2	16.16386601	0.13725702	-	-	-	-	N	-
3	18.65061463	0.25490589	-	-	-	-	N	-
4	27.35423478	0.37255477	-	-	-	-	N	-
5	47.24822372	0.60785252	-	-	-	-	N	-
6	110.6603134	1.54904351	-	-	-	-	N	-
7	294.6797111	4.49026538	-	-	-	-	N	-
8	436.4243822	5.15694233	-	-	-	-	N	-
9	550.8148186	5.50988895	-	-	-	-	N	-
10	716.1836016	5.70597041	-	-	-	-	N	-
11	758.4583281	5.54910525	-	-	-	-	N	-
12	859.1716471	6.03930889	-	-	-	-	N	-
13	869.1186416	6.78441843	-	-	-	-	N	-
14	1023.297056	7.60796055	-	-	-	-	N	-
15	1176.232096	7.3726628	-	-	-	-	N	-
16	1126.497123	6.78441843	-	-	-	-	N	-
17	1247.104431	7.45109538	-	-	-	-	N	-
18	1234.670688	8.23542121	-	-	-	-	N	-
19	938.7476028	7.1177569	-	-	-	-	N	-
20	936.2608542	8.15698863	-	-	-	-	N	-
21	697.532987	6.37264737	-	-	-	-	N	-
22	488.6461032	4.01966988	-	-	-	-	N	-
23	411.5568961	3.29416848	-	-	-	-	N	-
24	284.7327166	2.43141007	-	-	-	-	Ν	-
			0	0	0	0	0	0
Warrant 2 is not satisfied.			Ν	Ν	Ν	Ν	Ν	Ν

Warrant 2 Figure 4C-2 Four Hour Volume Warrant (population<10,000 or >40 mph on major street)



it is plotted at the maximum shown value(s).

Major Street-Total of Both Approaches-vph



Enter Traffic Volumes:

								
Street: U Location: S	US 60 Silver King Mine F	coad H Soad	2,500					
City/State: S Project #: 2	Superior, AZ 20010	Vehicles p	1,000					
Date: 1	1/18/2016	-	0					
Day of Week: I Data Source: 2	Friday 24-hour approach		100 300	500 700 900	1100 1300 150 Time of Day	00 1700	1900 2100 2300	D
4-Hour Volume:	24,954		Eastbound		Total Vehicles			
Time	Eastbound Vehicles	We Peds Vehicles	stbound Peds	Time	Eastbour Vehicles	nd Peds	Westbour Vehicles	nd Ped
12:00 AM	· · · · · · · · · · · · · · · · · · ·			12:00 PM		T Cub		100
12:15 AM				12:15 PM				
12:30 AM				12:30 PM				
12:45 AM				12:45 PM				
1:00 AM	15	13		1:00 PM	854		918	
1:15 AM	-			1:15 PM		1		
1:30 AM				1:30 PM				
1:45 AM				1:45 PM				
2:00 AM	29	4		2:00 PM	828		1203	
2:15 AM				2:15 PM				
2:30 AM				2:30 PM				
2:45 AM				2:45 PM				
3:00 AM	42	6		3:00 PM	762		1182	
3:15 AM				3:15 PM				
3:30 AM				3:30 PM				
3:45 AM				3:45 PM				
4:00 AM	68	15		4:00 PM	837		1315	
4:15 AM				4:15 PM				
4:30 AM				4:30 PM				
4:45 AM				4:45 PM				
5:00 AM	174	21		5:00 PM	925		1210	
5:15 AM				5:15 PM				
5:30 AM				5:30 PM				
5:45 AM				5:45 PM				
6:00 AM	504	17		6:00 PM	799		828	
6:15 AM				6:15 PM				
6:30 AM				6:30 PM				
6:45 AM				6:45 PM				
7:00 AM	579	186		7:00 PM	916		712	
7:15 AM				7:15 PM				
7:30 AM				7:30 PM				
7:45 AM				7:45 PM				
8:00 AM	619	342		8:00 PM	716		498	
8:15 AM				8:15 PM				
8:30 AM				8:30 PM				
8:45 AM				8:45 PM				
9:00 AM	641	602		9:00 PM	451		397	
9:15 AM				9:15 PM				
9:30 AM				9:30 PM				
9:45 AM				9:45 PM				
10:00 AM	623	690		10:00 PM	370		344	
10:15 AM				10:15 PM				
10:30 AM				10:30 PM				
10:45 AM				10:45 PM				
11:00 AM	678	809		11:00 PM	273		222	
11:15 AM				11:15 PM				
11:30 AM				11:30 PM				
11:45 AM				11:45 PM				
12.00 DM	762	745		12:00 AM	101		108	
12:00 PM								

