

Prepared in cooperation with the Flood Control District of Maricopa County

Methods for Estimating Magnitude and Frequency of 1-, 3-, 7-, 15-, and 30-day flood-duration flows in Arizona



Scientific Investigations Report 2014–5109

Version 1.1, April 2015

FRONT COVER

Photograph of December 30, 2004, flooding on the Agua Fria River downstream from U.S. Geological Survey streamgaging station 09512500, Agua Fria River near Mayer, Arizona (U.S. Geological Survey photograph by Jeffrey Kennedy).

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By Jeffrey R. Kennedy, Nicholas V. Paretti, and Andrea G. Veilleux

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[Available online only at <http://pubs.usgs.gov/of/2014/5109>]

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Acronyms and Abbreviations

AEP	annual exceedance probability
AVP	average variance of prediction
B-GLS	Bayesian generalized least squares
DRNAREA	drainage area/contributing area of the watershed
ELEV	average basin elevation
EMA	expected moments algorithm
AVP	average variance of prediction
GB	Grubbs-Beck
GIS	geographic information system
GLS	generalized least squares
IACWD	Interagency Advisory Committee on Water Data
LP3	log-Pearson type III
MGB	multiple Grubbs-Beck
OLS	ordinary least squares
PRISM	parameter-elevation regressions on independent slopes model
$SE_{p,ave}$	average standard error of prediction (also SEP)
STATSGO	State Soil Geographic (soil data)
USGS	U.S. Geological Survey
VIF	variance inflation factor
VP_r	variance of prediction, regression estimate
VP_s	variance of prediction, station estimate
VP_w	variance of prediction, weighted estimate
WLS	weighted least squares
WREG	USGS linear regression software

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Methods for Estimating Magnitude and Frequency of 1-, 3-, 7-, 15-, and 30-day flood-duration flows in Arizona

By Jeffrey R. Kennedy, Nicholas V. Paretti, and Andrea G. Veilleux

Abstract

Large floods have historically caused extensive damage in Arizona. Although peak-flow frequency estimates are required for managing the risk posed by floods, estimates of the frequency of sustained flood flow (flood-duration flow) are also useful for planning and assessing the adequacy of retention and conveyance structures and for water-resource planning. This report presents a flood-duration flow frequency analysis for selected durations (1 day, 3 day, 7 day, 15 day, and 30 day) at 173 streamgaging stations throughout Arizona and in western New Mexico. For each n -day duration, a log-Pearson type III distribution was fitted to the annual series of n -day flood-duration flows using the expected moments algorithm with a multiple Grubbs-Beck low-outlier test. Regional skews were developed independently for each n -day duration using a hybrid weighted least squares/generalized least squares method. No basin characteristics were found to adequately explain variation in skew among stations and a constant statewide skew model was used for all n -day durations. The regional skewness coefficient is negative for all n -day durations and becomes increasingly negative for longer n -day durations. Uncertainty associated with the skewness coefficient is estimated using a Bayesian generalized least squares technique.

Regression equations, which allow predictions of n -day flood-duration flows for selected annual exceedance probabilities at ungaged sites, were developed using generalized least-squares regression and flood-duration flow frequency estimates at 56 streamgaging stations within a single, relatively uniform physiographic region in the central part of Arizona, between the Colorado Plateau and Basin and Range Province, called the Transition Zone. Drainage area explained most of the variation in the n -day flood-duration annual exceedance probabilities, but mean annual precipitation and mean elevation were also significant variables in the regression models. Standard error of prediction for the regression equations varies from 28 to 53 percent and generally decreases with increasing n -day duration. Outside the Transition Zone there are insufficient streamgaging stations to develop regression equations, but flood-duration flow frequency estimates are presented at select streamgaging stations.

Introduction

Flood-frequency analyses are a common tool for assessing flood-hazard risk. Such analyses typically focus on the frequency of maximum instantaneous flow (peak flow), and use statistical methods to predict the annual peak flow for a specified probability, known as the annual exceedance probability (AEP). The estimated peak flow of large floods, with low AEP, are widely used to delineate flood-plain boundaries and predict potential property damage but also for designing structures designed to convey runoff at a sufficient rate, such as bridges, channels, and culverts. However, for detention and retention type structures, estimates of the frequency of a volume of flood flow over some duration of time (flood-duration flow) are also needed. Furthermore, flood-duration frequency estimates can be used for water-resources planning and management, particularly on river systems with water-storage reservoirs.

The log-Pearson type III (LP3) distribution has been adopted as the standard flood-frequency model throughout the United States. Methods for fitting the moments (mean, standard deviation, and skew) of the LP3 distribution are described in a report published by the Interagency Advisory Committee on Water Data (IACWD) and widely known as "Bulletin 17B" (IACWD, 1982). Although Bulletin 17B procedures are typically used for estimating the AEPs of flood peaks, the same procedures can also be used for flood-duration flows. Since publication, several improvements to Bulletin 17B have been suggested concerning the treatment of low-outlier, historical, and other censored flood information (Stedinger and Griffis, 2008). The expected moments algorithm (EMA), used with the multiple Grubbs-Beck (MGB) test, is a revision to the traditional Bulletin 17B estimation methods that explicitly addresses censored data (Cohn and others, 1997; Cohn and others, 2001; England and others, 2003). Of particular note for Arizona and other semiarid regions with large variability in annual maximum floods, the MGB test efficiently accounts for multiple potentially influential low-flows, which may otherwise have undue influence on the estimated magnitude of large, low-probability floods. An evaluation of the implications for replacing Bulletin 17B methods with EMA/MGB methods for Arizona streamgaging stations (Paretti and others,

2014a) found that although predicted peak flows using EMA/MGB were neither consistently larger nor smaller than Bulletin 17B predictions, goodness-of-fit criteria indicated EMA/MGB provided a better representation of the peak-flow data. Therefore, EMA/MBG methods are used to implement the flood-duration flow analysis in this report.

StreamStats is a national U.S. Geological Survey (USGS) map-based Web application that provides easy access to published flood-frequency and basin-characteristic statistics for user-selected watersheds. This interactive Web application allows the user to select a point on a stream channel (gaged or ungaged), delineate a watershed boundary, and retrieve flood-frequency estimates derived from the current regional regression equations and geographic information system (GIS) data within the basin selected. StreamStats provides consistent statistics, minimizes user error, and reduces the need for large datasets and costly standalone GIS software. Peak-flow frequency estimates (Paretti and others, 2014b) and n -day flood-duration flow frequency estimates (this report) are available online in the StreamStats Web application at http://streamstatsags.cr.usgs.gov/az_ss.

Physical Setting

Streamgaging stations used in the flood-duration flow frequency analysis are located throughout Arizona but are primarily concentrated in a region in the central part of the State, between the Basin and Range Province to the southwest and the Colorado Plateau to the northeast, called the Transition Zone (fig. 1). The Transition Zone region is characterized by high relief with small, relatively shallow aquifers. Land-surface elevations in this region range from about 2,000 feet near the confluence of the Salt and Verde Rivers to about 11,400 feet at the headwaters of the Salt River in eastern Arizona (fig. 2A). Most major Arizona streams and rivers, with the exception of the Colorado River, have their headwaters in this region, including the Gila, Salt, Verde, and Hassayampa Rivers. Smaller drainages in this region are mostly intermittent or ephemeral. Precipitation and air temperature are highly variable throughout the Transition Zone and are correlated with land-surface elevation; higher elevations experience lower average temperatures and greater precipitation amounts that do lower elevations. Average annual precipitation in the region ranges from 39 inches per year near the headwaters of the Salt River in the White Mountains to less than 10 inches per year in the lower deserts (fig. 2B).

Both the Basin and Range Province to the southwest of the Transition Zone and the Colorado Plateau to the northeast have very little perennial surface water, and streamflow, even in large drainages, often occurs only in response to discrete precipitation events. Notable exceptions are certain reaches of the Santa Cruz and San Pedro Rivers in the Basin and Range and the upper reaches of the Little Colorado River and its tributaries on the Colorado Plateau. Land-surface elevations in the Basin and Range vary from 100 feet along the Lower

Colorado River, to a few thousand feet on basin floors, to more than 10,000 feet in some mountain ranges (fig. 2A). Less rainfall and higher temperatures are characteristic of the Basin and Range lowlands as compared to the Transition Zone. Mean annual precipitation in this region ranges from less than 4 inches per year in southwest Arizona to greater than 30 inches per year at high elevations toward the southeast corner of the State (fig. 2B). The Colorado Plateau covers roughly 45,000 square miles in northeast Arizona and is characterized by low relief punctuated by numerous canyon drainages, the most notable being the Grand Canyon of the Colorado River. The average elevation on the plateau is about 5,000 feet and average rainfall is about 10 inches per year (fig. 2A and B). At higher altitudes on the plateau, annual peak flows can be influenced by snowmelt, but no streamgaging stations where flows are dominated by snowmelt are included in this flood-duration flow analysis. Streams are generally spring fed and, as with the Basin and Range, typically have high transmission losses, and streamflow quickly infiltrates downstream.

Purpose and Scope

The primary purposes of this report are to (1) present an application of newly developed flood-frequency methods, namely the expected moments algorithm and multiple Grubbs-Beck low-outlier test, and a hybrid Bayesian weighted least-squares/generalized least-squares method for estimating regional skewness coefficients and uncertainty; (2) present estimates of the annual maximum 1-, 3-, 7-, 15-, and 30-day flood-duration flows for the 50-, 20-, 10-, 4-, 2-, 1-, 0.5-, and 0.2-percent annual exceedance probabilities at 173 streamgaging stations in Arizona with 10 or more years of record; and (3) present regional regression equations for estimating the annual maximum 1-, 3-, 7-, 15-, and 30-day flood-duration flows for the 50-, 20-, 10-, 4-, 2-, 1-, 0.5-, and 0.2-percent annual exceedance probabilities at ungaged basins in the central part of the State.

Data Development

Flood-duration flow can be defined as the average mean daily flow over a specified duration, often referred to as the annual maximum n -day flood flow. Durations considered in this report are the 1-, 3-, 7-, 15-, and 30-day flood-duration flows. The 1-day flood-duration flow is simply the highest annual mean daily flow and most often occurs during the same event and on the same day as the annual instantaneous peak flow. The longer n -day intervals are determined as the period of n consecutive days with the highest average flow in a given water year. As the duration interval length increases, the probability that it encompasses the annual instantaneous peak flow decreases (fig. 3). At some stations, there may not be 15 or 30 days of continuous flow during the year, and high flows for these durations can include a period of zero flow.

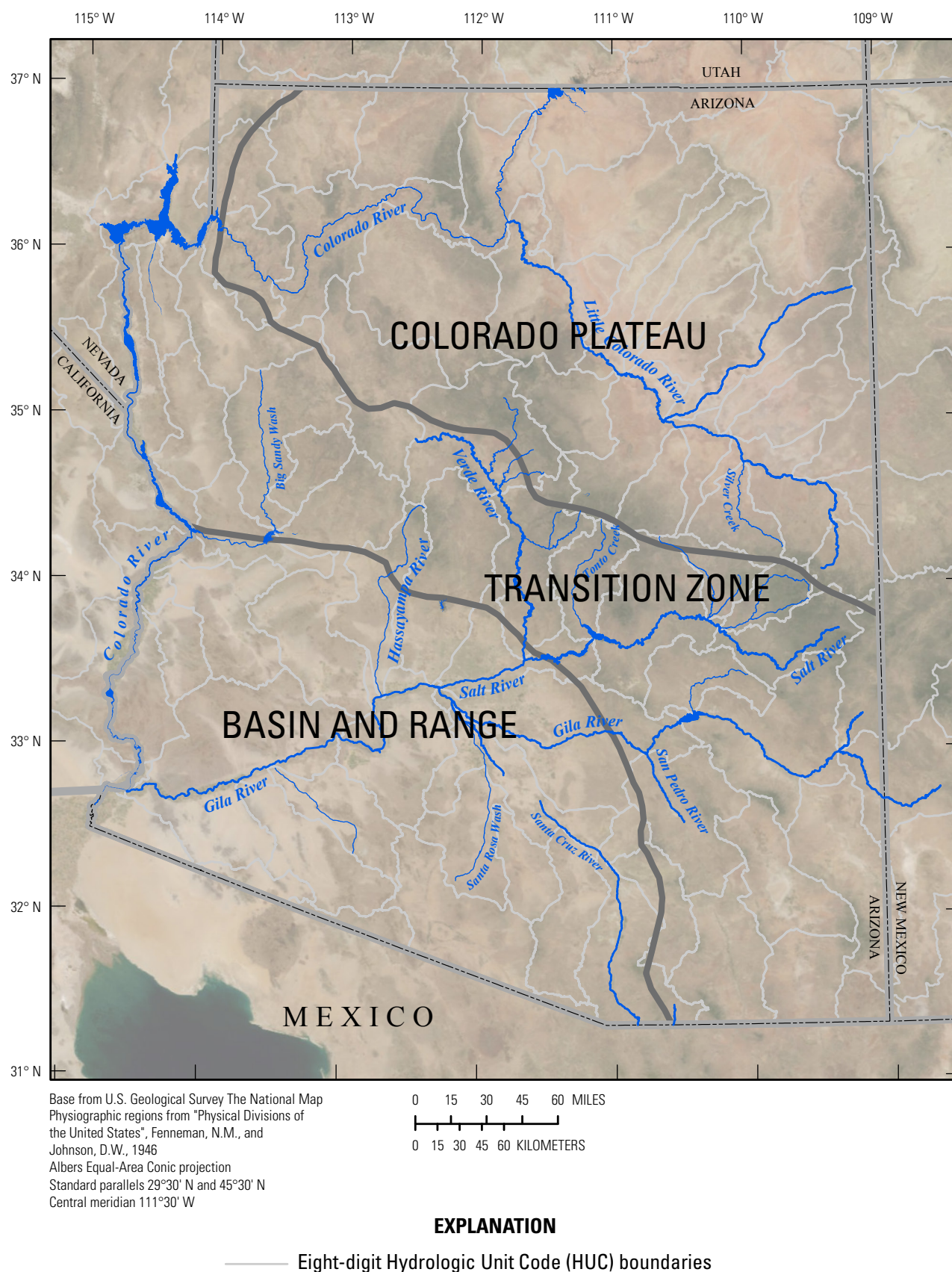


Figure 1. Map of Arizona showing major physiographic regions.

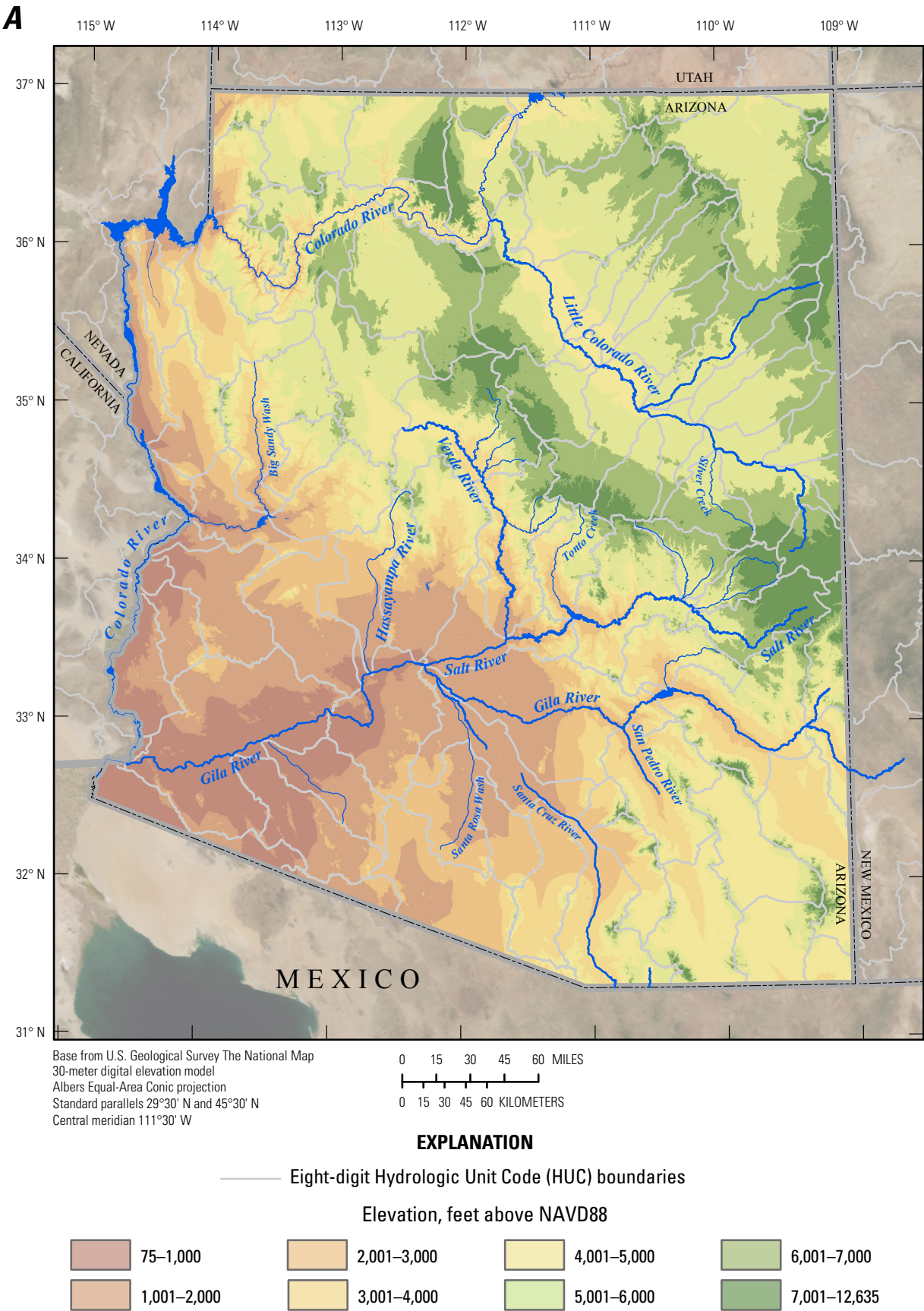


Figure 2. Maps of (A) elevation and (B) mean annual precipitation in Arizona.

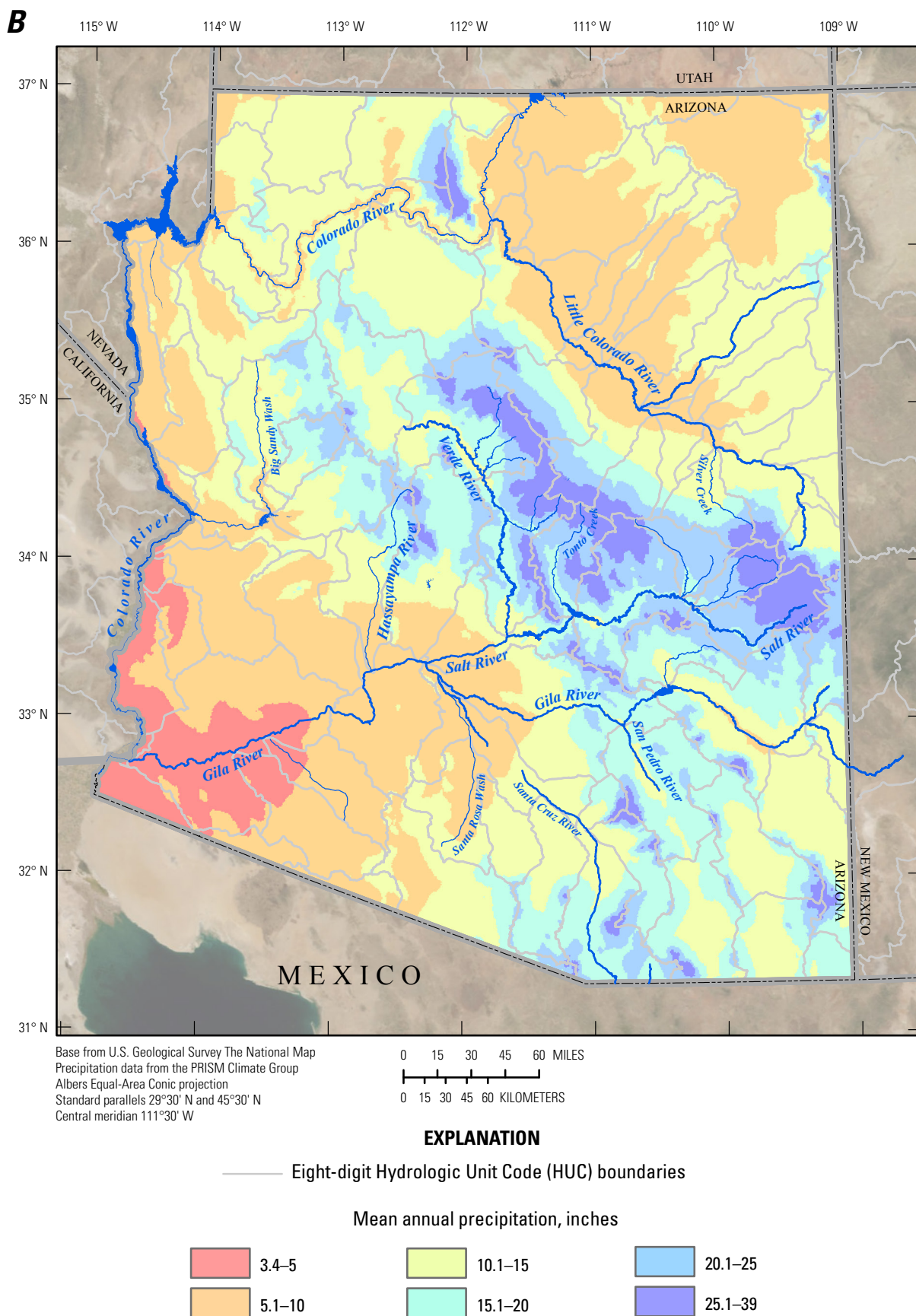


Figure 2.—Continued

Flood-duration flows are reported in dimensions of volume per time and units of cubic feet per second; to convert to total volume the flow rate is simply multiplied by the length of the duration interval considered.

Site Selection

Determination of *n*-day flood-duration flow requires continuous records of mean daily flow. Some gages operated by the USGS in Arizona are crest-stage gages, which record only the maximum stage between site visits; therefore, there are fewer gages available for flood-volume analysis than for flood-peak analysis. Annual maximum flood-duration flow data for 1-, 3-, 7-, 15-, and 30-day duration intervals were retrieved using SWSTAT software from the USGS National Water Information System for an initial dataset of 198 streamgaging stations with record lengths 10 years or longer. Stations are located primarily in central and southeastern Arizona, with significant gaps in northeast, northwest, and southwest Arizona (fig. 4). Stations with *n*-day flood-duration flows that were poorly represented by the LP3 distribution were excluded, as were stations significantly affected by impoundments, diversions, or urbanization. Therefore, no flood-duration flow frequency estimates are given for the Colorado River, Verde River below Horseshoe Dam, Salt River below Roosevelt Dam, and Gila River below Coolidge Dam. After removing unsuitable stations, 173 remaining stations were used in the analysis (table 1), of three types:

- Stations with greater than 20 years of record, well approximated by the LP3 distribution, used to determine the regional skewness coefficient of the LP3 distribution.

Redundant stations (stations that are near another station with similar basin characteristics) were removed as described below in the section Regional Skew Analysis and Cross-Correlation Models.

- Stations with between 10 and 20 years of record, located in the Transition Zone, and well approximated by the LP3 distribution. These stations, combined with stations used in the regional skew analysis that are in the Transition Zone, were used to generate the regional regression equations.
- Stations with more than 10 years of record not used in the regional skew or regional regression analyses.

The Mann-Kendall trend test was used to test for trends in *n*-day flood-duration flows at stations with 30 or more years of record (Helsel and Hirsch, 2002). The null hypothesis (H_0 , no trend in streamflow) was rejected at the 5-percent significance level at four streamgaging stations (table 2). At three stations, H_0 was rejected for all durations; at the fourth station, H_0 was rejected only for the 1-, 3-, and 7-day flood-duration flows. The trend in flood-duration flow was downward at all of these stations, and no stations had increasing trends in flood-duration flow. Despite the apparent trend at these stations, flood-duration flow frequency results are presented, and three of the four were used in the regional skew analysis because they have long records and represent watersheds in geographic and (or) physiographic regions where there are no alternative stations. None of the stations where H_0 was rejected were used in the regional regression equations. Possible reasons for downward trends at these stations include changes in watershed characteristics, such as vegetation and channel

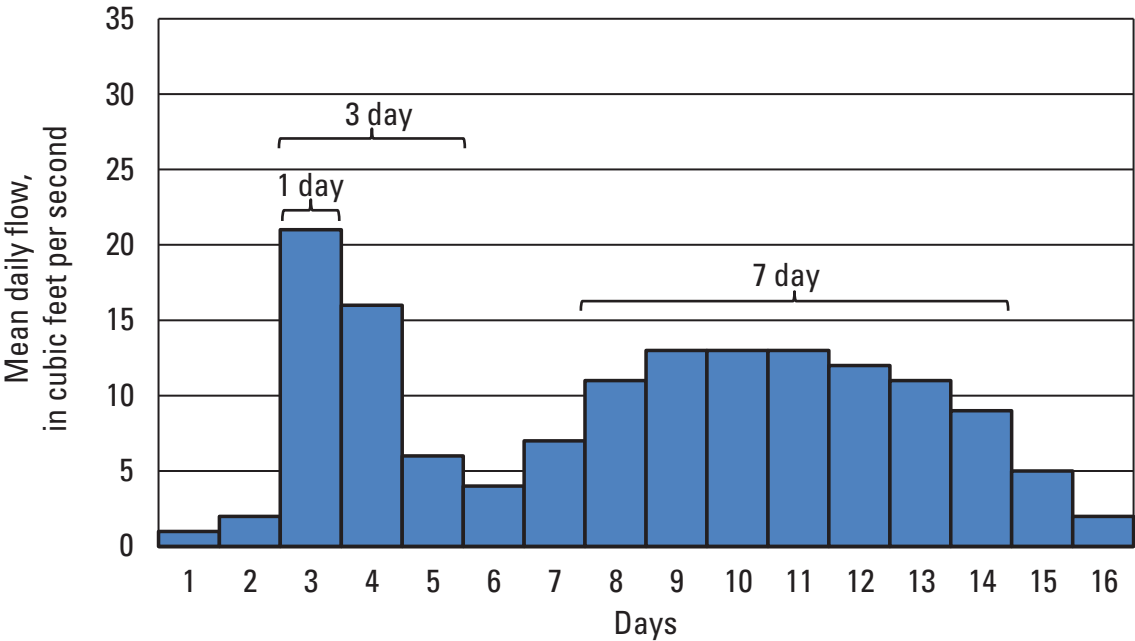


Figure 3. Graph of example data used to calculate 1-day, 3-day, and 7-day flood-duration flows and their relation to annual maximum peak flow.

morphology, human activities, and decreasing seasonal rainfall (Thomas and Pool, 2006; Kennedy and Gungle, 2010).

Basin Characteristics

As part of the larger Arizona StreamStats project (Ries and others, 2008; Paretti and others, 2014b), watershed boundaries for each streamgaging station were calculated using the Watershed Boundary Dataset, the 1:24,000 National Hydrography Dataset, and the 1/3 arc-second (10-meter) National Elevation Dataset. Within each watershed boundary, several characteristics were computed using the best available data (table 3). Elevation is calculated as the mean elevation throughout the watershed area. Precipitation metrics were identified using parameter-elevation regressions on independent slopes model (PRISM) monthly data (PRISM Climate Group, 2012). Two soil characteristics, permeability and available water capacity, were identified using the State Soil Geographic Database (STATSGO; Natural Resources Conservation Service, 2013). Full details of the basin characteristic identification process are in Paretti and others (2014b).

Flood-Volume Frequency Analysis

Several approaches to n -day flood-duration flow analysis have been presented in the hydrologic literature. The following discussion considers statistical approaches. Alternatively, rainfall-runoff models can be constructed to predict flood flow and duration as a function of the probability of a given rainfall event (for example, Bohman, 1990; Sherwood, 1994), but these are more suitable for small watersheds and do not fully use the historical runoff data collected by the USGS.

Statistical approaches to flood-duration flow frequency analysis, in which measured runoff volumes are used directly to estimate the probability of a given duration of flood flow, generally fall into one of three categories. First, individual distributions, such as the log-Pearson type III (LP3) distribution commonly used in analyzing instantaneous peaks (IACWD, 1982), can be fitted to the n -day flood-duration flows. The USGS Manual of Hydrology, discussing graphical methods of fitting curves to data on a probability plot states simply: “the frequency of flood volume can be determined by the same method as the frequency of flood peaks” (Dalrymple, 1960). The Bulletin 17B manual also recognizes “the same techniques could also be used to treat . . . flood volumes” but states such applications were not evaluated (IACWD, 1982). Devulapalli and Valdes (1996) used the LP3 distribution successfully to model 2-, 3-, 4-, 5-, 6-, 7-, 8-, 9-, and 10-day volumes for rural watersheds in Texas. Sherwood (1994) modeled small urban watershed flood volumes with the LP3 distribution, but flood intervals were much shorter (1 to 32 hours) than those considered in the present study. Two approaches to distribution-fitting can be taken—either (1) unique distribution moments can be defined for each n -day interval or (2) an average distribution can be fitted to all of the data and a scaling parameter identified that controls the spacing between

the different n -day intervals on a quantile-probability plot (Javelle and others, 2003; Cunderlik and Ouarda, 2006). The latter approach was investigated for streamgaging stations in Arizona, but large variation existed in the quantile-probability plots among different n -day durations, and it was found to be unsuitable.

A second approach to high-flow frequency analysis is the joint probability distribution approach, which treats flood peak, volume, and duration as random variables to be predicted concurrently (Yue and Rasmussen, 2002; Mediero and others, 2010). This method recognizes that flood peaks and volume are not independent; rather, for a flood peak with a given probability there may be a range of flood volumes that occur with varying probability. Finally, a third approach develops regression equations that relate flood volume to flood peak flow (Eychaner, 1976; Perry, 1984; Singh and Hossein, 1986). The instantaneous flood peak at a given probability is estimated by fitting a statistical distribution, and the corresponding flood volume at the same probability is determined from the regression. In effect, this approach is the opposite of the joint probability approach; it assumes a unique relation between a flood peak of a given probability and a corresponding flood volume. Both the joint probability and the peak-volume regression approaches require datasets that contain both the flood peak and corresponding flood volume for each year. Although the annual instantaneous flood peak often corresponds to the same event as the 1-day and 3-day flood volumes, at longer intervals this is often not the case (Balocki and Burges, 1994). Determining correspondence between flood peaks and volumes at these longer intervals is a significant task; therefore, the fitted-distribution approach is taken in the present study.

The LP3 distribution is defined by the first, second, and third moments (the mean, standard deviation, and skew, denoted by μ , σ , and γ , respectively). On a log-probability plot of annual flood peaks (fig. 5), the distribution can be represented as either a straight line (skew = 0) or one that curves. The distribution mean determines the position of the line along the y-axis and the standard deviation determines the slope of the line. The basic equation for determining flood frequency from the three moments is:

$$\log Q_p = \bar{X} + K_p S, \quad (1)$$

where

- Q_p is the annual-peak flow (in this case, n -day flood-duration flow) for the exceedance probability, P ,
- $\bar{X}$ is the mean of the logarithms of the annual-peak flow,
- K_p is a factor based on the weighted skew coefficient and the exceedance probability, P , which can be obtained from appendix 3 of Bulletin 17B (IACWD, 1982), and
- S is the standard deviation of the logarithms of the annual-peak flow, which is a measure of the degree of variation in the annual values about the mean value.

Table 1. Table of streamgaging stations in Arizona and western New Mexico with flood-duration flow frequency statistics.

[AZ, Arizona; NM, New Mexico; Y, yes; N, no]

Map ID	Station ID	Station Name	Period of Record	Number of years of record	Drainage area (square miles)	Mean annual precipitation (inches)	Mean elevation (feet above NAVD88)	Used in skew analysis	Used in regional regression
1	09379200	Chinle Creek near Mexican Water, AZ	1965–2010	46	3,612	10.1	6,244	Y	N
2	09382000	Paria River at Lees Ferry, AZ	1924–2010	87	1,362	11.9	6,138	Y	N
3	09383400	Little Colorado River at Greer, AZ	1961–2010	24	28.9	32.8	9,437	Y	N
4	09383500	Nutrios Creek above Nelson Res near Springerville, AZ	1968–2010	17	83.4	23.4	8,532	N	N
5	09384000	Little Colorado River above Lyman Lake near St. Johns, AZ	1941–2010	70	711	18.4	7,833	Y	N
6	09386250	Carrizo Wash near St. Johns, AZ	1999–2010	12	2,143	13.3	7,124	N	N
7	09386950	Zuni River above Black Rock Reservoir, NM	1970–2010	41	829	15.7	7,363	N	N
8	09390500	Show Low Creek near Lakeside, AZ	1954–2010	57	68.0	27.6	7,290	Y	N
9	09392500	Show Low Creek at Show Low, AZ	1945–1954	10	92.6	25.9	7,107	N	N
10	09393500	Silver Creek near Snowflake, AZ	1951–1995	45	840	18.0	6,354	Y	N
11	09394500	Little Colorado River at Woodruff, AZ	1906–2010	80	7,652	14.4	7,039	Y	N
12	09395900	Black Creek near Lupton, AZ	1965–1982	15	494	14.5	7,407	N	N
13	09397000	Little Colorado River at Holbrook, AZ	1906–2010	31	10,836	13.9	6,797	N	N
14	09397500	Chevelon Fork below Wildcat Canyon near Winslow, AZ	1948–2010	38	272	24.8	7,070	N	N
15	09398000	Chevelon Creek near Winslow, AZ	1917–2006	45	759	19.5	6,554	Y	N
16	09398500	Clear Creek below Willow Creek near Winslow, AZ	1948–1991	44	318	27.0	7,172	Y	N
17	09399000	Clear Creek near Winslow, AZ	1930–2006	52	607	20.8	6,560	Y	N
18	09400562	Oraibi Wash near Tolani Lake, AZ	1996–2010	15	665	10.4	6,278	N	N
19	09400568	Polacca Wash near Second Mesa, AZ	1995–2010	16	912	10.1	6,408	N	N
20	09400583	Jeddito Wash near Jeddito, AZ	1994–2005	12	148	10.8	6,371	N	N
21	09401000	Little Colorado River at Grand Falls, AZ	1927–1994	35	20,003	13.4	5,885	N	N
22	09401110	Dinnebito Wash near Sand Springs, AZ	1994–2010	17	478	10.4	6,300	N	N
23	09401260	Moenkopi Wash at Moenkopi, AZ	1977–2010	34	1,231	9.7	6,115	N	N
24	09401280	Moenkopi Wash near Tuba, AZ	1927–1940	14	1,393	9.5	6,035	N	N
25	09401400	Moenkopi Wash near Tuba City, AZ	1941–1978	26	1,731	9.1	5,892	Y	N
26	09401500	Moenkopi Wash near Cameron, AZ	1954–1964	11	1,881	8.9	5,807	N	N
27	09402000	Little Colorado River near Cameron, AZ	1948–2010	63	24,350	12.7	6,170	Y	N
28	09403000	Bright Angel Creek near Grand Canyon, AZ	1924–1973	50	101	22.1	7,312	Y	N

Table 1. Table of streamgaging stations in Arizona and western New Mexico with flood-duration flow frequency statistics.—Continued

Map ID	Station ID	Station Name	Period of Record	Number of years of record	Drainage area (square miles)	Mean annual precipitation (inches)	Mean elevation (feet above NAVD88)	Used in skew analysis	Used in regional regression
29	09403780	Kanab Creek near Fredonia, AZ	1964–1980	17	1,124	15.0	6,000	N	N
30	09404110	Havas Creek at Supai, AZ	1996–2010	15	2,428	14.3	6,054	N	N
31	09404208	Diamond Creek near Peach Springs, AZ	1994–2010	17	276	12.0	4,925	N	N
32	09404222	Spencer Creek near Peach Springs, AZ	1999–2010	12	257	12.8	4,781	N	N
33	09404343	Truxton Wash near Valentine, AZ	1994–2010	17	375	14.9	5,112	N	N
34	09415000	Virgin River at Littlefield, AZ	1930–2000	58	4,858	14.4	5,170	Y	N
35	09424200	Cottonwood Wash No. 1 near Kingman, AZ	1965–1978	14	135	19.3	5,363	N	Y
36	09424447	Burro Creek at Old US 93 Bridge near Bagdad, AZ	1981–2010	16	611	18.4	4,658	N	N
37	09424450	Big Sandy River near Wikieup, AZ	1967–2010	44	2,562	15.7	4,326	Y	Y
38	09424900	Santa Maria River near Bagdad, AZ	1967–2010	41	1,130	16.9	3,992	Y	Y
39	09425500	Santa Maria River near Alamo, AZ	1941–1965	25	1,433	15.9	3,725	N	N
40	09426500	Bill Williams River at Planet, AZ	1915–1946	19	5,307	14.9	3,718	N	N
41	09430500	Gila River near Gila, NM	1929–2010	82	1,856	20.4	7,451	Y	Y
42	09431500	Gila River near Redrock, NM	1931–2010	73	2,828	19.8	6,896	N	N
43	09442000	Gila River near Clifton, AZ	1912–2010	78	4,007	18.1	6,227	Y	Y
44	09442680	San Francisco River near Reserve, NM	1961–2010	50	333	21.0	7,800	Y	Y
45	09444000	San Francisco River near Glenwood, NM	1928–2010	83	1,653	20.8	7,231	N	N
46	09444200	Blue River near Clifton, AZ	1969–2010	38	505	23.1	6,852	Y	Y
47	09444500	San Francisco River at Clifton, AZ	1914–2010	84	2,765	20.9	6,811	Y	Y
48	09445500	Willow Creek near Point Of Pines near Morenci, AZ	1945–1967	23	107	21.0	6,295	N	N
49	09446000	Willow Creek near Double Circle Ranch near Morenci, AZ	1945–1967	23	155	21.0	6,239	Y	Y
50	09446500	Eagle Creek near Double Circle Ranch near Morenci, AZ	1945–1967	23	383	21.3	6,281	N	N
51	09447000	Eagle Creek above Pumping Plant near Morenci, AZ	1945–2010	66	621	20.5	6,009	Y	Y
52	09447800	Bonita Creek near Morenci, AZ	1982–2010	29	302	17.4	5,247	Y	Y
53	09448500	Gila River at Head of Safford Valley near Solomon, AZ	1921–2010	88	7,888	19.2	6,329	Y	Y
54	09456000	San Simon River near San Simon, AZ	1920–1940	13	823	16.2	4,881	N	N
55	09457000	San Simon River near Solomon, AZ	1932–1982	48	2,243	14.1	4,334	Y	N
56	09458200	Deadman Creek near Safford, AZ	1968–1993	15	4.7	29.1	7,361	N	N
57	09458500	Gila River at Safford, AZ	1941–1965	15	10,483.0	17.9	5,828	N	N
58	09460150	Frye Creek near Thatcher, AZ	1968–2010	30	4.0	33.6	8,127	Y	Y