C 4. 4	Cilvon Vin - Mr	Dead		60					
Street: Location:	: Sliver King Mine · US 60	e Koad	'n	50					
Location.			ber Hc	40					
City/State: Superior, AZ Project #:				30					
				10					
Date:	: 11/18/2016 : Friday			0	500 700 900	1100 1300 150	0 1700	1900 2100 2300	
Data Source:	24-hour approac	h				Time of Day			
-Hour Volume:	678			_	Northbound		—_≜— Tota	al Vehicles	
Time	Northbour	nd	Southbound		Time	Northbound		Southbound	
Time	Vehicles	Peds	Vehicles	Peds	Time	Vehicles	Peds	Vehicles	Р
12:00 AM					12:00 PM				
12:15 AM					12:59 PM				
12:30 AM					12:30 PM				
1:00 AM	0		1		1:00 PM	4		42	
1:15 AM			1	+	1:59 PM	т 		12	
1:30 AM					1:30 PM				
1:45 AM					1:45 PM				
2:00 AM	0		1		2:00 PM	4		41	
2:15 AM					2:59 PM				
2:30 AM					2:30 PM				
2:45 AM			2		2:45 PM				
3:00 AM	0		2		3:00 PM	3		38	
3:15 AM 3:30 AM					3:39 PM 3:30 PM				
3:45 AM					3:45 PM				
4:00 AM	0		3		4:00 PM	4		41	
4:15 AM					4:59 PM				
4:30 AM					4:30 PM				
4:45 AM					12:00 AM				
5:00 AM	1		9		5:00 PM	4		46	
5:15 AM					5:59 PM				
5:30 AM					5:30 PM				
5:43 AM 6:00 AM	2		25		5:45 PM 6:00 PM	4		40	
6:15 AM	-		20		6:59 PM			10	
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM	3		29		7:00 PM	4		45	
7:15 AM					7:59 PM				
7:30 AM					7:30 PM				
/:45 AM 8:00 AM	, s		21		7:45 PM	2		25	
8:15 AM	3		31	+	8.59 PM	3			
8:30 AM					8:30 PM				
8:45 AM					8:45 PM				
9:00 AM	3		32		9:00 PM	2		22	
9:15 AM					9:59 PM				
9:30 AM					9:30 PM				
9:45 AM					9:45 PM	_			
10:00 AM	3		31	+	10:00 PM	2		18	
10:15 AM					10:59 PM				
10.50 AM 10:45 AM					10:30 PM 10:45 PM				
11:00 AM	3		34		11:00 PM	1		14	
11:15 AM			ντ	+	11:59 PM				1
11:30 AM					11:30 PM				
11:45 AM					11:45 PM				
10 00 DM	3		20	1	12:00 AM	0		5	1
TRAFFIC SURVEY - COUNT ANALYSIS 20

009	MUT	CD	WAI	RRA	NTS

	2022 Wit	h Project					
	County:					District No.:	
	City:	Superior	Population:	4,000	_	Survey Date:	11/18/2016
	Route #	Name			Control	Section	85% Speed
Major		US 60					65
Minor		Silver Kin	g Mine Road				25

Warrant 1: Eight- Hour Volumes

Condition A

Number of Lanes		Major Both Ap	Street proaches	Minor Street High Volume Approach		
Maiar	Chroot	Minor	Req	uired	Requ	lired
Major	Sireei	Street	Urban	Rural*	Urban	Rural*
	1	1	500	350	150	105
2 or	more	1	600	420	150	105
2 or	more	2 or more	600	420	200	140
	1	2 or more	500	350	200	140

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

	Warrant 1						
					Cr	iteria	
	Ti	me	Vo	lume	Major	Minor	
	Begin	End	Major	Minor	>= 420	>= 105	Both Meet
	12:00 AM	1:00 AM	28.08224	0.7621311	N	Ν	N
	1:00 AM	2:00 AM	32.84911	1.4153864	N	Ν	N
	2:00 AM	3:00 AM	48.17265	2.0686416	N	Ν	N
	3:00 AM	4:00 AM	83.04239	3.3751521	N	Ν	N
	4:00 AM	5:00 AM	195.074	8.6011941	N	Ν	N
	5:00 AM	6:00 AM	521.1565	24.932575	Y	Ν	N
	6:00 AM	7:00 AM	764.9323	28.634355	Y	Ν	N
	7:00 AM	8:00 AM	960.8077	30.594121	Y	Ν	N
	8:00 AM	9:00 AM	1242.522	31.68288	Y	Ν	N
	9:00 AM	10:00 AM	1313.582	30.811873	Y	Ν	N
	10:00 AM	11:00 AM	1486.867	33.533769	Y	Ν	N
	11:00 AM	12:00 PM	1507.205	37.671053	Y	Ν	N
	12:00 PM	1:00 PM	1772.819	42.243839	Y	Ν	N
	1:00 PM	2:00 PM	2031.425	40.937329	Y	Ν	N
	2:00 PM	3:00 PM	1944.251	37.671053	Y	Ν	N
	3:00 PM	4:00 PM	2152.134	41.372832	Y	Ν	N
	4:00 PM	5:00 PM	2134.649	45.727867	Y	Ν	N
	5:00 PM	6:00 PM	1626.981	39.521943	Y	Ν	N
	6:00 PM	7:00 PM	1627.566	45.292364	Y	Ν	N
	7:00 PM	8:00 PM	1213.936	35.384659	Y	Ν	N
	8:00 PM	9:00 PM	848.3477	22.319554	Y	Ν	N
	9:00 PM	10:00 PM	714.0891	18.291147	Y	Ν	N
	10:00 PM	11:00 PM	494.7423	13.500608	Y	Ν	N
	11:00 PM	12:00 AM	208.9716	5.0082902	N	N	N
			Total numb	er of hours, b	oth the major(ooth	
			approaches	s) and minor(high volume ap	proach) met:	0
Condition	A is not sat	isfied			Ho	ours Required:	8
Warrant 1	not satisfie	d.					

Warrant 1: Eight- Hour Volumes Condition B

Numbe	er of Lanes		Major Both Apr	Street proaches	Minor High Volum	Street e Approach		
Major Street	Street	Minor	Req	uired	Requ	Required		
	Slieel	Street	Urban	Rural*	Urban	Rural*		
	1	1	750	525	75	53		
2 or more		1	900	630	75	53		
2 o	r more	2 or more	900	630	100	70		
	1	2 or more	750	525	100	70		

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 2						
				Cri	iteria	
Т	ime	Vol	lume	Major	Minor	
Begin	End	Major	Minor	>= 630	> = 53	Both Meet
12:00 AM	1:00 AM	28.08224	0.7621311	Ν	Ν	N
1:00 AM	2:00 AM	32.84911	1.4153864	Ν	Ν	N
2:00 AM	3:00 AM	48.17265	2.0686416	Ν	Ν	N
3:00 AM	4:00 AM	83.04239	3.3751521	Ν	Ν	N
4:00 AM	5:00 AM	195.074	8.6011941	Ν	Ν	N
5:00 AM	6:00 AM	521.1565	24.932575	Ν	Ν	N
6:00 AM	7:00 AM	764.9323	28.634355	Y	Ν	N
7:00 AM	8:00 AM	960.8077	30.594121	Y	Ν	N
8:00 AM	9:00 AM	1242.522	31.68288	Y	Ν	N
9:00 AM	10:00 AM	1313.582	30.811873	Y	Ν	N
10:00 AM	11:00 AM	1486.867	33.533769	Y	Ν	N
11:00 AM	12:00 PM	1507.205	37.671053	Y	Ν	N
12:00 PM	1:00 PM	1772.819	42.243839	Y	Ν	N
1:00 PM	2:00 PM	2031.425	40.937329	Y	Ν	N
2:00 PM	3:00 PM	1944.251	37.671053	Y	Ν	N
3:00 PM	4:00 PM	2152.134	41.372832	Y	Ν	N
4:00 PM	5:00 PM	2134.649	45.727867	Y	Ν	N
5:00 PM	6:00 PM	1626.981	39.521943	Y	Ν	N
6:00 PM	7:00 PM	1627.566	45.292364	Y	Ν	N
7:00 PM	8:00 PM	1213.936	35.384659	Y	Ν	N
8:00 PM	9:00 PM	848.3477	22.319554	Y	Ν	N
9:00 PM	10:00 PM	714.0891	18.291147	Y	Ν	N
10:00 PM	11:00 PM	494.7423	13.500608	Ν	Ν	N
11:00 PM	12:00 AM	208.9716	5.0082902	Ν	Ν	N

Total number of hours, both the major(both approaches) and minor(high volume approach) met:

0

8

Hours Required:

Condition B is not satisfied Warrant 1 not satisfied.

Warrant 2: Four Hour Vehicular Volumes

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of an average day. The traffic volumes required are based on curves (Figure 4C-1) shown in the MUTCD.

* The required traffic volumes for Warrant 2 do not meet for any one hour.