Table 1. Table of streamgaging stations in Arizona and western New Mexico with flood-duration flow frequency statistics.—Continued

Map ID	Station ID	Station Name	Period of Record	Number of years of record	Drain-age area (square miles)	Mean annual precipitation (inches)	Mean elevation (feet above NAVD88)	Used in skew analysis	Used in regional regression
59	09468500	San Carlos River near Peridot, AZ	1930–2010	81	1,026.0	19.1	4,443	Y	Y
60	09470500	San Pedro River at Palominas, AZ	1931–2010	54	738.0	19.2	5,033	N	N
61	09470750	Ramsey Canyon near Sierra Vista, AZ	2001–2010	10	4.2	28.5	7,325	N	N
62	09470800	Garden Canyon near Fort Huachuca, AZ	1960–2010	21	8.6	25.8	6,707	Y	N
63	09471000	San Pedro River at Charleston, AZ	1905–2010	95	1,216	18.1	4,938	Y	N
64	09471310	Huachuca Canyon near Fort Huachuca, AZ	2001–2010	10	4.1	26.4	6,811	N	N
65	09471380	Upper Babocomari River near Huachuca City, AZ	2001–2010	10	156	18.6	5,138	N	N
66	09471400	Babocomari River near Tombstone, AZ	2001–2010	10	303	17.8	5,005	N	N
67	09471550	San Pedro River near Tombstone, AZ	1968–2010	33	1,729	17.7	4,898	N	N
68	09471800	San Pedro River near Benson, AZ	1967–2010	14	2,487	17.1	4,746	N	N
69	09472000	San Pedro River near Redington, AZ	1944–1997	50	2,925	17.1	4,681	Y	N
70	09472050	San Pedro River at Redington Bridge near Redington, AZ	1999–2010	12	2,925	17.1	4,681	N	N
71	09473000	Aravaipa Creek near Mammoth, AZ	1932–2010	54	538	18.6	4,572	Y	Y
72	09473500	San Pedro River at Winkelman, AZ	1967–1978	12	4,451	17.4	4,444	N	N
73	09480000	Santa Cruz River near Lochiel, AZ	1950–2010	61	82.0	19.7	5,093	Y	N
74	09480500	Santa Cruz River near Nogales, AZ	1914–2010	82	532	19.8	4,891	Y	N
75	09481500	Sonoita Creek near Patagonia, AZ	1931–1972	40	209	21.2	4,919	Y	N
76	09481740	Santa Cruz River at Tubac, AZ	1996–2010	14	1,213	20.1	4,617	N	N
77	09482000	Santa Cruz River at Continental, AZ	1941–2010	58	1,673	19.6	4,391	Y ²	N
78	09482400	Airport Wash at Tucson, AZ	1966–1981	16	29.6	13.7	2,848	N	N
79	9482500	Santa Cruz River at Tucson, AZ	1906–2010	84	2,192	18.5	4,095	Y	N
80	09483000	Tucson Arroyo at Vine Ave at Tucson, AZ	1945–1981	37	7.6	12.4	2,516	N	N
81	09483010	High School Wash at Tucson, AZ	1974–1983	10	1.0	12.2	2,464	N	N
82	09483100	Tanque Verde Creek near Tucson, AZ	1960–1974	15	43.1	21.3	4,858	N	N
83	09484000	Sabino Creek near Tucson, AZ	1933–2010	63	35.2	30.2	6,077	Y	N
84	09484200	Bear Creek near Tucson, AZ	1960–1974	15	16.9	27.9	5,781	N	N
85	09484500	Tanque Verde Creek at Tucson, AZ	1941–2010	25	220.0	21.1	4,372	Y	N
86	09484600	Pantano Wash near Vail, AZ	1960–2010	36	456.0	19.1	4,618	Y	N
87	09485000	Rincon Creek near Tucson, AZ	1953–2010	43	44.7	21.4	5,104	Y	N
88	09485450	Pantano Wash at Broadway Blvd. at Tucson, AZ	1989–2010	21	598	18.7	4,434	Y ²	N

Table 1. Table of streamgaging stations in Arizona and western New Mexico with flood-duration flow frequency statistics.—Continued

Map ID	Station ID	Station Name	Period of Record	Number of years of record	Drain-age area (square miles)	Mean annual precipitation (inches)	Mean elevation (feet above NAVD88)	Used in skew analysis	Used in regional regression
89	09485700	Rillito Creek at Dodge Boulevard at Tucson, AZ	1991–2010	19	868	19.2	4,348	N	N
90	09486055	Rillito Creek at La Cholla Blvd near Tucson, AZ	1996–2010	15	912	19.0	4,283	N	N
91	09486300	Canada Del Oro near Tucson, AZ	1966–1978	13	250	19.0	3,926	N	N
92	09486350	Canada Del Oro below Ina Road near Tucson, AZ	1996–2010	15	255	18.6	3,894	N	N
93	09486500	Santa Cruz River at Cortaro, AZ	1940–2010	59	3,461	18.5	4,084	N	N
94	09486800	Altar Wash near Three Points, AZ	1967–2010	27	466	18.6	3,741	Y ^c	N
95	09487000	Brawley Wash near Three Points, AZ	1993–2010	18	785	17.5	3,622	N	N
96	09488500	Santa Rosa Wash near Vaiva Vo, AZ	1955–1980	26	1,734	11.1	2,217	Y	N
97	09489000	Santa Cruz River near Laveen, AZ	1941–2010	68	8,568	14.5	3,019	N	N
98	09489070	North Fork of East Fork Black River near Alpine, AZ	1966–1978	13	38.4	28.9	9,052	N	Y
99	09489100	Black River near Maverick, AZ	1963–1982	20	314	28.7	8,538	N	N
100	09489200	Pacheta Creek at Maverick, AZ	1958–1980	23	16.3	32.9	8,604	N	N
101	09489500	Black River below Pumping Plant near Point of Pines, AZ	1954–2010	57	556	27.7	8,058	Y	Y
102	09489700	Big Bonito Creek near Fort Apache, AZ	1958–1980	23	114	31.3	8,077	Y	Y
103	09490500	Black River near Fort Apache, AZ	1915–2010	54	1,224	25.5	7,222	N	N
104	09490800	North Fork White River near Greer, AZ	1966–1978	13	40	36.4	9,520	N	Y
105	09492400	East Fork White River near Fort Apache, AZ	1958–2010	53	47	35.5	8,425	Y	Y
106	09494000	White River near Fort Apache, AZ	1958–2010	53	628	29.1	7,241	Y	Y
107	09496000	Corduroy Creek near Mouth near Show Low, AZ	1952–2005	26	206	22.3	6,372	N	N
108	09496500	Carrizo Creek near Show Low, AZ	1952–2010	50	441	22.2	6,329	Y	Y
109	09496600	Cibecue 1 Trib. Carrizo Creek near Show Low, AZ	1959–1971	13	0.1	21.0	5,428	N	N
110	09496700	Cibecue 2 Trib. Carrizo Cr, AZ	1959–1971	13	0.1	20.6	5,221	N	N
111	09497500	Salt River near Chrysotile, AZ	1925–2010	86	2,831	25.1	6,755	Y	Y
112	09497800	Cibecue Creek near Chrysotile, AZ	1960–2010	51	290	23.2	5,743	Y	Y
113	09497900	Cherry Creek near Young, AZ	1964–1977	14	62.2	30.2	5,992	N	N
114	09497980	Cherry Creek near Globe, AZ	1966–2010	44	200	26.8	5,543	Y	Y
115	09498400	Pinal Creek at Inspiration Dam near Globe, AZ	1981–2010	30	195	21.0	4,172	Y	Y
116	09498500	Salt River near Roosevelt, AZ	1914–2010	97	4,289	24.5	6,183	N	N
117	09498501	Pinto Creek below Haunted Canyon near Miami, AZ	1996–2010	15	36.4	24.4	4,416	N	N
118	09498502	Pinto Creek near Miami, AZ	1995–2010	16	102	23.0	4,216	N	Y

Table 1. Table of streamgaging stations in Arizona and western New Mexico with flood-duration flow frequency statistics.—Continued

Map ID	Station ID	Station Name	Period of Record	Number of years of record	Drain-age area (square miles)	Mean annual precipitation (inches)	Mean elevation (feet above NAVD88)	Used in skew analysis	Used in regional regression
119	09498503	South Fork Parker Creek near Roosevelt, AZ	1987–2010	22	1.1	33.5	6,647	Y	Y
120	09498800	Tonto Creek near Gisela, AZ	1966–1975	10	433	27.9	5,536	N	Y
121	09498870	Rye Creek near Gisela, AZ	1967–1985	19	123	22.9	4,294	N	Y
122	09499000	Tonto Creek above Gun Creek near Roosevelt, AZ	1942–2010	69	672	25.9	5,083	Y	Y
123	09502800	Williamson Valley Wash near Paulden, AZ	1966–2010	29	255	16.6	5,136	Y	Y
124	09502900	Del Rio Springs near Chino Valley, AZ	1997–2010	14	39.9	13.0	4,762	N	N
125	09502960	Granite Creek at Prescott, AZ	1996–2010	15	30.2	22.6	5,952	N	Y
126	09503000	Granite Creek near Prescott, AZ	1933–2010	31	39.4	22.4	5,906	Y	Y
127	09503700	Verde River near Paulden, AZ	1964–2010	47	2,149	16.2	5,434	Y	Y
128	09504000	Verde River near Clarkdale, AZ	1916–2010	49	3,143	17.5	5,719	N	N
129	09504420	Oak Creek near Sedona, AZ	1982–2010	29	233	27.1	6,727	N	N
130	09504500	Oak Creek near Cornville, AZ	1941–2010	67	355	24.8	6,108	Y	Y
131	09505200	Wet Beaver Creek near Rimrock, AZ	1962–2010	42	109	25.0	6,549	Y	Y
132	09505250	Red Tank Draw near Rimrock, AZ	1958–1978	21	51.0	24.3	6,065	Y	Y
133	09505300	Rattlesnake Canyon near Rimrock, AZ	1958–1980	23	25.1	25.8	6,451	Y	Y
134	09505350	Dry Beaver Creek near Rimrock, AZ	1961–2010	50	142	25.1	6,191	Y	Y
135	09505800	West Clear Creek near Camp Verde, AZ	1966–2010	45	241	26.1	6,635	Y	Y
136	09506000	Verde River near Camp Verde, AZ	1935–2010	33	4,650	18.9	5,573	N	N
137	09507600	East Verde River near Pine, AZ	1962–1971	10	6.4	31.7	6,396	N	N
138	09507700	Webber Creek above West Fork Webber Creek near Pine, AZ	1960–1974	15	4.8	33.3	7,026	N	Y
139	09507980	East Verde River near Childs, AZ	1962–2010	47	326	26.5	5,246	Y	Y
140	09508300	Wet Bottom Creek near Childs, AZ	1968–2010	43	36.3	24.3	4,918	Y	Y
141	09508500	Verde River below Tangle Creek above Horseshoe Dam, AZ	1946–2010	60	5,499	19.6	5,573	Y	Y
142	09510070	West Fork Sycamore Creek above McFarland Canyon near Sunflower, AZ	1966–1985	12	4.6	31.9	5,443	N	N
143	09510080	West Fork Sycamore Creek near Sunflower, AZ	1962–1974	13	9.8	31.5	5,335	N	Y
144	09510100	East Fork Sycamore Creek near Sunflower, AZ	1962–1985	24	4.5	30.5	5,228	Y	Y
145	09510150	Sycamore Creek near Sunflower, AZ	1962–1976	15	52.4	27.9	4,560	N	Y
146	09510200	Sycamore Creek near Fort McDowell, AZ	1961–2010	50	164	24.3	3,803	Y	Y
147	09512100	Indian Bend Wash at Scottsdale, AZ	1961–1984	23	59.8	10.1	1,432	Y	N

Table 1. Table of streamgaging stations in Arizona and western New Mexico with flood-duration flow frequency statistics.—Continued

Map ID	Station ID	Station Name	Period of Record	Number of years of record	Drainage area (square miles)	Mean annual precipitation (inches)	Mean elevation (feet above NAVD88)	Used in skew analysis	Used in regional regression
148	09512162	Indian Bend Wash at Curry Road Tempe, AZ	1993–2010	18	221	11.6	1,733	N	N
149	09512165	Salt River at Priest Drive near Phoenix, AZ	1995–2010	16	13,285	21.3	5,223	N	N
150	09512200	Salt River Trib. in South Mountain Park Phoenix, AZ	1961–1996	36	1.7	8.8	1,801	N	N
151	09512280	Cave Creek below Cottonwood Creek near Cave Creek, AZ	1981–2010	30	72.8	20.0	3,766	Y	Y
152	09512400	Cave Creek at Phoenix, AZ	1958–1991	34	229	15.1	2,644	N	N
153	09512450	Agua Fria River near Humboldt, AZ	2001–2010	10	175	18.8	5,425	N	N
154	09512500	Agua Fria River near Mayer, AZ	1941–2010	70	585	19.2	4,938	Y	Y
155	09512600	Turkey Creek near Cleator, AZ	1980–1992	13	89.3	21.8	5,267	N	Y
156	09512800	Agua Fria River near Rock Springs, AZ	1971–2010	39	1,111	19.4	4,528	N	N
157	09512860	Humbug Creek near Castle Hot Springs	1984–1994	11	59.9	22.2	3,964	N	Y
158	09513780	New River near Rock Springs, AZ	1966–2010	44	68.4	20.8	3,967	Y	Y
159	09513800	New River at New River, AZ	1961–1982	22	84.7	19.6	3,642	N	N
160	09513835	New River at Bell Road near Peoria	1968–1993	20	186	15.4	2,604	Y	Y
161	09513860	Skunk Creek near Phoenix, AZ	1968–2010	43	65.0	13.9	2,241	Y	Y
162	09513910	New River near Glendale, AZ	1965–1998	14	623	13.8	2,293	N	N
163	09513970	Agua Fria River at Avondale, AZ	1968–1982	14	2,403	16.5	3,309	N	N
164	09515500	Hassayampa River at Box Damsite near Wickenburg, AZ	1947–1982	36	416	19.8	4,535	Y	Y
165	09516500	Hassayampa River near Morristown, AZ	1939–2010	27	796	17.0	3,747	N	N
166	09517000	Hassayampa River near Arlington, AZ	1991–2010	20	1,423	14.0	2,901	Y	N
167	09517490	Centennial Wash at Southern Pacific Railroad Bridge, AZ	1981–2010	25	1,681	8.9	1,862	Y	N
168	09517500	Centennial Wash near Arlington, AZ	1961–1979	19	1,769	8.8	1,817	N	N
169	09520170	Rio Cornez near Ajo, AZ	1968–1978	11	244	8.4	1,928	N	N
170	09535100	San Simon Wash near Pisinimo, AZ	1973–2010	38	579	10.2	2,231	Y	N
171	09535300	Vamori Wash at Kom Vo, AZ	1973–2010	37	1,290	14.5	2,664	Y	N
172	09537200	Leslie Creek near McNeal, AZ	1970–2010	36	78.8	18.2	5,332	Y	N
173 ¹	09537500	Whitewater Draw near Douglas, AZ	1919–2010	56	1,231	15.8	4,745	Y	N

¹Downward trend in streamflow.²Station used in flood volume frequency skew analysis but not in the flood peak skew analysis.

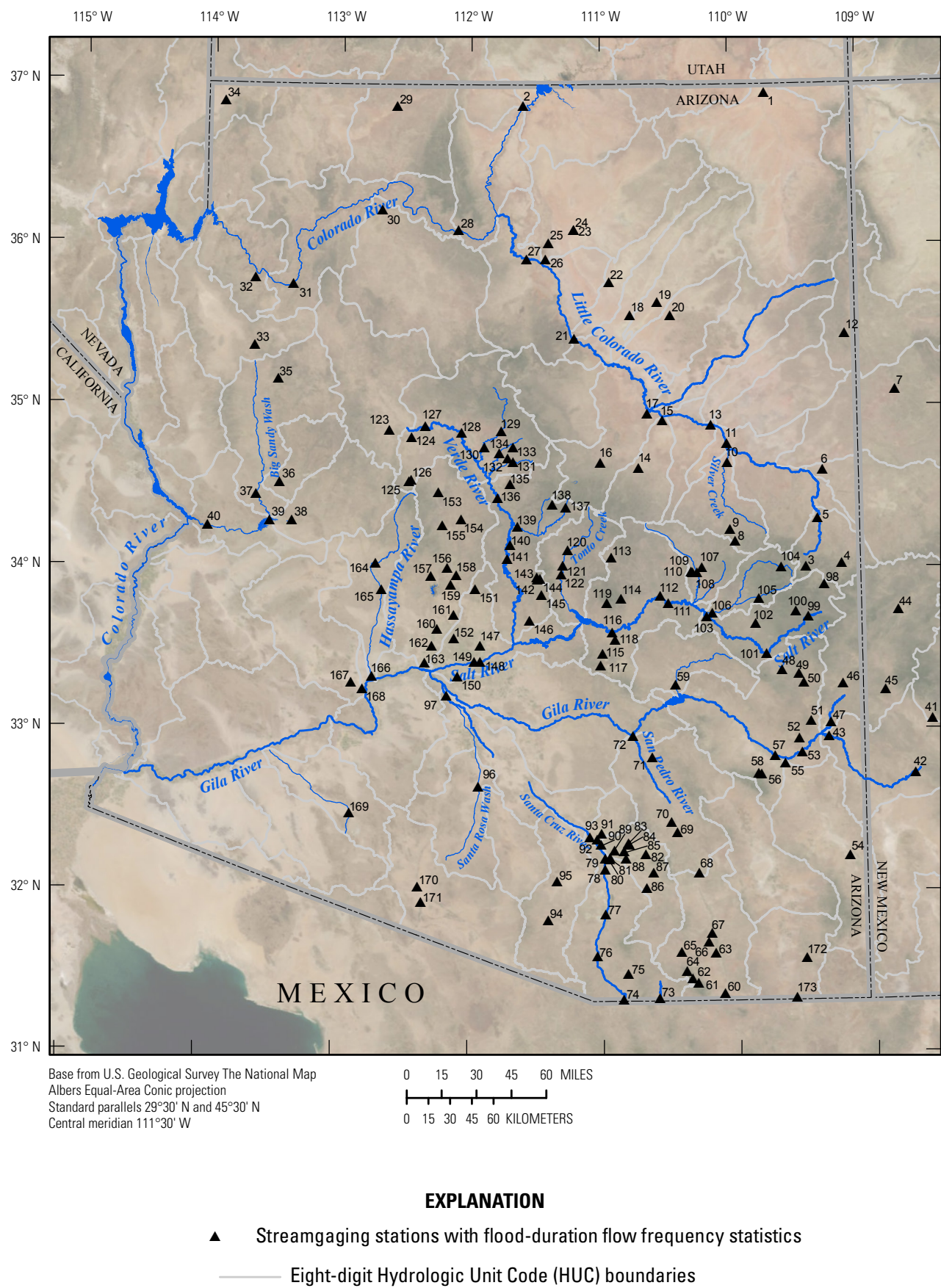


Figure 4. Map of streamgaging stations in Arizona and western New Mexico used in the flood-duration flow frequency analysis.

Table 2. Streamgaging stations in Arizona and western New Mexico with significant trends in 1-, 3-, 7-, 15-, and 30-day flood-duration flows.

[AZ, Arizona; NM, New Mexico; Y, yes; N, no]

Map ID	Station ID	Station name	Flood duration					Used in skew analysis?
			1 day	3 day	7 day	15 day	30 day	
2	09382000	Paria River at Lees Ferry, AZ	x	x	x	x	x	Y
7	09386950	Zuni River above Black Rock Reservoir, NM	x	x	x	x	x	N
63	09471000	San Pedro River at Charleston, AZ	x	x	x	x	x	Y
173	09537500	Whitewater Draw near Douglas, AZ	x	x	x			Y

Table 3. Basin characteristics and data sources considered in the regionalization analysis of flood-duration flow for streamgaging stations in Arizona.

[STATSGO, State Soil Geographic; PRISM, parameter-elevation regressions on independent slopes model; DEM, digital elevation model elevation]

Basin characteristic ID	Basin characteristic description	Data source
DRNAREA	Drainage area/contributing area of the watershed	Calculated from 10-meter DEM
ELEV	Mean basin elevation	10-meter DEM
PRECIP	Mean annual precipitation	PRISM
AUGAVPRE	Mean August precipitation	PRISM
I24H100Y	100-year, 24-hour rainfall intensity	PRISM
SOILPERM	Soil permeability	STATSGO
WATCAP	Soil water capacity	STATSGO

Methods for fitting LP3 moments are described in Bulletin 17B (IACWD, 1982). Research has shown that the computed quantile confidence intervals using Bulletin 17B methods fail to represent the correct uncertainty in the skewness coefficient and that the recommended statistical procedures for computing a regional skewness coefficient are not adequate for estimating the accuracy and precision of the skewness coefficient error (Cohn and others, 2001; Reis and others, 2005). EMA, used with the MGB test, is a revision to the traditional Bulletin 17B estimation methods that explicitly accounts for that method's shortcomings (Cohn and others, 1997; Cohn and others, 2001; England and others, 2003). As with Bulletin 17B, EMA assumes that the LP3 distribution represents the probability distribution function of annual maximum peak flows, except when historical, low-outlier, or censored information exists (Cohn and others, 1997; Griffis and others, 2004). EMA permits the efficient use of interval and threshold data, which most accurately represents historical information, low outliers, and censored flood data (Cohn and others, 1997). Although historical flood records, from first-hand accounts such as newspaper articles or geomorphic evidence such as slackwater deposits, are useful evidence of large floods prior to the systematic record at a particular gage, they provide no information about flood volume. Therefore, the primary benefit of EMA/MBG is to accurately identify and incorporate potentially influential low flows.

Visual inspection of the quantile-probability plots shows that at many streamgaging stations in Arizona, a distinct "dogleg," or shift, exists between a few small events and the remaining data (fig. 5A). These small events in the left-hand

tail of the distribution, termed potentially influential low flows, can have significant influence on the fit of the distribution to the right-hand tail (that is, the largest flood events with lower AEPs). Therefore, a statistical test is useful to determine if these observations are unusually small compared to the rest of the population. Bulletin 17B allows for the identification and removal (by truncation) of potentially influential low flows using the Grubbs-Beck (GB) test (Grubbs and Beck, 1972), but as implemented in the USGS software PeakFQ version 5.2 (Flynn and others, 2006), typically only a single potentially influential low flow is identified. For the streamgaging stations in this analysis, visual inspection of quantile-probability plots suggest that often several low-flow data points depart from the trend of the data and multiple potentially influential low flows should be considered for censoring. The MGB test, a generalization of the GB test, was developed to address this situation (Gotvald and others, 2012). The MGB test differs from an iterative GB test in that it tests a group of potentially influential low flows against the remaining population simultaneously, rather than removing low flows one at a time. Furthermore, in an EMA analysis, these potentially influential low flows are not completely removed from the analysis as in the B17B-GB procedure but instead are recoded as censored data with reduced influence for determining the LP3 moments.

For this report, the annual series of n -day flood-duration flows are assumed independent for each duration, and the MGB test is applied individually to each duration at each station. Nearly half of the streamgaging stations used in the analysis have one or more potentially influential low flows identified using the MGB test (table 4). At some stations, the

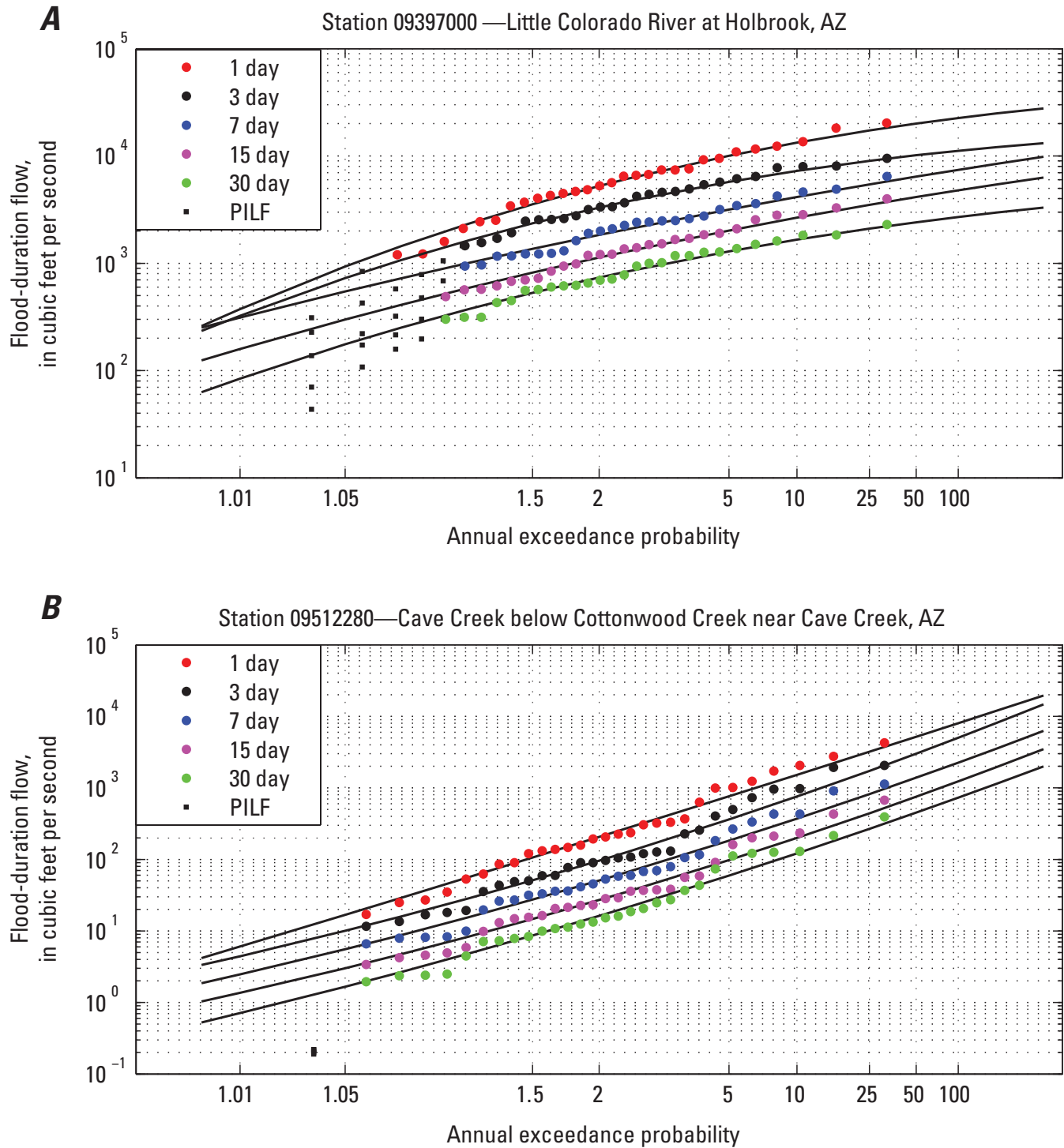


Figure 5. Example quantile-probability plots of LP3 distributions fit to different flood-duration flow data from streamgaging stations in Arizona. *A*, station 09397000, where different numbers of potentially influential low flows and skewness coefficients were identified for each duration; *B*, station 09512280, where no potentially influential low flows were identified and skewness coefficients are similar for all durations. PILF, potentially influential low flow.

number of potentially influential low flows identified is similar for all durations, and these stations tend to have similar skewness coefficients and goodness-of-fit for all durations (fig. 5B). At other stations the number of potentially influential low flows varies differs for the different durations, and the resulting LP3 moments can vary significantly (fig. 5A). However, for each duration, the most statistically probable number of potentially influential low flows are identified and treated as censored data, and therefore each duration was not forced to have the same number of potentially influential low flows.

Regional Skew Analysis and Cross-Correlation Models

The third moment of the LP3 distribution, skew, determines the curvature of the fitted distribution on quantile-probability plots. Negative skewness coefficients result in a concave-down profile and relatively low estimates of low-AEP events; positive skews result in a concave-up profile and estimates of low-AEP events are relatively high. To decrease variability from station to station, most studies using the LP3 distribution combine the at-site skewness coefficient estimate, determined from data at a single streamgaging station, with a regional skewness coefficient estimate. For peak-flow studies, a map of regional skew is presented in Bulletin 17B. As an alternative, many recent USGS studies have used a Bayesian generalized least squares (B-GLS) method to relate regional skew to basin characteristics (Reis and others, 2005; Weaver and others, 2009; Parrett and others, 2011). B-GLS regression considers the precision of the regional skew model, differences in record length between stations, and cross correlation of skewness coefficients between stations. The Bayesian aspect of the B-GLS regression provides an estimate of the precision of the estimated model error variance, a pseudo analysis of variance, and enhanced diagnostic statistics (Griffis and Stedinger, 2007).

An important part of B-GLS regression is estimating the cross-correlation of skew between gages, which can be estimated from the cross-correlation of the annual time series between gages (Martins and Stedinger, 2002; Lamontagne and others, 2012):

$$\rho(\gamma_i, \gamma_j) = \text{Sign}(\rho_{ij}) cf_{ij} |\rho_{ij}|^\kappa, \quad (2)$$

where ρ_{ij} is the cross-correlation of concurrent annual n -day flood-duration flows for two streamgaging stations, $\text{Sign}(\rho_{ij})$ denotes the sign (positive or negative) of the cross-correlation, κ is a constant between 2.8 and 3.3, and cf_{ij} is a factor that accounts for the sample size difference between stations and their concurrent-record length, defined as:

$$cf_{ij} = n_{ij} / \sqrt{(n_{ij} + n_i)(n_{ij} + n_j)}, \quad (3)$$

where n_{ij} is the length of the period of concurrent record, and n_i and n_j are the number of nonconcurrent observations corresponding to sites i and j , respectively.

As part of the B-GLS skew analysis for peak flows (Paretti and others, 2014b), streamgaging stations suitable for skew analysis were identified as those with record lengths greater than 20 years and adequate LP3 flood-frequency fits. Not all of these stations represent unique watershed characteristics; stations may be identified as redundant if one is nested entirely within another and the streamflow data are highly correlated. The drainage-area ratio of a nested station and the nearest downstream station was used to screen for redundancy; in general, a ratio less than or equal to 5 was used to identify redundant station pairs. When redundant pairs were identified, the station with a longer period of record was retained unless the other station was determined to be better represented by the LP3 distribution using goodness-of-fit criteria (Paretti and others, 2014b). Seventy-nine nonredundant stations were identified for the flood-duration flow skew analysis (table 1, fig. 6). Redundant stations not used in the skew analysis are identified in table 1.

Although the cross-correlation of the concurrent annual flood-duration flows between two sites, ρ_{ij} , has high variability, there is a downward trend with increasing distance (fig. 7). Various models relating cross-correlation to various basin characteristics were considered. A logit model using the Fisher z -transform ($Z = \log[(1+r)/(1-r)]$) provided a convenient transformation of the sample correlations r_{ij} from the $(-1, +1)$ range to the $(-\infty, +\infty)$ range. The adopted model for estimating the cross-correlations of concurrent annual peak flow at two stations, which used the distance between basin centroids, D_{ij} , as the only explanatory variable, is

$$Z_{ij} = b_1 + \exp(b_2 + b_3 \times D_{ij}), \quad (4)$$

which is the same form as the cross-correlation model used in the peak-flow analysis (Paretti and others, 2014b). The coefficients b_1 , b_2 , and b_3 vary for each n -day duration (table 5).

The cross-correlation models for n -day flood-duration flows show increasing correlation between gages with longer durations of flood flow, and all durations show greater cross-correlation than the time series of annual peak flows (fig. 8). The greater cross-correlation for longer duration flood events can be explained by Arizona's hydroclimatology. The largest flood peaks in watersheds throughout the State are generally caused by summer convective thunderstorms, which are relatively small in spatial extent and of short duration (Sheppard and others, 2002), affecting only one or a few streamgaging stations. In contrast, long-duration, high-volume flood events are often frontal or tropical storms (Sheppard and others, 2002) that cause widespread runoff across many streamgaging stations.

Pseudo- R^2 was used as a diagnostic statistic for the cross-correlation models. Pseudo- R^2 is a measure of the percent of the variability in the dependent variable (n -day flood-duration

Table 4. Number of potentially influential low flows and at-site skewness coefficients for streamgaging stations in Arizona and western New Mexico used in the flood-duration flow frequency analysis.

Map ID	Station ID	Number of censored potentially influential low flows for indicated flood-duration flow					Sample log-space skewness coefficient for indicated flood-duration flow				
		1 day	3 day	7 day	15 day	30 day	1 day	3 day	7 day	15 day	30 day
1	09379200	0	0	0	0	0	0.063	0.116	0.049	-0.122	-0.044
2	09382000	0	0	0	0	0	0.073	0.130	0.150	0.150	0.134
3	09383400	12	12	0	0	0	-0.132	-0.200	-0.165	-0.153	-0.204
4	09383500	0	0	0	0	0	-0.121	-0.136	-0.102	-0.084	-0.125
5	09384000	6	8	12	12	0	-0.155	-0.223	-0.195	-0.200	-0.294
6	09386250	3	3	3	3	3	-0.052	-0.145	-0.152	-0.149	-0.094
7	09386950	0	0	0	0	0	-0.228	-0.166	-0.039	0.078	0.147
8	09390500	0	22	24	22	28	-0.039	-0.230	-0.214	-0.250	-0.339
9	09392500	0	0	0	0	0	-0.042	-0.058	-0.112	-0.130	-0.240
10	09393500	0	0	0	0	0	-0.075	0.021	0.005	0.018	-0.035
11	09394500	32	32	0	36	40	-0.146	-0.262	-0.198	-0.243	-0.341
12	09395900	2	0	0	5	5	-0.144	-0.236	-0.210	-0.109	-0.131
13	09397000	2	5	5	4	4	-0.208	-0.269	-0.172	-0.203	-0.346
14	09397500	3	3	7	18	19	-0.117	-0.267	-0.246	-0.194	-0.296
15	09398000	0	0	15	19	22	0.016	-0.104	-0.180	-0.198	-0.298
16	09398500	10	11	18	21	22	-0.179	-0.262	-0.141	-0.156	-0.314
17	09399000	1	7	16	22	24	-0.069	-0.208	-0.170	-0.200	-0.260
18	09400562	1	1	4	5	3	-0.131	-0.264	-0.118	-0.124	-0.266
19	09400568	1	1	1	1	1	-0.168	-0.160	-0.135	-0.119	-0.228
20	09400583	2	2	2	2	2	-0.078	-0.070	-0.073	-0.087	-0.139
21	09401000	0	0	2	2	2	-0.061	-0.120	0.009	0.081	0.046
22	09401110	4	5	8	5	4	-0.048	-0.123	-0.179	-0.151	-0.165
23	09401260	0	0	0	0	0	-0.139	-0.255	-0.258	-0.247	-0.326
24	09401280	0	0	0	0	0	-0.116	-0.111	-0.125	-0.118	-0.080
25	09401400	0	0	0	0	0	-0.169	-0.184	-0.192	-0.192	-0.284
26	09401500	1	1	1	1	1	-0.095	-0.138	-0.060	0.047	0.114
27	09402000	0	0	0	0	0	-0.054	-0.045	-0.145	-0.211	-0.257
28	09403000	0	0	0	0	0	0.079	-0.023	-0.102	-0.109	-0.112
29	09403780	5	0	6	7	5	-0.029	-0.269	-0.189	-0.191	-0.275
30	09404110	0	0	0	0	0	-0.018	-0.002	-0.015	-0.035	-0.086
31	09404208	7	0	0	0	0	-0.107	-0.074	-0.028	0.028	0.049
32	09404222	0	0	0	0	0	-0.070	-0.090	-0.075	-0.011	-0.008
33	09404343	1	0	0	1	1	-0.159	-0.240	-0.213	-0.163	-0.246
34	09415000	0	0	0	0	0	0.125	0.204	0.171	0.202	0.237

Table 4. Number of potentially influential low flows and at-site skewness coefficients for streamgaging stations in Arizona and western New Mexico used in the flood-duration flow frequency analysis.—Continued