Warrant 2 is not satisfied

Warrant 3, Condition A- Peak Hour Delay

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - Peak Hour Volume

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This wa is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is N/A.

Warrant 4: Po	edestrian '	Volume	
	Re	equired* Existing	
100 or more 190 or more	e for each o e during an	of any four hours OR ny one hour	
* For predom 50 percent.	inant pede	estrian crossing speeds less than 3.5 ft/sec, the pedestrian volume may be reduced as	much as
Gap Requir	ements		
YES YES	NO NO	Is the nearest signal located more than 300 feet away? For traffic flow which is not platooned, are there less than 60 gaps per hour of adeq length for the pedestrians to cross the street?	uate
Warrant 4	is N/A.		

Warrant 5: School Crossing

YES NO Is the number of adequate gaps in traffic stream during the period when the children are using the crossing less than the number of minutes in the same period?

Warrant 5 is N/A.

Warrant 6: C	oordinate	Systems	
YES	NO	Are the adjacent signals in a signal system?	
YES	NO	Would the resultant spacing be 1000 feet or more?	
Warrant 6	is N/A.		

Warrant 7: Crash Experience

YES	NO	Is 80% or more of one of Warrants #1, #2, or #3 met?
YES	NO	Have there been more than five accidents susceptible to correction by a traffic signal in 12
		months?

Warrant 7 is N/A.

Warrant 8: Roadway Network

YES	NO	Does the major street having an existing or immediately projected entering volume of > 1000 vehicles per hour of a typical weekday?
YES	NO	Do 5-year projected traffic volumes meet Warrants 1, 2, or 3?
YES	NO	Is there an entering traffic volume of at least 1000 vehicles per hour for each of any 5 hours on a Saturday or Sunday?
Warrant 8	is N/A.	

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none

85th % speed: > 40 mph Population: < 10,000

Major Street Lanes: 2 Minor Street Lanes: 1

Use Figure: 4C-2 2&1

Donk	Major Street	Minor Street	t Figure 4C-1				Figure 4C-2		
Rank	Volume	Volume	1&1	2&1	2&2	1&1	2&1	2&2	
1	208.9715907	5.00829024	-	-	-	-	N	-	
2	28.08224139	0.76213112	-	-	-	-	N	-	
3	32.84911146	1.41538637	-	-	-	-	N	-	
4	48.17265011	2.06864162	-	-	-	-	N	-	
5	83.04239484	3.37515212	-	-	-	-	N	-	
6	195.0740355	8.60119411	-	-	-	-	N	-	
7	521.1564914	24.9325753	-	-	-	-	N	-	
8	764.93235	28.6343551	-	-	-	-	N	-	
9	960.8076598	30.5941208	-	-	-	-	N	-	
10	1242.522048	31.6828796	-	-	-	-	N	-	
11	1313.581791	30.8118726	-	-	-	-	N	-	
12	1486.867333	33.5337695	-	-	-	-	N	-	
13	1507.20462	37.6710527	-	-	-	-	N	-	
14	1772.819419	42.2438394	-	-	-	-	N	-	
15	2031.425061	40.9373289	-	-	-	-	N	-	
16	1944.250699	37.6710527	-	-	-	-	N	-	
17	2152.133884	41.3728324	-	-	-	-	N	-	
18	2134.648565	45.7278674	-	-	-	-	N	-	
19	1626.981215	39.5219426	-	-	-	-	N	-	
20	1627.565671	45.2923639	-	-	-	-	N	-	
21	1213.935858	35.3846593	-	-	-	-	N	-	
22	848.3477406	22.3195543	-	-	-	-	N	-	
23	714.0891339	18.291147	-	-	-	-	N	-	
24	494.7422755	13.5006085	-	-	-	-	Ν	-	
			0	0	0	0	0	0	
Warrant 2 is not	satisfied.		Ν	Ν	N	Ν	Ν	Ν	

Warrant 2 Figure 4C-2 Four Hour Volume Warrant (population<10,000 or >40 mph on major street)



* If data point is outside graph boundaries, it is plotted at the maximum shown value(s).