Map ID	Station ID	Number of censored potentially influential low flows for indicated flood-duration flow					Sample log-space skewness coefficient for indicated flood-duration flow				
		1 day	3 day	7 day	15 day	30 day	1 day	3 day	7 day	15 day	30 day
35	09424200	0	0	0	0	0	-0.123	-0.223	-0.210	-0.236	-0.342
36	09424447	0	0	0	0	0	-0.095	-0.150	-0.145	-0.130	-0.189
37	09424450	18	18	20	20	20	-0.138	-0.154	-0.141	-0.154	-0.139
38	09424900	20	20	20	20	20	-0.153	-0.233	-0.201	-0.207	-0.320
39	09425500	0	0	0	0	0	-0.122	-0.144	-0.101	-0.075	-0.063
40	09426500	0	0	7	0	0	-0.151	-0.220	-0.174	-0.201	-0.275
41	09430500	0	0	0	0	0	0.064	0.056	-0.024	-0.028	-0.034
42	09431500	31	0	0	0	0	-0.169	-0.035	-0.110	-0.078	-0.069
43	09442000	0	0	0	28	29	0.048	-0.013	-0.094	-0.181	-0.310
44	09442680	0	0	0	21	22	0.088	0.026	-0.059	-0.206	-0.111
45	09444000	0	0	41	0	0	0.121	0.131	-0.175	0.082	0.049
46	09444200	0	0	0	0	0	-0.021	-0.017	-0.050	-0.070	-0.098
47	09444500	0	0	0	0	0	0.021	0.043	0.048	0.064	0.042
48	09445500	0	0	0	0	0	0.017	0.060	0.049	0.077	0.098
49	09446000	0	0	0	0	0	-0.005	0.052	0.049	0.086	0.100
50	09446500	0	0	0	0	0	0.024	0.054	0.043	0.088	0.102
51	09447000	0	0	0	0	0	0.085	0.183	0.192	0.270	0.317
52	09447800	0	0	0	0	0	-0.094	-0.027	0.002	0.094	0.144
53	09448500	0	0	0	0	0	0.051	0.066	0.043	0.075	0.069
54	09456000	0	0	0	0	0	-0.132	-0.188	-0.145	-0.120	-0.142
55	09457000	1	0	0	0	0	0.013	-0.123	-0.050	-0.091	-0.099
56	09458200	4	5	6	6	5	-0.059	-0.204	-0.147	-0.052	-0.157
57	09458500	0	0	0	0	0	-0.047	-0.080	-0.129	-0.122	-0.122
58	09460150	3	3	4	11	11	-0.189	-0.279	-0.200	-0.154	-0.189
59	09468500	0	40	40	0	0	0.026	-0.278	-0.236	0.018	0.064
60	09470500	1	1	1	1	1	0.034	0.117	0.017	-0.040	-0.196
61	09470750	0	0	0	0	0	-0.149	-0.226	-0.197	-0.210	-0.313
62	09470800	0	3	3	8	8	-0.144	-0.203	-0.195	-0.123	-0.182
63	09471000	1	1	1	1	1	-0.019	0.034	0.013	-0.016	-0.162
64	09471310	0	0	0	0	0	-0.145	-0.218	-0.181	-0.181	-0.251
65	09471380	1	1	0	0	0	-0.038	-0.095	-0.155	-0.128	-0.070
66	09471400	0	0	0	0	0	-0.084	-0.192	-0.188	-0.200	-0.278
67	09471550	1	1	1	8	8	0.013	-0.030	-0.076	-0.172	-0.250
68	09471800	0	1	1	1	1	-0.122	-0.177	-0.100	-0.121	-0.215

Table 4. Number of potentially influential low flows and at-site skewness coefficients for streamgaging stations in Arizona and western New Mexico used in the flood-duration flow frequency analysis.—Continued

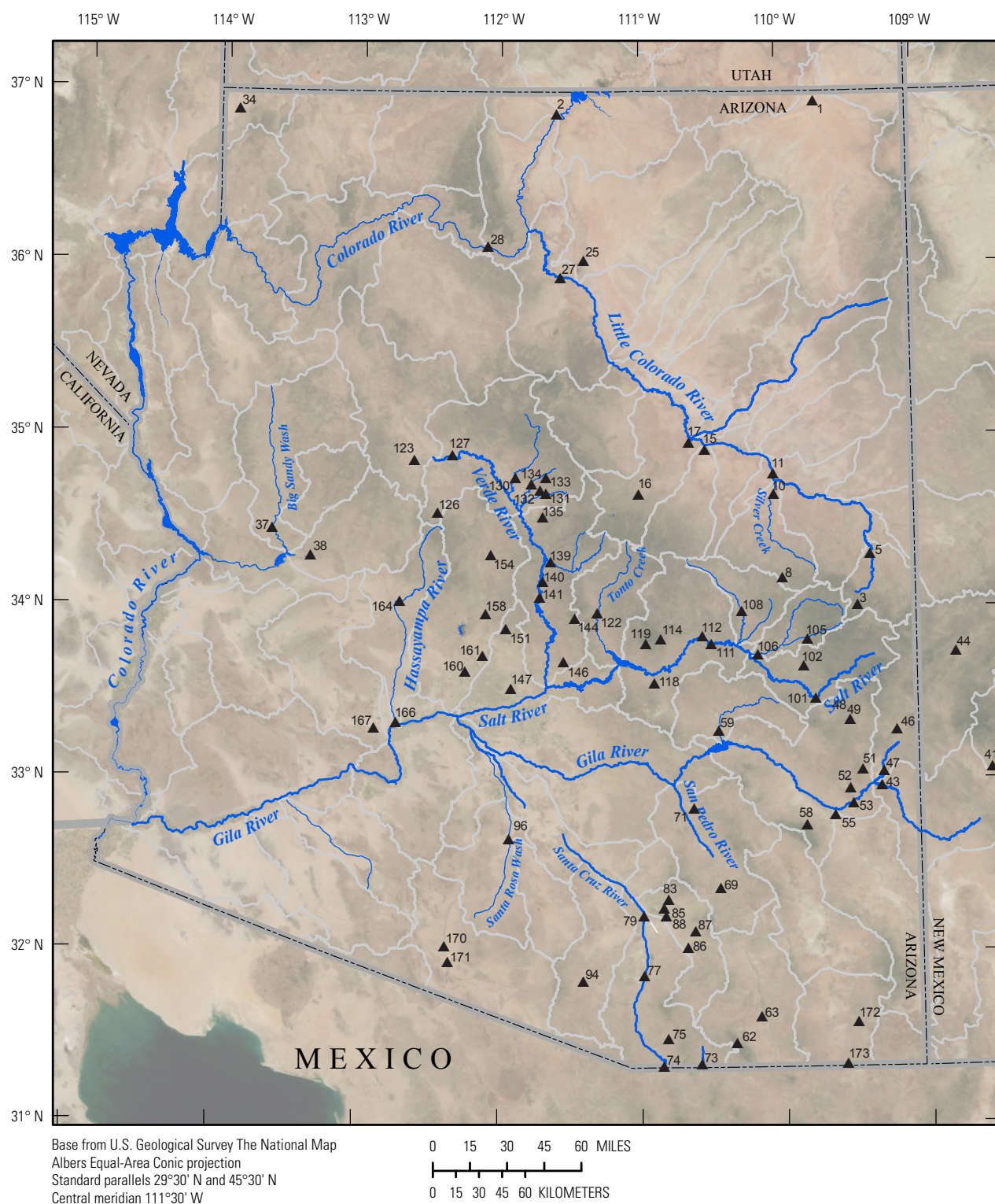
Map ID	Station ID	Number of censored potentially influential low flows for indicated flood-duration flow					Sample log-space skewness coefficient for indicated flood-duration flow				
		1 day	3 day	7 day	15 day	30 day	1 day	3 day	7 day	15 day	30 day
69	09472000	6	5	4	2	2	-0.140	-0.254	-0.242	-0.304	-0.282
70	09472050	1	1	1	1	1	-0.160	-0.207	-0.191	-0.161	-0.323
71	09473000	0	0	0	0	0	0.088	0.147	0.128	0.178	0.222
72	09473500	0	0	0	0	0	-0.038	-0.035	-0.034	-0.033	-0.133
73	09480000	6	6	7	7	0	-0.146	-0.242	-0.178	-0.222	-0.383
74	09480500	0	0	0	0	0	-0.009	0.015	-0.114	-0.183	-0.304
75	09481500	0	0	0	0	0	-0.142	-0.220	-0.164	-0.016	-0.014
76	09481740	0	0	0	0	0	-0.024	-0.055	-0.042	0.001	0.005
77	09482000	0	0	0	0	0	0.036	0.043	0.058	0.070	0.007
78	09482400	0	0	0	0	0	-0.061	-0.084	-0.100	-0.102	-0.173
79	09482500	0	1	0	0	0	-0.140	-0.045	-0.144	-0.138	-0.161
80	09483000	0	0	0	0	0	-0.029	-0.066	-0.092	-0.128	-0.298
81	09483010	0	0	0	0	0	-0.049	-0.067	-0.111	-0.090	-0.034
82	09483100	0	0	0	0	0	-0.055	-0.072	-0.112	-0.057	-0.023
83	09484000	1	1	0	0	0	0.039	-0.020	-0.171	-0.126	-0.226
84	09484200	4	4	4	4	4	-0.109	-0.149	-0.150	-0.028	-0.050
85	09484500	12	8	8	12	7	-0.132	-0.225	-0.187	-0.073	-0.300
86	09484600	2	1	1	1	9	-0.128	-0.160	-0.146	-0.153	-0.312
87	09485000	19	13	15	14	7	-0.138	-0.210	-0.138	-0.179	-0.342
88	09485450	1	0	0	1	1	-0.097	-0.220	-0.199	-0.142	-0.219
89	09485700	2	2	2	2	2	-0.056	-0.076	-0.051	-0.056	-0.103
90	09486055	2	2	2	3	3	-0.092	-0.103	-0.136	-0.149	-0.233
91	09486300	0	0	0	0	0	-0.104	-0.169	-0.161	-0.165	-0.264
92	09486350	0	0	0	0	0	-0.161	-0.181	-0.159	-0.124	-0.208
93	09486500	0	0	0	0	0	-0.057	0.004	-0.026	-0.002	-0.081
94	09486800	0	0	0	0	3	-0.136	-0.222	-0.188	-0.222	0.008
95	09487000	1	1	1	1	1	-0.191	-0.267	-0.273	-0.290	-0.386
96	09488500	0	7	7	7	7	-0.174	-0.174	-0.093	-0.087	-0.079
97	09489000	0	0	0	0	0	0.076	0.109	0.092	0.108	0.062
98	09489070	0	0	0	0	0	-0.147	-0.199	-0.175	-0.176	-0.230
99	09489100	0	0	10	0	0	-0.034	-0.100	-0.169	-0.153	-0.179
100	09489200	10	11	11	11	11	-0.115	-0.208	-0.182	-0.174	-0.227
101	09489500	0	10	11	20	7	-0.204	-0.147	-0.278	-0.208	-0.427
102	09489700	0	0	0	11	11	-0.086	-0.123	-0.159	-0.200	-0.282

Table 4. Number of potentially influential low flows and at-site skewness coefficients for streamgaging stations in Arizona and western New Mexico used in the flood-duration flow frequency analysis.—Continued

Map ID	Station ID	Number of censored potentially influential low flows for indicated flood-duration flow					Sample log-space skewness coefficient for indicated flood-duration flow				
		1 day	3 day	7 day	15 day	30 day	1 day	3 day	7 day	15 day	30 day
103	09490500	0	0	0	10	25	-0.118	-0.173	-0.218	-0.261	-0.246
104	09490800	0	0	0	0	0	-0.024	-0.021	-0.028	-0.025	-0.056
105	09492400	1	1	2	11	19	0.105	0.009	-0.173	-0.269	-0.223
106	09494000	0	1	24	26	26	-0.066	-0.133	-0.197	-0.224	-0.318
107	09496000	0	9	13	12	13	-0.099	-0.224	-0.174	-0.182	-0.259
108	09496500	0	19	19	19	20	-0.093	-0.212	-0.126	-0.169	-0.329
109	09496600	0	0	0	0	0	-0.076	-0.162	-0.111	-0.014	0.005
110	09496700	0	0	0	0	0	-0.026	-0.069	-0.041	-0.063	-0.023
111	09497500	0	0	0	39	41	0.020	0.048	-0.008	-0.133	-0.164
112	09497800	0	0	0	0	0	0.004	0.104	0.118	0.213	0.254
113	09497900	0	0	5	5	0	-0.092	-0.174	-0.098	-0.091	-0.195
114	09497980	9	10	11	11	11	-0.138	-0.268	-0.238	-0.196	-0.260
115	09498400	0	0	0	0	0	-0.053	0.012	0.083	0.123	0.143
116	09498500	0	0	0	0	0	-0.001	0.047	0.049	0.057	-0.038
117	09498501	1	0	0	0	0	-0.077	-0.221	-0.182	-0.164	-0.180
118	09498502	0	0	0	0	0	-0.172	-0.224	-0.181	-0.163	-0.156
119	09498503	2	2	2	2	3	-0.117	-0.145	-0.209	-0.249	-0.285
120	09498800	0	0	0	0	0	-0.132	-0.191	-0.178	-0.111	-0.091
121	09498870	0	0	0	0	0	-0.133	-0.215	-0.201	-0.204	-0.297
122	09499000	30	1	32	17	17	-0.176	-0.288	-0.133	-0.140	-0.245
123	09502800	0	0	0	0	0	-0.161	-0.226	-0.161	-0.139	-0.143
124	09502900	0	0	0	0	0	-0.069	-0.099	-0.064	-0.060	-0.157
125	09502960	0	0	0	0	0	-0.014	-0.012	-0.047	-0.024	-0.108
126	09503000	0	0	0	0	15	-0.044	-0.081	-0.089	-0.047	-0.285
127	09503700	0	0	0	0	0	0.044	0.115	0.157	0.234	0.292
128	09504000	0	0	0	0	0	-0.074	-0.082	-0.069	-0.033	-0.016
129	09504420	0	5	9	10	0	-0.189	-0.234	-0.120	-0.097	-0.226
130	09504500	0	0	18	22	21	-0.083	-0.213	-0.209	-0.240	-0.323
131	09505200	21	15	16	15	21	-0.148	-0.220	-0.115	-0.186	-0.291
132	09505250	5	5	8	7	8	-0.132	-0.166	-0.071	-0.185	-0.064
133	09505300	0	6	6	5	5	-0.194	-0.190	-0.161	-0.166	-0.265
134	09505350	0	11	11	13	16	-0.178	-0.223	-0.212	-0.231	-0.333
135	09505800	18	18	18	20	15	-0.152	-0.216	-0.164	-0.176	-0.346
136	09506000	0	0	0	0	0	-0.064	-0.075	-0.071	-0.038	-0.100

Table 4. Number of potentially influential low flows and at-site skewness coefficients for streamgaging stations in Arizona and western New Mexico used in the flood-duration flow frequency analysis.—Continued

Map ID	Station ID	Number of censored potentially influential low flows for indicated flood-duration flow					Sample log-space skewness coefficient for indicated flood-duration flow				
		1 day	3 day	7 day	15 day	30 day	1 day	3 day	7 day	15 day	30 day
137	09507600	0	0	3	4	4	-0.138	-0.228	-0.105	-0.144	-0.082
138	09507700	0	0	0	0	0	-0.138	-0.221	-0.223	-0.240	-0.267
139	09507980	23	23	0	0	0	-0.160	-0.238	-0.285	-0.233	-0.248
140	09508300	3	3	2	7	20	-0.207	-0.321	-0.324	-0.296	-0.231
141	09508500	0	29	0	22	0	-0.129	-0.201	-0.128	-0.165	-0.060
142	09510070	2	6	6	2	0	-0.140	-0.133	-0.108	-0.174	-0.287
143	09510080	0	0	0	0	0	-0.163	-0.247	-0.214	-0.231	-0.324
144	09510100	0	5	0	7	9	-0.190	-0.259	-0.255	-0.193	-0.273
145	09510150	0	0	0	0	0	-0.069	-0.155	-0.181	-0.196	-0.262
146	09510200	4	10	0	13	4	-0.213	-0.234	-0.335	-0.229	-0.462
147	09512100	3	3	3	3	3	-0.113	-0.208	-0.204	-0.208	-0.301
148	09512162	0	0	0	0	0	-0.103	-0.082	-0.018	0.022	-0.014
149	09512165	2	2	2	2	2	-0.089	-0.133	-0.109	-0.071	-0.075
150	09512200	16	16	16	16	16	-0.131	-0.203	-0.180	-0.189	-0.283
151	09512280	1	1	1	1	1	-0.057	0.041	0.025	0.060	0.040
152	09512400	8	11	8	8	6	-0.123	-0.207	-0.224	-0.235	-0.357
153	09512450	0	0	0	0	0	-0.125	-0.207	-0.178	-0.187	-0.228
154	09512500	0	0	0	0	0	0.080	0.091	0.024	0.079	0.044
155	09512600	1	1	1	1	1	-0.115	-0.141	-0.079	-0.005	0.000
156	09512800	0	0	0	0	0	0.024	0.004	-0.043	-0.051	-0.059
157	09512860	0	0	0	0	0	-0.039	-0.040	-0.038	-0.009	-0.021
158	09513780	10	3	3	3	22	-0.161	-0.331	-0.299	-0.319	-0.318
159	09513800	2	2	2	0	0	-0.199	-0.291	-0.265	-0.286	-0.400
160	09513835	7	7	8	8	8	-0.137	-0.227	-0.170	-0.172	-0.229
161	09513860	14	14	14	14	15	-0.124	-0.225	-0.208	-0.235	-0.285
162	09513910	2	2	2	2	2	-0.053	-0.083	-0.079	-0.050	-0.143
163	09513970	7	7	7	7	7	-0.115	-0.168	-0.141	-0.137	-0.213
164	09515500	11	11	0	0	0	-0.082	-0.192	-0.057	-0.012	0.035
165	09516500	1	5	1	1	1	-0.206	-0.208	-0.255	-0.193	-0.253
166	09517000	0	0	0	0	0	-0.030	0.038	0.017	0.047	0.085
167	09517490	1	12	1	12	6	-0.193	-0.050	-0.295	-0.045	-0.337
168	09517500	4	4	4	4	4	-0.160	-0.230	-0.206	-0.219	-0.326
169	09520170	0	1	0	1	0	-0.154	-0.135	-0.180	-0.099	-0.162
170	09535100	2	2	2	2	2	-0.025	-0.028	-0.075	-0.082	0.001
171	09535300	0	0	0	0	0	0.068	0.069	0.033	-0.059	-0.129
172	09537200	15	15	15	8	0	-0.130	-0.203	-0.183	-0.266	-0.392
173	09537500	17	20	5	7	6	-0.150	-0.174	-0.237	-0.220	-0.231



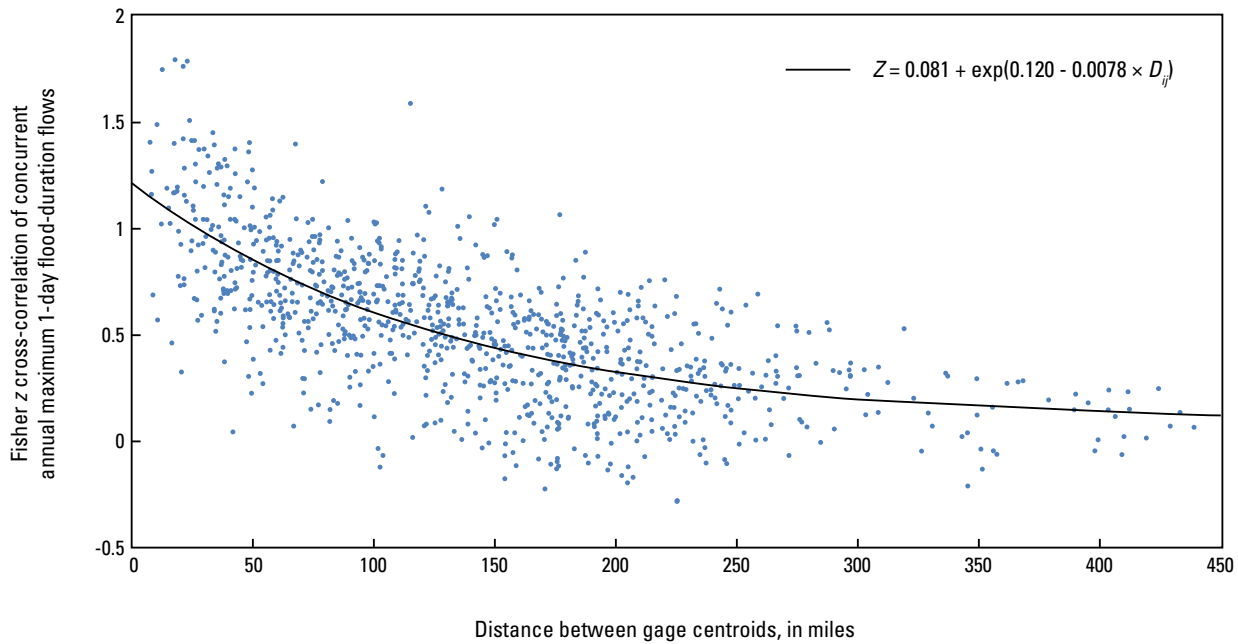
EXPLANATION

- ▲ Streamgaging stations used in regional skew analysis
- Eight-digit Hydrologic Unit Code (HUC) boundaries

Figure 6. Map showing streamgaging stations in Arizona and western New Mexico used in the regional skew analysis.