Major Street-Total of Both Approaches-vph



Enter Traffic Volumes:

Automated Traffic Counts $\begin{array}{c} 2,000\\ 1,800\\ 1,600\\ 1,400\\ 1,200\\ 1,000\\ 800\\ 600\\ 400\\ 200 \end{array}$ Street: US 60 Vehicles per Hour Location: Silver King Mine Road City/State: Superior, AZ Project #: 20010 Date: 11/18/2016 0 300 100 500 700 900 1100 1300 1500 1700 1900 2100 2300 Day of Week: Friday Time of Day Data Source: 24-hour approach ------ Eastbound Total Vehicles 24-Hour Volume: 20,281 Eastbound Westbound Eastbound Westbound Time Time Vehicles Peds Vehicles Peds Vehicles Peds Vehicles Peds 12:00 AM 12:00 PM 12:15 AM 12:15 PM 12:30 AM 12:30 PM 12:45 AM 12:45 PM 1:00 AM 12 10 1:00 PM 683 759 1:15 AM 1:15 PM 1:30 AM 1:30 PM 1:45 AM 1:45 PM 2:00 AM 23 3 2:00 PM 662 994 2:15 PM 2:15 AM 2:30 PM 2:30 AM 2:45 AM 2:45 PM 3:00 AM 33 5 3:00 PM 609 977 3:15 AM 3:15 PM 3:30 AM 3:30 PM 3:45 AM 3:45 PM 4:00 AM 55 12 4:00 PM 669 1087 4:15 AM 4:15 PM 4:30 AM 4:30 PM 4:45 AM 4:45 PM 5:00 AM 139 17 5:00 PM 739 1000 5:15 AM 5:15 PM 5:30 AM 5:30 PM 5:45 PM 5:45 AM 403 6:00 AM 14 6:00 PM 639 684 6:15 PM 6:15 AM 6:30 AM 6:30 PM 6:45 AM 6:45 PM 154 7:00 PM 588 7:00 AM 463 732 7:15 AM 7:15 PM 7:30 AM 7:30 PM 7:45 AM 7:45 PM 572 8:00 AM 495 283 8:00 PM 412

Equipment ID#:				24-Hour	Volume 20,281
				10,046	10,235
12:00 PM	609	616	12:00 AM	81	89
11:45 AM			11:45 PM		
11:30 AM			11:30 PM		
11:15 AM			11:15 PM		
11:00 AM	542	668	11:00 PM	218	183
10:45 AM			10:45 PM		
10:30 AM			10:30 PM		
10:15 AM			10:15 PM		
10:00 AM	498	570	10:00 PM	296	284
9:45 AM			9:45 PM		
9:30 AM			9:30 PM		
9:15 AM			9:15 PM		
9:00 AM	512	497	9:00 PM	361	328
8:45 AM			8:45 PM		
8:30 AM			8:30 PM		
8:15 AM			8:15 PM		

Ctract	Silvon Kina Mi	Dood		10					
Location:	US 60	e Road	r Hour	18 16 14 12					
City/State: Project #:	Superior, AZ		hicles pe						
Date:	11/18/2016		Vel	4					
Day of Week:	Fridav			100 300	500 700 900	1100 1300 150	0 1700 [·]	1900 2100 2300	-¥
Data Source:	24-hour approad	ch				Time of Day			
-Hour Volume:	224				Northbound	-B- Southbound	—≗— Tota	Il Vehicles	
Time	Northbou	ind	Southbou	ind	Time	Northbou	nd	Southbou	Ind
	Vehicles	Peds	Vehicles	Peds		Vehicles	Peds	Vehicles	Pe
12:00 AM					12:00 PM				
12:15 AM					12:59 PM				
12:30 AM					12:30 PM				
12:45 AM	0		0		12:45 PM	4		44	
1:00 AM	U	╂───┤	U	┼───┤	1:00 PM	4		11	
1:15 AIVI					1:39 PM				
1.30 AIVI 1.45 AM					1:50 PM				
1:43 AM	0		0		1:43 PM 2:00 PM	4		11	
2.00 AIVI	U	+ +	U	<u> </u>	2.00 PW	4			
2.13 AM					2.39 FM				
2:30 AM					2:45 PM				
2.45 AM	0		1		2.45 FM	А		10	
3:15 AM	0	1 1	1		3:50 PM			10	
3:30 AM					3.30 PM				
3:45 AM					3:45 PM				
4:00 AM	0		1		4.00 PM	4		11	
4:15 AM	Ŭ		•		4:59 PM				
4:30 AM					4:30 PM				
4:45 AM					12:00 AM				
5:00 AM	1		2		5:00 PM	5		12	
5:15 AM					5:59 PM				
5:30 AM					5:30 PM				
5:45 AM					5:45 PM				
6:00 AM	2		6		6:00 PM	4		10	
6:15 AM					6:59 PM				
6:30 AM					6:30 PM				
6:45 AM					6:45 PM				
7:00 AM	3		7		7:00 PM	5		12	
7:15 AM					7:59 PM				
7:30 AM					7:30 PM				
7:45 AM	-		<u>,</u>		7:45 PM				
8:00 AM	3	├ ───┤	8	<u> </u>	8:00 PM	4		9	
8:15 AM					8:59 PM				
8:30 AM					8:30 PM				
8:45 AM	2		0		8:45 PM	0		<u> </u>	
9:00 AM	3	+ +	8	<u> </u>	9:00 PM	2		0	
9.13 AIVI 9.20 AM					9:39 PM 0.20 DM				
9.30 AIVI 9.45 AM					9.50 PWI 0.45 DM				
10.00 AM	3		R		10.00 PM	2		5	
10:15 AM	5	┼ ┤	5		10.50 PM	<u> </u>			
10:30 AM					10.37 PM				
10:45 AM					10:45 PM				
11:00 AM	3		9		11:00 PM	1		4	
11:15 AM	<u> </u>		<u> </u>		11:59 PM	· · ·			
11:30 AM					11:30 PM				
11:45 AM					11:45 PM				
12:00 PM	4		10		12:00 AM	1		1	
				·	L				

TRAFFIC SURVEY - COUNT ANALYSIS 20

009	MUT	CD	WAF	RRA	NTS

	2027 Wit	h Project					
	County:					District No.:	
	City:	Superior	Population:	4,000	_	Survey Date:	11/18/2016
	Route #	Name			Control	Section	85% Speed
Major		US 60					65
Minor		Silver Kin	g Mine Road				25

Warrant 1: Eight- Hour Volumes

Condition A

Number of Lanes			Major Both Ap	Street proaches	Minor Street High Volume Approach		
Major	Street	Minor	Req	uired	Required		
	Slieel	Street	Urban	Rural*	Urban	Rural*	
1		1	500	350	150	105	
2 or more		1	600	420	150	105	
2 or more		2 or more	600	420	200	140	
1		2 or more	500	350	200	140	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

	Warrant 1						
					Cr	iteria	
	Ti	me	Vo	lume	Major	Minor	
	Begin	End	Major	Minor	>= 420	>= 105	Both Meet
	12:00 AM	1:00 AM	22.78888	0.1985852	Ν	Ν	N
	1:00 AM	2:00 AM	26.37265	0.3688011	Ν	Ν	N
	2:00 AM	3:00 AM	38.67883	0.539017	Ν	Ν	N
	3:00 AM	4:00 AM	66.78016	0.8794488	Ν	Ν	N
	4:00 AM	5:00 AM	156.5072	2.241176	Ν	Ν	N
	5:00 AM	6:00 AM	417.0607	6.4965734	Ν	Ν	N
	6:00 AM	7:00 AM	616.469	7.4611302	Y	Ν	N
	7:00 AM	8:00 AM	777.2457	7.9717779	Y	Ν	N
	8:00 AM	9:00 AM	1009.42	8.255471	Y	Ν	N
	9:00 AM	10:00 AM	1068.606	8.0285165	Y	Ν	N
	10:00 AM	11:00 AM	1210.304	8.7377494	Y	Ν	N
	11:00 AM	12:00 PM	1224.86	9.8157834	Y	Ν	N
	12:00 PM	1:00 PM	1441.84	11.007295	Y	Ν	N
	1:00 PM	2:00 PM	1656.221	10.666863	Y	Ν	N
	2:00 PM	3:00 PM	1585.968	9.8157834	Y	Ν	N
	3:00 PM	4:00 PM	1755.72	10.78034	Y	Ν	N
	4:00 PM	5:00 PM	1738.907	11.915113	Y	Ν	N
	5:00 PM	6:00 PM	1322.82	10.298062	Y	Ν	N
	6:00 PM	7:00 PM	1320.168	11.801636	Y	Ν	N
	7:00 PM	8:00 PM	983.7899	9.2200278	Y	Ν	N
	8:00 PM	9:00 PM	688.8207	5.8157098	Y	Ν	N
	9:00 PM	10:00 PM	580.0782	4.7660451	Y	Ν	N
	10:00 PM	11:00 PM	401.4458	3.5177952	N	Ν	N
	11:00 PM	12:00 AM	169.9416	1.3049885	Ν	Ν	N
			Total numb	er of hours, b	oth the major(ooth	
			approaches	s) and minor(high volume ap	proach) met:	0
Condition	A is not sat	isfied			Ho	ours Required:	8
Warrant 1	not satisfie	d.					