Table 5. Model coefficients in equation 4 and pseudo- R^2 for the cross-correlation models of annual time series of n -day flood-duration flow for Arizona.

Flood duration	Beta parameters			Pseudo- R^2
	b_1	b_2	b_3	
1 day	0.081	0.120	-0.0078	60 percent
3 day	0.087	0.165	-0.0078	59 percent
7 day	0.128	0.187	-0.0084	57 percent
15 day	0.159	0.194	-0.0085	54 percent
30 day	0.153	0.254	-0.0081	52 percent
Annual peaks	0.11	-0.67	-0.0094	35 percent

**Figure 7.** Scatterplot showing the cross-correlation model for 1-day flood-duration flows for Arizona. Each point represents the correlation of the annual time series between two streamgaging stations. Z , Fisher z cross-correlation; $\exp$, exponential function; D_{ij} , distance between basin centroids.

flow) explained by the regression after removing the effect of time-sampling error, calculated as:

$$R_{pseudo}^2 = 1 - \frac{\sigma_{\delta}^2(k)}{\sigma_{\delta}^2(0)}, \quad (5)$$

where $\sigma_{\delta}^2(k)$ is the model error variance from a regression analysis with k independent variables and $\sigma_{\delta}^2(0)$ is the model error variance from a regression analysis with no independent variables. Pseudo- R^2 (table 5) decreases with increasing length of flood-duration flow but is consistently higher for all durations than for the cross-correlation model developed for annual peaks (Paretti and others, 2014b).

The significant cross-correlation between stations complicates the GLS regression, but the relatively low precision of the cross-correlation model doesn't justify the sophisticated weighting matrix generated by B-GLS. Therefore, an alternative procedure was used (presented in detail in Lamontagne

and others, 2012). First, an ordinary least squares (OLS) analysis is used to develop an initial regional-skew model, which is then used to generate a regional-skew estimate for each site. That OLS skewness coefficient estimate is used to compute the sampling variance of each skew estimator for use in a WLS analysis. Then, WLS is used to generate the estimator of the regional-skew model parameters. Finally, B-GLS is used to estimate the precision of that parameter estimator and to estimate the model-error variance. The three-step procedure was repeated to develop a regional-skew model and the associated error analysis for each flood duration.

The at-site skew for nearly all of the streamgaging stations in the study is negative; for all durations, less than 20 percent of stations have positive skew (table 4). Several basin characteristics were tested as explanatory variables in the B-GLS skew regression, including location (latitude and longitude), drainage area, mean elevation, mean annual precipitation, August mean precipitation (representative of summer

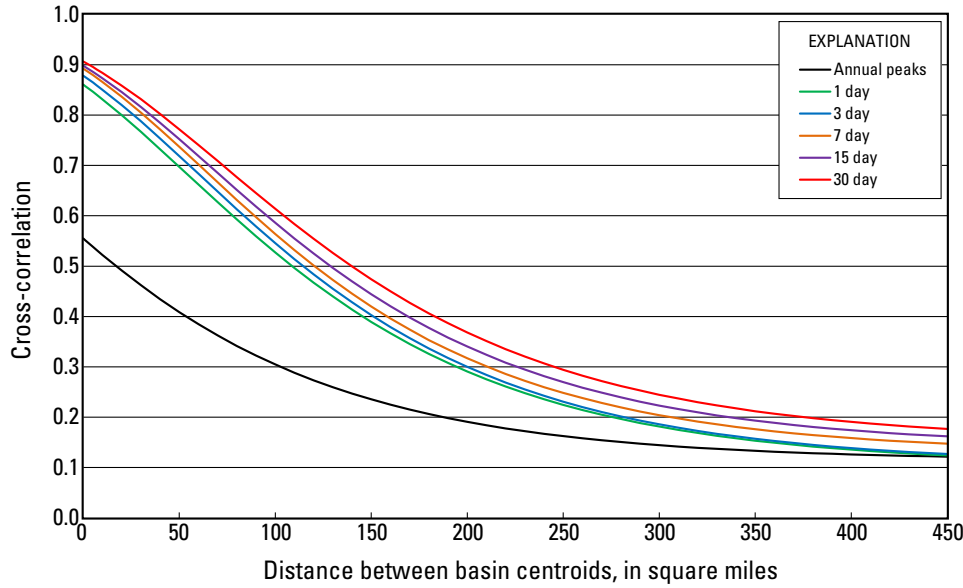


Figure 8. Graph showing cross-correlation models used in the regional skew and regional regression analyses for flood-duration flow for Arizona. The cross-correlation model for annual peak flows (Paretti and others, 2014b) is shown for reference.

convective thunderstorm runoff), 24-hour 100-year precipitation intensity, soil permeability, and soil water capacity. Maps of station skewness coefficients were also created to evaluate spatial patterns. None of these explanatory variables significantly improved skew estimates as compared to a constant model (the weighted average skew at all stations). Regional skew is highest for 1-day flood-duration flow, most negative for 30-day flood-duration flow, and the intervening 3-, 7-, and 15-day flood-duration flow skewness coefficients are intermediate, although not in a systematic manner (table 6). For comparison, the constant skewness coefficient for annual flood peaks is -0.09 (Paretti and others, 2014b) and the average variance of prediction (AVP) is 0.079 . AVP is a diagnostic statistic that reflects both the underlying model error variance, σ_δ^2 , and the sampling variance:

$$AVP = \sigma_\delta^2 + \frac{1}{n} \sum_{p=1}^n \mathbf{x}_p (\mathbf{X}^T \mathbf{\Lambda}^{-1} \mathbf{X})^{-1} \mathbf{x}_p^T \quad (6)$$

where $\mathbf{x}_p$ is a vector of independent variables at the p th gage, and $\mathbf{X}$ and $\mathbf{\Lambda}$ are the design matrix and covariance matrix from the regression analysis, respectively. AVP is used to weight the regional skewness coefficient when combined with station skewness coefficient to determine a weighted skewness coefficient (Reis and others, 2005):

$$\gamma_w = \frac{AVP_{new} \gamma_s + MSE_s \gamma_r}{AVP_{new} + MSE_s} \quad (7)$$

where γ_w , γ_r , and γ_s are the weighted, regional, and station skewness coefficients, respectively, and MSE_s is the estimated mean square error of the station skewness coefficient. Further details of the skew analysis in Arizona are found in Paretti and others (2014b).

Table 6. Regional skewness coefficients and their average variance of prediction, by duration, for flood-duration flows in Arizona.

Flood duration	Skewness coefficient	Average variance of prediction
1 day	-0.103	0.091
3 day	-0.155	0.158
7 day	-0.133	0.133
15 day	-0.130	0.169
30 day	-0.209	0.259
Annual peaks	-0.090	0.079

Regionalization

Definition of Regions

The spatial extent and density of streamgaging stations with adequate data for the n -day flood-duration flow frequency analysis is smaller than that of the stations used to analyze peak flows (Paretti and others, 2014b), and, if the same regions were used (high elevation, Colorado Plateau, western Basin and Range, central highlands, and southeastern Basin and Range) there would be an inadequate number of gages in each region to define regression equations. Therefore, three alternative regions were tested:

- A single statewide region using all of the streamgaging stations with adequate data.
- A single statewide region but with the furthest outlying gages removed. This region includes streamgaging stations in the Colorado Plateau, central highlands, and

southeastern Basin and Range regions (Paretti and others, 2014b).

- A single region comprising the central highland region only (Paretti and others, 2014b) with the addition of streamgaging stations lying just outside this region to the southwest.

Two statistics were used to evaluate the alternative region definitions—(1) average standard error of prediction ($SE_{p,ave}$) and (2) pseudo- R^2 (equation 5). $SE_{p,ave}$ is an alternative way to express AVP as a percent of the predicted flood volume and is simply a transformation of units:

$$SE_{p,ave} = 100 \{ e^{(\ln 10)^2 AVP} - 1 \}^{1/2}. \quad (8)$$

Regression diagnostic statistics were significantly better using the third alternative, which is comprised of 85 stations (table 7). Therefore, regression equations were only developed for the central highland region. Outside this region, of the stations with record lengths and statistical fits adequate for regression analysis, only 30 stations are located to the south and 22 stations to the north. These were considered insufficient to be standalone regions, and regression equations in these areas are not presented. Flood-volume frequency estimates at streamgaging stations outside of the central highland region, where the regression equations are not applicable, are presented in appendix 1 (note that appendixes 1–3 are available online only at <http://pubs.usgs.gov/of/2014/5109>).

The basin characteristics of streamgaging stations used in the regression analysis for the central highland region vary widely as a result of the diverse topography and climatology in this area (table 8). The drainage areas of streamgaging stations used in the regression analysis vary over four orders of magnitude (fig. 10), but most are between 25 and 2,500 square miles. Basin centroid elevations also vary widely, from 2,240 foot elevation (Skunk Creek near Phoenix, station 09513860) to 9,520 foot elevation (North Fork White River near Greer, station 09490800). Mean basin precipitation varies from 13.9 inches in the low desert to 36.7 inches at high elevations, part of which is typically from snowfall (fig. 2B). The regression equations are only valid within the parameter ranges of these basin characteristics (table 8) and the region of the applicable streamgaging stations (fig. 9).

Model Development

Regression analysis was used to relate the quantile AEP estimates at each streamgaging station to basin characteristics, such as drainage area or mean annual rainfall. If an explanatory relation exists, the regression equations can then be used to make predictions at ungaged sites where no flood-duration flow data are available. Initially, seven basin characteristics considered to be most closely related to flood-duration flow (table 3) were tested for significance using OLS regression and

the weighted-multiple-linear regression program WREG (Eng and others, 2009) by evaluating the T value statistic for each characteristic,

$$T \text{ value} = \frac{\beta_k}{(Var\beta_k)^{1/2}}, \quad (9)$$

where β_k is the predicted coefficient of the k th basin characteristic and $Var\beta_k$ is the covariance of taken from the covariance matrix of the regression parameters. The T value statistic is assumed to follow a Student's t -distribution, and the probability, or p -value, that the null hypothesis (H_0 , the model parameter is equal to zero) should be rejected can be calculated. Regression parameters with p -values less than 0.05 are deemed to be significant and are included in the final regression equations.

One assumption of regression analyses is that the explanatory variables are independent. If multicollinearity (correlation between variables) exists, model error may be underestimated. Variance inflation factor (VIF) was used to screen for multicollinearity (Johnston, 1972). Although increasing elevation generally corresponds to increasing precipitation in Arizona (fig. 10), the VIF for these two variables is 1.7, well below the commonly used threshold of 10 (Kroll and Song, 2013), and both were retained in the regression equations.

The final regression equations (table 9) use GLS regression to account for the cross-correlation between gages. As with the GLS regression used for the skew analysis, covariance matrices that account for cross-correlation of annual flood-duration flows are needed for each AEP for the regional regression analysis. The same cross-correlation models (table 5) were used for both.

For this study, the log of drainage area in square miles (DRNAREA), the log of mean annual precipitation in inches (PRECIP), and mean basin elevation in feet divided by 1,000 (ELEV) were determined to be significant in the final equations identified using GLS regression. ELEV was divided by 1,000 so that regression coefficients were smaller and more easily calculated (Eng and others, 2009). For most durations and AEPs, all three explanatory variables are included in the regression equations, but mean basin elevation is not included for some 50-percent AEP equations.

Model Diagnostics and Verification

Two statistics, leverage and influence, serve as regression model diagnostics. Leverage is calculated from the covariance matrix used in the GLS regression analysis. It represents the potential impact a single streamgaging station has on the regression and is primarily a factor of how “unique” a station is. If basin characteristics at a particular station are far from the mean of the remaining stations, it can potentially, but not necessarily, have a dominant effect on the regression. Such a station is said to have high leverage. Alternatively, influence measures the actual effect a particular station has on the

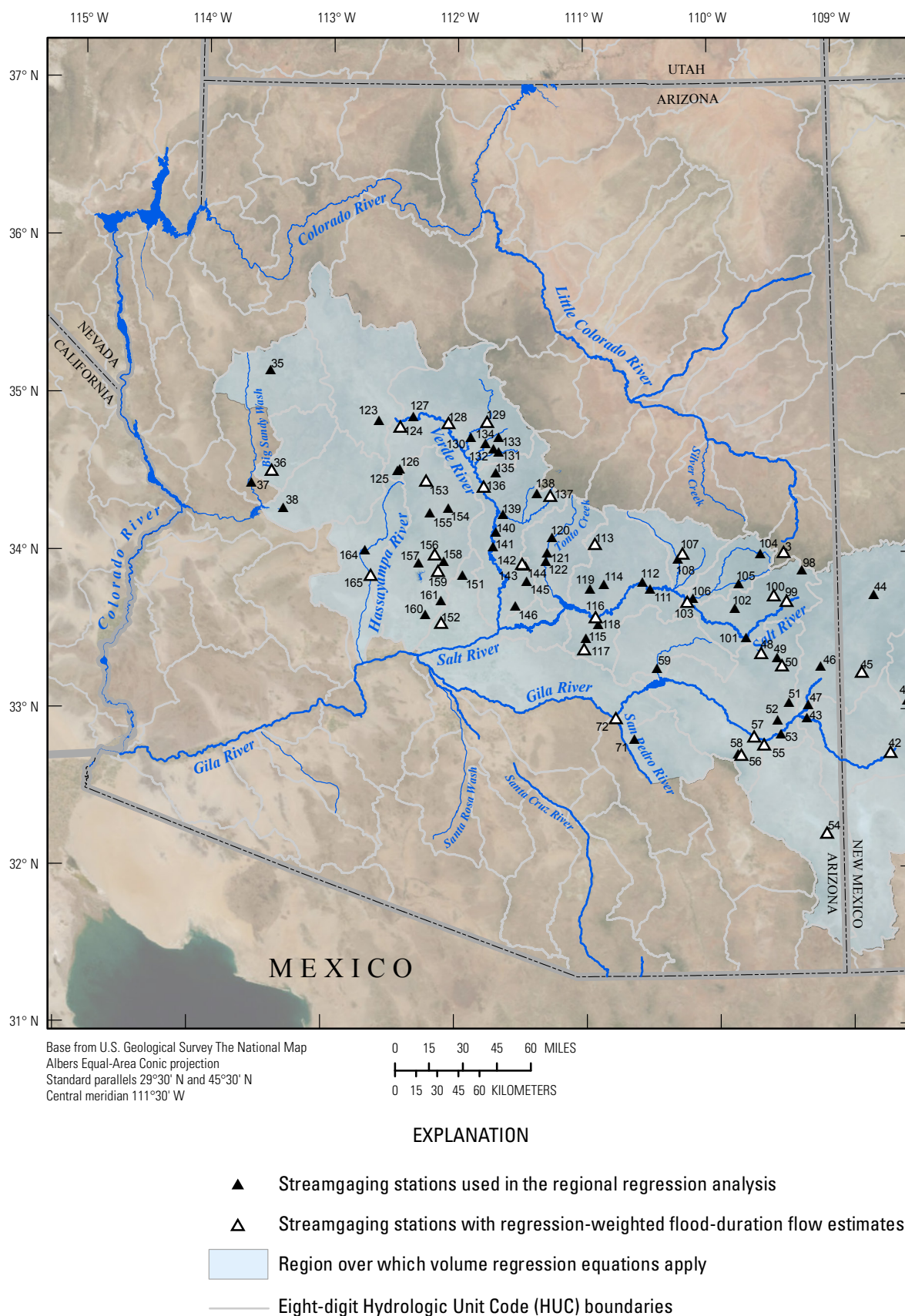


Figure 9. Map showing the region in Arizona and western New Mexico for which regression equations are developed and streamgaging stations used in the analysis of flood-duration flow in Arizona.

Table 7. Regression diagnostic statistics for three regionalization schemes used for estimating flood duration-flows in Arizona.

[Pct. AEP, percent annual exceedance probability; Avg. SEP, average standard error of prediction]

All gages			Single statewide region, select gages		Central highland region only	
1 day						
Pct. AEP	Avg. SEP	Pseudo- R^2	Avg. SEP	Pseudo- R^2	Avg. SEP	Pseudo- R^2
50	70.9	85.2	72.2	86.2	50.8	90.4
20	65.9	86.0	67.2	87.0	43.7	92.1
10	68.4	85.0	69.9	86.1	40.6	92.7
4	75.3	82.7	75.9	84.3	37.0	93.5
2	81.1	80.9	81.2	82.8	36.4	93.6
1	87.3	79.1	87.8	81.0	36.7	93.5
0.5	93.9	77.2	93.9	79.4	37.8	93.1
0.2	102.5	74.9	102.8	77.2	37.7	93.1
3 day						
Pct. AEP	Avg. SEP	Pseudo- R^2	Avg. SEP	Pseudo- R^2	Avg. SEP	Pseudo- R^2
50	71.7	85.6	70.9	87.2	52.3	90.0
20	65.8	86.8	66.7	87.8	43.6	92.4
10	66.5	86.4	68.3	87.2	40.4	93.3
4	69.4	85.3	71.6	86.2	40.3	93.2
2	72.6	84.3	75.0	85.2	39.3	93.6
1	76.8	83.0	78.7	84.2	39.9	93.5
0.5	81.1	81.8	82.6	83.2	40.6	93.4
0.2	86.3	80.4	88.1	81.8	35.3	95.2
7 day						
Pct. AEP	Avg. SEP	Pseudo- R^2	Avg. SEP	Pseudo- R^2	Avg. SEP	Pseudo- R^2
50	72.4	85.9	71.5	87.4	52.9	90.2
20	65.7	87.1	66.3	88.1	42.7	92.6
10	66.0	86.6	66.7	87.7	36.1	94.5
4	68.5	85.6	69.5	86.6	34.9	94.7
2	72.1	84.3	72.6	85.6	32.6	95.4
1	75.3	83.3	76.8	84.4	33.1	95.4
0.5	79.4	82.0	80.4	83.4	33.6	95.3
0.2	85.0	80.3	86.4	81.7	35.2	95.0
15 day						
Pct. AEP	Avg. SEP	Pseudo- R^2	Avg. SEP	Pseudo- R^2	Avg. SEP	Pseudo- R^2
50	75.0	85.7	73.1	87.4	51.3	91.2
20	67.0	86.7	66.6	88.0	40.4	93.6
10	65.9	86.9	66.4	87.9	36.4	94.5
4	68.2	85.8	69.6	86.6	34.9	94.7
2	71.6	84.6	72.4	85.6	32.6	95.4
1	74.5	83.6	75.5	84.6	32.0	95.6
0.5	78.4	82.2	79.7	83.3	31.6	95.8
0.2	75.9	82.3	78.1	83.2	28.7	96.9

Table 7. Regression diagnostic statistics for three regionalization schemes used for estimating flood duration-flows in Arizona.—Continued

All gages			Single statewide region, select gages		Central highland region only	
30 day						
Pct. AEP	Avg. SEP	Pseudo- R^2	Avg. SEP	Pseudo- R^2	Avg. SEP	Pseudo- R^2
50	76.9	85.7	72.9	88.2	49.5	92.0
20	68.4	86.8	68.0	88.0	42.7	92.9
10	68.1	86.7	70.4	86.7	38.2	94.1
4	70.0	85.6	70.4	86.6	36.6	94.2
2	71.8	84.9	72.3	85.9	34.2	94.9
1	74.4	83.8	75.0	84.8	32.6	95.4
0.5	77.3	82.8	78.0	83.8	31.2	95.9
0.2	81.4	81.3	81.5	82.6	27.1	97.1

estimated regression parameter values (Eng and others, 2009). Stations may have high leverage and high influence, high leverage and low influence, or low leverage and high influence. For this study, influence is calculated using a generalized Cook's D value (Eng and others, 2009). High influence may indicate an error in either the station record or basin characteristics at a station, but if no such errors exist, it alone is not sufficient justification for removing a station from the regression analysis.

Streamgaging stations used in the regression equations generally show uniform influence and leverage, although there are consistently a few stations with relatively high influence (appendix 3). The stations with high influence are not consistent across all flood durations or AEPs. Only two stations, Cibecue No. 1, Tributary to Carrizo Creek, near Show Low, Arizona (station 09496600), and Cibecue No. 2, Tributary to Carrizo Creek, near Show Low, Arizona (station 09496700), were completely removed from the regression analysis because they showed large influence. The watershed area for both of these stations, 0.1 square miles, is much smaller than any other stations in the regression, and therefore they have undue influence on the regression results for other small watersheds. The next largest watershed area, 1.1 square miles at South Fork Parker Creek near Roosevelt, Arizona (station 09498503), should be considered the lower limit of applicability of the regional regression equations. One or two other stations, as shown in appendix 3, were selectively removed from the regression for individual n -day flood-duration flows and (or) AEPs. These stations were determined to have poor quantile predictions, based on the quantile-probability plots, most often because of short record lengths.

The final regression equation exponents for each basin characteristic show that flood volume increases with increasing DRNAREA and PRECIP, and decreases with ELEV (table 9). The relative importance of drainage area and precipitation generally is higher for flood volumes with higher AEPs, and decreases with decreasing AEP. Conversely, the relative importance of elevation increases with decreasing AEP. AVP for the regression equations is generally low, ranging from

27 percent to 53 percent (table 10). The average for all n -day flood-duration flows and all AEPs is 38 percent. For comparison, AVP for the central highland region regression equations in the peak flow analysis (Paretti and others, 2014b) ranges from 57 to 91 percent. The lower AVP values for this study reflect decreased variability in n -day flood-duration flows as compared to instantaneous peak flow. Pseudo- R^2 can range from 0 to 100, with higher values indicating better performance. As with AVP , the Pseudo- R^2 values for the regression equations in this study, which range from 90.0 to 97.1 percent (table 10), indicate relatively good model performance. Model performance improves slightly for longer duration n -day flood-duration flows, as indicated by lower AVP and higher Pseudo- R^2 .

The following limitations apply when using the final regional regression equations:

1. Applying the equations to sites on streams having explanatory variables outside the ranges of those used in this study (table 8) may result in prediction errors that are considerably greater than those indicated by the standard error of prediction percentages listed in table 10.
2. The methods are not appropriate (or applicable) for sites where flood-duration flows are affected substantially by flow regulation.
3. The methods are not appropriate (or applicable) for streams in urban areas with substantial impervious area unless the effects of urbanization are deemed insignificant.

Weighting Estimates at Streamgaging Stations

Flood-frequency estimates at a streamgaging station, particularly stations with short records, can be improved by taking the weighted average of the station estimate and the estimate from the regional regression equations (Cohn and others, 2012). The weighting is inversely proportional to the

Table 8. Statistics of the basin characteristics used for the regression equations in estimating flood-duration flow in Arizona.

Characteristic	Minimum	Maximum	Mean	Median
Drainage area, square miles	1.1	7,888.3	742.3	195.4
Basin centroid elevation, feet	2,240.5	9,520.3	5,750.3	5,543.0
Mean annual precipitation, inches	13.9	36.7	23.6	22.7

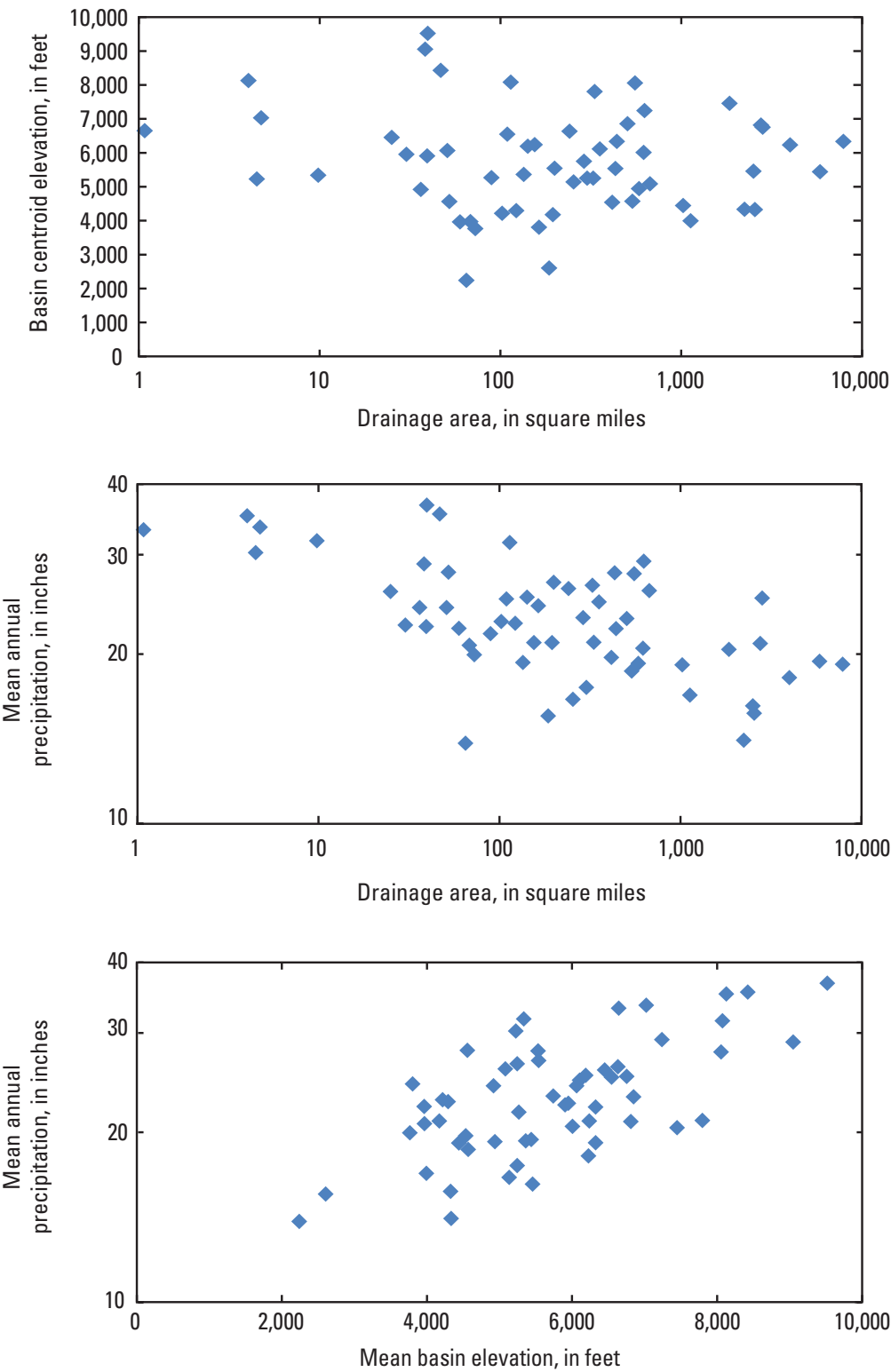


Figure 10. Scatterplots showing joint distributions of basin characteristics used for estimating flood-duration flow for Arizona.

Table 9. Regression equations for 1-, 3-, 7-, 15-, and 30-day flood duration flows for the central highland region in Arizona (see fig. 9).

[Pct. AEP, percent annual exceedance probability; DRNAREA, drainage area in square miles; PRECIP, mean annual precipitation in inches. ELEV, mean basin elevation in feet]

Pct. AEP	Regression equation
1 day	
50	$0.00759 (DRNAREA)^{0.882} (PRECIP)^{2.454} 10^{(-0.095 * ELEV/1,000)}$
20	$0.0692 (DRNAREA)^{0.836} (PRECIP)^{2.310} 10^{(-0.128 * ELEV/1,000)}$
10	$0.189 (DRNAREA)^{0.808} (PRECIP)^{2.233} 10^{(-0.131 * ELEV/1,000)}$
4	$0.240 (DRNAREA)^{0.781} (PRECIP)^{2.422} 10^{(-0.136 * ELEV/1,000)}$
2	$0.619 (DRNAREA)^{0.765} (PRECIP)^{2.278} 10^{(-0.138 * ELEV/1,000)}$
1	$1.50 (DRNAREA)^{0.751} (PRECIP)^{2.132} 10^{(-0.139 * ELEV/1,000)}$
0.5	$3.44 (DRNAREA)^{0.739} (PRECIP)^{1.988} 10^{(-0.140 * ELEV/1,000)}$
0.2	$30.1 (DRNAREA)^{0.700} (PRECIP)^{1.503} 10^{(-0.144 * ELEV/1,000)}$
3 day	
50	$0.00597 (DRNAREA)^{0.875} (PRECIP)^{1.978}$
20	$0.0127 (DRNAREA)^{0.868} (PRECIP)^{2.516} 10^{(-0.101 * ELEV/1,000)}$
10	$0.0524 (DRNAREA)^{0.847} (PRECIP)^{2.360} 10^{(-0.121 * ELEV/1,000)}$
4	$0.173 (DRNAREA)^{0.826} (PRECIP)^{2.285} 10^{(-0.144 * ELEV/1,000)}$
2	$0.568 (DRNAREA)^{0.812} (PRECIP)^{2.081} 10^{(-0.152 * ELEV/1,000)}$
1	$1.68 (DRNAREA)^{0.800} (PRECIP)^{1.882} 10^{(-0.158 * ELEV/1,000)}$
0.5	$4.61 (DRNAREA)^{0.790} (PRECIP)^{1.688} 10^{(-0.163 * ELEV/1,000)}$
0.2	$23.6 (DRNAREA)^{0.753} (PRECIP)^{1.365} 10^{(-0.165 * ELEV/1,000)}$
7 day	
50	$0.000538 (DRNAREA)^{0.916} (PRECIP)^{2.527}$
20	$0.00314 (DRNAREA)^{0.877} (PRECIP)^{2.669} 10^{(-0.074 * ELEV/1,000)}$
10	$0.00820 (DRNAREA)^{0.871} (PRECIP)^{2.719} 10^{(-0.118 * ELEV/1,000)}$
4	$0.0267 (DRNAREA)^{0.847} (PRECIP)^{2.672} 10^{(-0.147 * ELEV/1,000)}$
2	$0.180 (DRNAREA)^{0.816} (PRECIP)^{2.288} 10^{(-0.161 * ELEV/1,000)}$
1	$0.298 (DRNAREA)^{0.816} (PRECIP)^{2.246} 10^{(-0.168 * ELEV/1,000)}$
0.5	$0.877 (DRNAREA)^{0.803} (PRECIP)^{2.041} 10^{(-0.175 * ELEV/1,000)}$
0.2	$3.24 (DRNAREA)^{0.788} (PRECIP)^{1.787} 10^{(-0.183 * ELEV/1,000)}$
15 day	
50	$0.0000440 (DRNAREA)^{0.958} (PRECIP)^{3.121}$
20	$0.000508 (DRNAREA)^{0.908} (PRECIP)^{3.006} 10^{(-0.065 * ELEV/1,000)}$
10	$0.00209 (DRNAREA)^{0.884} (PRECIP)^{2.880} 10^{(-0.094 * ELEV/1,000)}$
4	$0.00652 (DRNAREA)^{0.860} (PRECIP)^{2.865} 10^{(-0.129 * ELEV/1,000)}$
2	$0.0217 (DRNAREA)^{0.844} (PRECIP)^{2.678} 10^{(-0.144 * ELEV/1,000)}$
1	$0.0668 (DRNAREA)^{0.829} (PRECIP)^{2.490} 10^{(-0.157 * ELEV/1,000)}$
0.5	$0.192 (DRNAREA)^{0.816} (PRECIP)^{2.305} 10^{(-0.168 * ELEV/1,000)}$
0.2	$1.20 (DRNAREA)^{0.808} (PRECIP)^{1.857} 10^{(-0.172 * ELEV/1,000)}$
30 day	
50	$0.00000789 (DRNAREA)^{0.978} (PRECIP)^{3.519}$
20	$0.000512 (DRNAREA)^{0.889} (PRECIP)^{2.637}$
10	$0.000361 (DRNAREA)^{0.903} (PRECIP)^{3.208} 10^{(-0.078 * ELEV/1,000)}$
4	$0.000897 (DRNAREA)^{0.882} (PRECIP)^{3.255} 10^{(-0.113 * ELEV/1,000)}$
2	$0.00261 (DRNAREA)^{0.868} (PRECIP)^{3.103} 10^{(-0.129 * ELEV/1,000)}$
1	$0.00716 (DRNAREA)^{0.855} (PRECIP)^{2.942} 10^{(-0.141 * ELEV/1,000)}$
0.5	$0.0187 (DRNAREA)^{0.843} (PRECIP)^{2.778} 10^{(-0.152 * ELEV/1,000)}$
0.2	$0.111 (DRNAREA)^{0.837} (PRECIP)^{2.327} 10^{(-0.154 * ELEV/1,000)}$

Table 10. Average variance of prediction, average standard error of prediction, and pseudo- R^2 for the regression equations used to predict 1-, 3-, 7-, 15-, and 30-day flood-duration flow in the central highland region of Arizona.

[Pct. AEP, percent annual exceedance probability; AVP, average variance of prediction; Avg. SEP, average standard error of prediction]

variance of the regression and station estimates (VP_r and VP_s , respectively, in log units) so that estimates with greater uncertainty have less weight in the weighted average. The variance of the regression estimate is calculated differently depending on if the site was used to develop the regional regression equations. For sites not used in the regional regression equations, VP_r is equivalent to AVP and calculated using equation 6. For individual sites that are used in the regional regression equations, VP_r is:

$$VP_r = \sigma_o^2 + \mathbf{x}_i (\mathbf{X}^T \mathbf{\Lambda}^{-1} \mathbf{X})^{-1} \mathbf{x}_i^T \quad (10)$$

The matrix is provided as output from WREG. A first-order approximation of VP_s is output for each AEP from the PeakFQ software used to fit the LP3 distribution (Cohn and others, 2001). The weighted average for a particular AEP is then:

$$\log(\hat{Q}) = \frac{VP_r \times \log(Q_s) + VP_s \times \log(Q_r)}{VP_r + VP_s} \quad (11)$$

For the stations within the central highland region for which the regional regression equations apply (fig. 9, table 8), the station estimate, regression estimate, and weighted estimates of discharge for varying AEPs are presented in appendix 1. The variance of prediction associated with the weighted estimate, VP_w , is computed as:

$$VP_w = \frac{VP_s VP_r}{VP_s + VP_r} \quad (12)$$

VP estimates are given in appendix 2 for the station estimate, regression estimate, and weighted estimate of the predicted n -day flood-duration flows.

Estimates Near Streamgaging Stations on the Same Stream

Within the central highland region, if an ungaged site is near an existing streamgaging station for which flood-duration flow frequency statistics have been calculated, a weighted average flood-duration flow may be calculated that incorporates that station explicitly, rather than using only the regression equations (Ries and Crouse, 2002). Generally, “near” is defined as having a drainage area between 50 and 150 percent of that at the streamgaging station. First, the estimated

Pct. AEP	AVP (log units)	Avg. SEP (percent)	Pseudo- R^2 (percent)
1 day			
50	0.043	0.043	90.4
20	0.033	0.033	92.1
10	0.029	0.029	92.7
4	0.024	0.024	93.5
2	0.024	0.024	93.6
1	0.024	0.024	93.5
0.5	0.025	0.025	93.1
0.2	0.025	0.025	93.1
3 day			
50	0.046	52.3	90.0
20	0.033	43.6	92.4
10	0.028	40.4	93.3
4	0.028	40.3	93.2
2	0.027	39.3	93.6
1	0.028	39.9	93.5
0.5	0.029	40.6	93.4
0.2	0.022	35.3	95.2
7 day			
50	0.047	52.9	90.2
20	0.032	42.7	92.6
10	0.023	36.1	94.5
4	0.022	34.9	94.7
2	0.019	32.6	95.4
1	0.020	33.1	95.4
0.5	0.020	33.6	95.3
0.2	0.022	35.2	95.0
15 day			
50	0.044	51.3	91.2
20	0.029	40.4	93.6
10	0.023	36.4	94.5
4	0.022	34.9	94.7
2	0.019	32.6	95.4
1	0.018	32.0	95.6
0.5	0.018	31.6	95.8
0.2	0.015	28.7	96.9
30 day			
50	0.041	49.5	92.0
20	0.032	42.7	92.9
10	0.026	38.2	94.1
4	0.024	36.6	94.2
2	0.021	34.2	94.9
1	0.019	32.6	95.4
0.5	0.017	31.2	95.9
0.2	0.013	27.1	97.1

flood-duration flow at the ungaged site is determined by comparing the drainage area to that at the streamgaging station:

$$Q_u = \left(\frac{\log(A_u)}{\log(A_g)} \right)^b \times Q_s \quad (13)$$

where Q_u is the area-weighted flood-duration flow estimate at the ungaged site, Q_s is the station estimate before weighting with the regional regression estimate (appendix 2), and A_u and A_g are the areas of the ungaged and gaged drainage areas, respectively. The exponent b is the exponent of the drainage area variable in the regional regression equation (DRNAREA; table 9).