Warrant 1: Eight- Hour Volumes Condition B

Number of Lanes			Major Both Apr	Street proaches	Minor Street High Volume Approach		
Major	Street	Minor	Req	uired	Required		
	Slieel	Street	Urban	Rural*	Urban	Rural*	
	1	1	750	525	75	53	
2 o	r more	1	900	630	75	53	
2 or more 1		2 or more	900	630	100	70	
		2 or more	750	525	100	70	

*Criteria when the 85th percentile speed is greater than 40 mph or when the population is less than 10,000

Warrant 2						
				Cri	teria	
Т	ime	Vol	ume	Major	Minor	
Begin	End	Major	Minor	>= 630	> = 53	Both Meet
12:00 AM	1:00 AM	22.78888	0.1985852	N	Ν	N
1:00 AM	2:00 AM	26.37265	0.3688011	Ν	Ν	N
2:00 AM	3:00 AM	38.67883	0.539017	Ν	Ν	N
3:00 AM	4:00 AM	66.78016	0.8794488	Ν	Ν	N
4:00 AM	5:00 AM	156.5072	2.241176	Ν	Ν	N
5:00 AM	6:00 AM	417.0607	6.4965734	Ν	Ν	N
6:00 AM	7:00 AM	616.469	7.4611302	Ν	Ν	N
7:00 AM	8:00 AM	777.2457	7.9717779	Y	Ν	N
8:00 AM	9:00 AM	1009.42	8.255471	Y	Ν	N
9:00 AM	10:00 AM	1068.606	8.0285165	Y	Ν	N
10:00 AM	11:00 AM	1210.304	8.7377494	Y	Ν	N
11:00 AM	12:00 PM	1224.86	9.8157834	Y	Ν	N
12:00 PM	1:00 PM	1441.84	11.007295	Y	Ν	N
1:00 PM	2:00 PM	1656.221	10.666863	Y	Ν	N
2:00 PM	3:00 PM	1585.968	9.8157834	Y	Ν	N
3:00 PM	4:00 PM	1755.72	10.78034	Y	Ν	N
4:00 PM	5:00 PM	1738.907	11.915113	Y	Ν	N
5:00 PM	6:00 PM	1322.82	10.298062	Y	Ν	N
6:00 PM	7:00 PM	1320.168	11.801636	Y	Ν	N
7:00 PM	8:00 PM	983.7899	9.2200278	Y	Ν	N
8:00 PM	9:00 PM	688.8207	5.8157098	Y	Ν	N
9:00 PM	10:00 PM	580.0782	4.7660451	Ν	Ν	N
10:00 PM	11:00 PM	401.4458	3.5177952	Ν	Ν	N
11:00 PM	12:00 AM	169.9416	1.3049885	N	Ν	N

Total number of hours, both the major(both approaches) and minor(high volume approach) met:

0 Hours Required: 8

Condition B is not satisfied Warrant 1 not satisfied.

Warrant 2: Four Hour Vehicular Volumes

This warrant is similar to Warrant 1A, except that the required traffic volumes must be present for at least four hours of an average day. The traffic volumes required are based on curves (Figure 4C-1) shown in the MUTCD.

* The required traffic volumes for Warrant 2 do not meet for any one hour.

Warrant 2 is not satisfied

Warrant 3, Condition A- Peak Hour Delay

This warrant is intended for application where traffic conditions will cause undue delay to traffic entering or crossing the major street. The peak hour delay warrant is satisfied when the following conditions exist for one hour (any four consecutive 15-minute periods) of an average weekday:

- (1) The total delay by the traffic on a side street controlled by a stop sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle-hours for a two-lane approach, **and**
- (2) the volume on the side street (one direction) equals or exceeds 100 vph for one moving lane of traffic and 150 vph for two moving lanes, **and**
- (3) the total traffic volume serviced during 1 hour equals or exceeds 800 vph for an intersection with four (or more) approaches or 650 vph for three approaches.

*Part 1 - N/A *Part 2 - N/A

*Part 3 - N/A

Warrant 3, Condition B - Peak Hour Volume

This warrant applies to traffic entering from the minor street which encounters undue delay crossing the main street. This wa is satisfied when the main street and side street traffic volumes satisfy the curves shown in Figure 4C-4 of the TMUTCD.

Warrant 3 is N/A.

Warrant 4: Po	edestrian '	Volume	
	Re	equired* Existing	
100 or more 190 or more	e for each o e during an	of any four hours OR ny one hour	
* For predom 50 percent.	inant pede	estrian crossing speeds less than 3.5 ft/sec, the pedestrian volume may be reduced as	much as
Gap Requir	ements		
YES YES	NO NO	Is the nearest signal located more than 300 feet away? For traffic flow which is not platooned, are there less than 60 gaps per hour of adeq length for the pedestrians to cross the street?	uate
Warrant 4	is N/A.		

Warrant 5: School Crossing

YES NO Is the number of adequate gaps in traffic stream during the period when the children are using the crossing less than the number of minutes in the same period?

Warrant 5 is N/A.

Narrant 6: Coordinate Systems								
YES	NO	Are the adjacent signals in a signal system?						
YES	NO	Would the resultant spacing be 1000 feet or more?						
Warrant 6	is N/A.							

Warrant 7: Crash Experience

YES	NO	Is 80% or more of one of Warrants #1, #2, or #3 met?
YES	NO	Have there been more than five accidents susceptible to correction by a traffic signal in 12
		months?

Warrant 7 is N/A.

Warrant 8: Roadway Network

YES	NO	Does the major street having an existing or immediately projected entering volume of > 1000 vehicles per hour of a typical weekday?
YES	NO	Do 5-year projected traffic volumes meet Warrants 1, 2, or 3?
YES	NO	Is there an entering traffic volume of at least 1000 vehicles per hour for each of any 5 hours on a Saturday or Sunday?
Warrant 8	is N/A.	

Summary:

Warrants satisfied: none

Warrants not satisfied: 1, 2

Warrants not applicable: 3, 4, 5, 6, 7, 8

Warrants not included in study: none

85th % speed: > 40 mph Population: < 10,000

Major Street Lanes: 2 Minor Street Lanes: 1

Use Figure: 4C-2 2&1

Donk	Major Street	Minor Street		Figure 4C-1			Figure 4C-2	2
Rdiik	Volume	Volume	1&1	2&1	2&2	1&1	2&1	2&2
1	169.9416271	1.30498855	-	-	-	-	N	-
2	22.78887677	0.19858521	-	-	-	-	N	-
3	26.37264818	0.36880111	-	-	-	-	N	-
4	38.67883058	0.53901701	-	-	-	-	N	-
5	66.78015979	0.8794488	-	-	-	-	N	-
6	156.507208	2.24117598	-	-	-	-	N	-
7	417.0607487	6.49657342	-	-	-	-	N	-
8	616.4689592	7.46113017	-	-	-	-	N	-
9	777.2457427	7.97177786	-	-	-	-	N	-
10	1009.419887	8.25547102	-	-	-	-	N	-
11	1068.605872	8.02851649	-	-	-	-	N	-
12	1210.30396	8.7377494	-	-	-	-	N	-
13	1224.860262	9.81578342	-	-	-	-	N	-
14	1441.839704	11.0072947	-	-	-	-	N	-
15	1656.2214	10.6668629	-	-	-	-	N	-
16	1585.968077	9.81578342	-	-	-	-	N	-
17	1755.72009	10.7803402	-	-	-	-	N	-
18	1738.907315	11.9151128	-	-	-	-	N	-
19	1322.819885	10.2980618	-	-	-	-	N	-
20	1320.168382	11.8016356	-	-	-	-	N	-
21	983.7898936	9.22002778	-	-	-	-	N	-
22	688.8207439	5.81570983	-	-	-	-	N	-
23	580.0782043	4.76604513	-	-	-	-	N	-
24	401.4457686	3.51779521	-	-	-	-	Ν	-
			0	0	0	0	0	0
Warrant 2 is not	satisfied.		Ν	Ν	Ν	N	Ν	Ν

Warrant 2 Figure 4C-2 Four Hour Volume Warrant (population<10,000 or >40 mph on major street)



it is plotted at the maximum shown value(s).

Major Street-Total of Both Approaches-vph



TRAFFIC IMPACT ANALYIS - ADDENDUM #1 RESOLUTION COPPER MINE PROJECT SUPERIOR, ARIZONA

APPENDIX

Crash Data

2014-2018

IncidentID	IncidentDate	CollisionManner	Totallnjuries	TotalFatalities	InjurySeverity	Onroad	CrossingFeature	Offset
2818772	3/15/2014 0:00	4	0	0	1	US Highway 60	Main St	0.0379
3012714	10/7/2015 0:00	3	0	0	1	US Highway 60	Main St	0

LEGEND

CollisionManner

SINGLE_VEHICLE
ANGLE (front to side)(other than left turn)
LEFT_TURN
REAR_END
HEAD_ON
SIDESWIPE_SAME_DIRECTION
SIDESWIPE_OPPOSITE_DIRECTION
REAR_TO_SIDE
REAR_TO_REAR
U_TURN
OTHER
UNKNOWN

InjurySeverity

1 NO_INJURY 2 POSSIBLE_INJURY 3 SUSPECTED_MINOR_INJURY 4 SUSPECTED_SERIOUS_INJURY 5 FATAL 99 UNKNOWN