After calculating the expected flood-duration flow at the ungaged site based on the drainage area ratio, it can be combined with the regional regression equation:

$$Q_{u(w)} = \left[\left(\frac{2\Delta A}{A_g} \right) Q_{u(r)} + \left(1 - \frac{2\Delta A}{A_g} \right) Q_u \right] \quad (14)$$

where $Q_{u(w)}$ is the weighted estimate of flood-duration flow at the ungaged site, ΔA is the absolute value of the difference between the drainage areas of the streamgaging station and the ungaged site, $|A_g - A_u|$, and $Q_{u(r)}$ is the flood-duration flow estimate for the ungaged site derived from the applicable regional equation (table 9).

Unlike the procedure for calculating the weighted average at streamgaging stations, the procedure for stations near a streamgaging station does not take into account the length of the streamgaging record. If the nearby streamgaging record is short (less than about 20 years) and the difference in drainage areas is large, the estimated flood-duration flow at streamgaging station may be excessively weighted in equation 14. In this case, the method for ungaged sites may produce better estimates of flood-duration flow.

If an ungaged site lies between two streamgaging stations on the same stream, the weighted average of the predicted flood volume at the two stations may be calculated, incorporating the relative distance of the ungaged site between the two stations. Major tributaries and (or) nonlinear variation in drainage area should be accounted for, and consideration given to the length of record, and therefore uncertainty, at each station. In areas of distributary flow, this method may not be appropriate.

Summary and Conclusions

This report presents n -day flood-duration flow frequency estimates at 173 streamgaging stations in Arizona and western New Mexico. These estimates are valuable for the design of runoff detention and retention structures and also

for water-resources planning. For short duration flood flows (1-day and 3-day flood-duration flows), flood-duration flows and flood peaks are generally correlated (that is, a streamgaging station having a relatively large peak flow will also have high 1-day and 3-day flood-duration flows). However, longer duration flood flows, such as the 15-day and 30-day flood-duration flows, may have little or no correlation with peak flows. Ephemeral streams in particular, and especially those at which peak flows are generally caused by short-lived convective thunderstorms, may have large peak flows that pass quickly and have relatively low flood volume.

The expected moments algorithm with a multiple Grubbs-Beck low-outlier test was used to estimate annual maximum n -day flood-duration flow at the 50-, 20-, 10-, 4-, 2-, 1-, 0.5-, and 0.2-percent annual exceedance probabilities for 1-, 3-, 7-, 15-, and 30-day durations at streamgaging stations throughout Arizona. A Bayesian generalized least squares regression analysis of regional skewness coefficients indicates that no basin characteristics were significant explanatory variables of skew, and a constant statewide model for each high-flow duration was used. Skew becomes increasingly negative with increasing duration of n -day flood-duration flow. Variance of the skewness coefficients also increases with increasing duration.

Flood-duration flow frequency estimates are presented at stations with 10 or more years of continuous streamflow data, unaffected by impoundments and urbanization, and adequately represented by the LP3 distribution. Not surprisingly, most stations are established near major population centers (often as flood-warning gages) and on major stream channels. As a result, gages with continuous streamgaging records, required for the flood-duration flow analysis, are most common in the central part of the State, especially on the streams that flow south and east from the Colorado Plateau, including the Verde, Salt, and Gila Rivers and their tributaries. This central highland region is the only part of the State where sufficient information exists to develop regression equations for ungaged sites. Outside this region, high-flow frequency estimates may still be calculated at streamgaging stations and for ungaged sites between two streamgaging stations.

The regression analysis in the central highland region indicates that drainage area, mean annual precipitation, and mean basin elevation are all determining factors when predicting the frequency of flood-duration flows. Regression verification statistics are generally good, with an average standard error of prediction that varies from 28 to 53 percent. Pseudo- R^2 varies from 90 to 97 percent. The flood-duration frequency regression equations developed in this study are available through the USGS StreamStats program, a Web-based application that provides streamflow statistics and basin characteristics for USGS streamgaging stations and ungaged sites of interest.

References Cited

- Balocki, J.B., and Burges, S.J., 1994, Relationships between n -day flood volumes for infrequent large floods: *Journal of Water Resources Planning and Management*, v. 120, no. 6, p. 794–818.
- Bohman, L.R., 1990, Determination of flood hydrographs for streams in South Carolina—Volume 1, simulation of flood hydrographs for rural watersheds in South Carolina: U.S. Geological Survey Water-Resources Investigations Report 89–4087, 53 p.
- Cohn, T.A., Lane, W.L., and Baier, W.G., 1997, An algorithm for computing moments-based flood quantile estimates when historical flood information is available: *Water Resources Research*, v. 33, no. 9, p. 2089–2096.
- Cohn, T.A., Lane, W.L., and Stedinger, J.R., 2001, Confidence intervals for Expected Moments Algorithm flood quantile estimates: *Water Resources Research*, v. 37, no. 6, p. 1695–1706.
- Cohn, T.A., Berenbrock, C., Kiang, J.E., and Mason Jr., R.R., 2012, Calculating weighted estimates of peak streamflow statistics: U.S. Geological Survey Fact Sheet 2012–3038, 4 p., available at <http://pubs.usgs.gov/fs/2012/3038/>.
- Cunderlik, J.M., and Ouarda, T.B.M.J., 2006, Regional flood-duration–frequency modeling in the changing environment: *Journal of Hydrology*, v. 318, p. 276–291.
- Dalrymple, T., 1960, Flood-frequency analyses, *Manual of Hydrology*, part 3: U.S. Geological Survey Water Supply Paper 1543–A, 80 p.
- Devulapalli, R.S., and Valdes, J.N., 1996, Volume-duration-frequencies for ungagged catchments in Texas—Volume I, Calculation of regional regression equations: Technical Report No. 173, Texas Water Resources Institute, College Station, Texas, 332 p.
- Eng, K., Chen, Yin-Yu, and Kiang, J.E., 2009, User's guide to the weighted-multiple-linear-regression program (WREG version 1.0): U.S. Geological Survey Techniques and Methods, book 4, chapter A8, 21 p., available at <http://pubs.usgs.gov/tm/tm4a8/>.
- England J.F., Jr., Salas, J.D., and Jarret, R.D., 2003, Comparisons of two moments-based estimators that utilize historical and paleoflood data for the log Pearson type III distribution: *Water Resources Research*, v. 39, no. 9, 1243.
- Eychaner, J.H., 1976, Estimating runoff volumes and flood hydrographs in the Colorado River Basin, southern Utah: U.S. Geological Survey Water-Resources Investigations 76–102, 18 p.
- Fenneman, N.M., and Johnson, D.W., 1946, Physical divisions of the United States: U.S. Geological Survey, scale 1:7,000,000, 1 sheet.
- Flynn, K.M., Kirby, W.H., and Hummel, P.R., 2006, User's manual for program PeakFQ, annual flood-frequency analysis using Bulletin 17B guidelines: U.S. Geological Survey, Techniques and Methods, book 4, chapter B4, 52 p.
- Griffis, V.W., Stedinger, J.R., and Cohn, T.A., 2004, Log Pearson type 3 quantile estimators with regional skew information and low outlier adjustments: *Water Resources Research*, v. 40, no. 10, W07503.
- Griffis, V.W., and Stedinger, J.R., 2007, The use of GLS regression in regional hydrologic analyses: *Journal of Hydrology*, v. 344, p. 82–95.
- Gotvald, A.J., Barth, N.A., Veilleux, A.G., and Parrett, Charles, 2012, Methods for determining magnitude and frequency of floods in California, based on data through water year 2006: U.S. Geological Survey Scientific Investigations Report 2012–5113, 38 p., available at <http://pubs.usgs.gov/sir/2012/5113/>.
- Grubbs, F.E., and Beck, G., 1972, Extension of sample sizes and percentage points for significance tests of outlying observations: *Technometrics*, v. 14, no. 4, p. 847–854.
- Helsel, D.R., and Hirsch, R.M., 2002, Statistical methods in water resources techniques of water resources investigations, book 4, chapter A3: U.S. Geological Survey, 522 p., available at <http://pubs.usgs.gov/twri/twri4a3/>.
- Interagency Advisory Committee on Water Data [IACWD], 1982, Guidelines for determining flood-flow frequency, Bulletin 17B of the Hydrology subcommittee, Office of Water Data Coordination: U.S. Geological Survey, 183 p.
- Javelle, P., Ouarda, T.B.M.J., and Bobée, B., 2003, Spring flood analysis using the flood-duration–frequency approach—Application to the provinces of Quebec and Ontario, Canada: *Hydrological Processes*, v. 17, p. 3717–3736.
- Johnston, J., 1972, *Econometric methods*: McGraw-Hill, New York, 437 p.
- Kennedy, J.R., and Gungle, B., 2010, Quantity and sources of base flow in the San Pedro River near Tombstone, Arizona: U.S. Geological Survey Scientific Investigations Report 2010–5200, 43 p., available at <http://pubs.usgs.gov/sir/2010/5200/>.
- Kroll, C.N., and Song, P., 2013, Impact of multicollinearity on small sample hydrologic regression models: *Water Resources Research*, v. 49, doi:10.1002/wrcr.20315.

- Lamontagne, J.R., Stedinger, J.R., Berenbrock, Charles, Veilleux, A.G., Ferris, J.C., and Knifong, D.L., 2012, Development of regional skews for selected flood durations for the Central Valley region, California, based on data through water year 2008: U.S. Geological Survey Scientific Investigations Report 2012–5130, 60 p., available at <http://pubs.usgs.gov/sir/2012/5130/>.
- Martins, E.S., and Stedinger, J.R., 2002, Cross-correlation among estimators of shape: *Water Resources Research*, v. 38, no. 11, p. 1252–1259.
- Mediero, L., Jiménez-Álvarez, A., and Garrote, L., 2010, Design flood hydrographs from the relationship between flood peak and volume: *Hydrology and Earth System Sciences*, v. 14, p. 2495–2505, doi:10.5194/hess-14-2495-2010.
- Natural Resources Conservation Service, 2013, U.S. general soil map (STATSGO2) for Arizona: Natural Resources Conservation Service, United States Department of Agriculture, accessed April 22, 2013, available at <http://soildatamart.nrcs.usda.gov>.
- Parrett, C., Veilleux, A., Stedinger, J.R., Barth, N.A., Knifong, D.L., and Ferris, J.C., 2011, Regional skew for California, and flood frequency for selected sites in the Sacramento–San Joaquin River Basin, based on data through water year 2006: U.S. Geological Survey Scientific Investigations Report 2010–5260, 94 p., available at <http://pubs.usgs.gov/sir/2010/5260/>.
- Paretti, N.V., Kennedy, J.R., and Cohn, T.A., 2014a, Evaluation of the expected moments algorithm and a multiple low-outlier test for flood frequency analysis at streamgaging stations in Arizona: U.S. Geological Survey Scientific Investigations Report 2014–5026, 61 p., available at <http://pubs.usgs.gov/sir/2014/5026/>.
- Paretti, N.V., Kennedy, J.R., Turney, L.A., and Veilleux, A.G., 2014b, Methods for estimating magnitude and frequency of floods in Arizona, developed with unregulated and rural peak-flow data through water year 2010: U.S. Geological Survey Scientific Investigations Report 2014–5211, 55 p., <http://pubs.usgs.gov/sir/2014/5211/>.
- Perry, C.A., 1984, A method of estimating flood volumes in western Kansas: U.S. Geological Survey Water-Resources Investigations Report 84–4164, 22 p.
- PRISM Climate Group, 2012, PRISM Climate Group, Northwest Alliance for Computational Science and Engineering: Oregon State University, Web site, accessed December 1, 2012, available at <http://prism.oregonstate.edu>.
- Reis, D.S., Stedinger, J.R., and Martins, E.S., 2005, Bayesian generalized least squares regression with application to log Pearson type 3 regional skew estimation: *Water Resources Research*, v. 41, W10419, 14 p.
- Ries, K.G., III, and Crouse, M.Y., 2002, The national flood frequency program, Version 3—A computer program for estimating magnitude and frequency of floods for ungaged sites: U.S. Geological Survey Water-Resources Investigations Report 02–4168, 42 p.
- Ries, K.G., III, Guthrie, J.D., Rea, A.H., Steeves, P.A., and Stewart, D.W., 2008, StreamStats—A water resources web application: U.S. Geological Survey Fact Sheet 2008–3067, 6 p., available at <http://pubs.usgs.gov/fs/2008/3067/>.
- Sheppard, P.R., Comrie, A.C., Packin, G.D., Angersbach, K., and Hughes, M.K., 2002, The climate of the US Southwest: *Climate Research*, v. 21, p. 219–238.
- Sherwood, J.M., 1994, Estimation of peak-frequency relations, flood hydrographs, and volume-duration-frequency relations of ungaged small urban streams in Ohio: U.S. Geological Survey Water-Supply Paper 2432, 41 p.
- Singh, V.P., and Hossein, A., 1986, An empirical relation between flood volume and peak of direct runoff: *Water Resources Bulletin*, v. 22, no. 5, p. 725–730.
- Stedinger, J.R., and Griffis, V.W., 2008, Flood frequency analysis in the United States—Time to update: *Journal of Hydrologic Engineering*, v. 13, p. 199–204.
- Thomas, B.E., and Pool, D.R., 2006, Trends in streamflow of the San Pedro river, southeastern Arizona, and regional trends in precipitation and streamflow in southeastern Arizona and southwestern New Mexico: U.S. Geological Survey Professional Paper 1712, 92 p., available at <http://pubs.usgs.gov/pp/pp1712/>.
- Weaver, J.C., Feaster, T.D., and Gotvald, A.J., 2009, Magnitude and frequency of rural floods in the southeastern United States, through 2006—Volume 2, North Carolina: U.S. Geological Survey Scientific Investigations Report 2009–5158, 111 p., available at <http://pubs.usgs.gov/sir/2009/5158/>.
- Yue, S., and Rasmussen, P., 2002, Bivariate frequency analysis: discussion of some useful concepts in hydrological application: *Hydrological Processes*, v. 16, p. 2881–2898.

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133 09505300 ¹	240	92	186	723	279	513	1,260	519	840	2,220	1,040	1,420	3,180	1,540	1,980	4,370	2,190	2,690	5,810	2,980	3,550	8,140	4,480	5,100
134 09505350 ¹	1,040	422	910	3,090	1,200	2,530	5,340	2,140	4,110	9,410	4,070	6,640	13,400	5,920	8,930	18,400	8,240	11,700	24,500	11,000	15,000	34,300	15,800	20,100
135 09505800 ¹	1,290	672	1,140	3,610	1,800	3,080	6,060	3,130	4,860	10,400	5,890	7,930	14,600	8,430	10,800	19,800	11,600	14,300	26,000	15,300	18,500	36,000	20,900	24,600
136 09506000	5,000	5,220	5,040	14,300	13,800	14,200	24,500	22,900	23,900	43,300	37,900	40,600	62,500	54,500	57,800	86,600	75,300	79,400	117,000	101,000	106,000	167,000	145,000	151,000
137 09507600	54	46	51	143	145	144	235	275	258	393	594	528	546	877	784	729	1,230	1,110	948	1,650	1,490	1,300	2,380	2,170
138 09507700 ¹	38	35	37	117	106	111	206	201	203	374	439	416	546	644	616	764	900	866	1,030	1,200	1,160	1,490	1,700	1,660
139 09507980 ¹	1,950	1,230	1,820	4,300	3,610	4,200	6,420	6,290	6,390	9,730	12,000	10,500	12,700	17,100	14,400	16,000	23,400	19,100	19,700	30,800	24,600	25,300	41,900	33,500
140 09508300 ¹	410	154	338	1,380	519	1,060	2,530	971	1,790	4,730	1,930	3,050	7,000	2,900	4,250	9,890	4,150	5,740	13,500	5,690	7,570	19,500	8,820	10,900
141 09508500 ¹	7,410	6,620	7,310	21,400	17,300	20,600	36,600	28,500	34,500	64,200	47,200	57,600	91,700	67,300	80,200	126,000	92,300	108,000	168,000	123,000	141,000	236,000	173,000	194,000
142 09510070	17	44	27	66	149	110	130	288	231	267	634	542	421	944	838	630	1,330	1,210	907	1,800	1,660	1,400	2,630	2,490
143 09510080 ¹	23	84	51	138	281	229	343	532	484	884	1,150	1,110	1,610	1,690	1,680	2,730	2,370	2,400	4,390	3,170	3,260	7,750	4,540	4,710
144 09510100 ¹	14	40	21	67	140	98	150	272	215	342	597	509	575	896	808	910	1,270	1,190	1,370	1,730	1,660	2,240	2,600	2,550
145 09510150 ¹	203	324	247	764	1,080	923	1,510	1,980	1,800	3,120	4,030	3,780	4,950	5,900	5,700	7,470	8,230	8,100	10,900	11,000	11,000	17,100	15,800	15,900
146 09510200 ¹	578	746	607	2,230	2,540	2,310	4,360	4,600	4,440	8,720	8,900	8,810	13,500	13,100	13,300	19,700	18,400	18,900	27,800	24,800	25,800	41,800	36,700	38,000
147 09512100	79	--	--	436	--	--	1,046	--	--	2,614	--	--	4,683	--	--	7,865	--	--	12,579	--	--	22,085	--	--
148 09512162	201	--	--	572	--	--	976	--	--	1,709	--	--	2,443	--	--	3,357	--	--	4,477	--	--	6,326	--	--
149 09512165	512	--	--	5,595	--	--	19,062	--	--	69,201	--	--	157,618	--	--	328,350	--	--	639,293	--	--	1,423,582	--	--
150 09512200	1	--	--	4	--	--	9	--	--	21	--	--	37	--	--	59	--	--	92	--	--	155	--	--
151 09512280 ¹	211	228	216	771	832	794	1,500	1,560	1,530	3,050	2,980	3,000	4,790	4,580	4,640	7,180	6,680	6,800	10,400	9,360	9,560	16,200	15,700	15,800
152 09512400	147	401	169	352	1,580	476	548	2,950	885	872	5,250	1,820	1,170	8,280	2,990	1,520	12,400	4,680	1,930	17,900	6,940	2,560	33,300	13,200
153 09512450	201	295	232	527	921	695	860	1,670	1,280	1,440	3,030	2,440	1,990	4,590	3,760	2,660	6,650	5,510	3,450	9,270	7,730	4,730	15,200	12,700
154 09512500 ¹	782	1,000	801	2,180	3,060	2,290	3,750	5,390	4,070	6,750	9,520	7,630	9,900	14,200	11,600	14,000	20,100	16,900	19,300	27,600	23,700	28,500	43,000	37,300
155 09512600 ¹	314	243	296	677	774	708	1,000	1,420	1,170	1,510	2,690	2,110	1,950	4,040	3,120	2,460	5,790	4,440	3,040	7,960	6,070	3,910	12,500	9,500
156 09512800	1,660	1,980	1,730	6,480	6,050	6,330	13,200	10,500	11,900	28,400	18,300	21,800	46,700	27,000	32,100	73,000	38,000	45,100	110,000	51,600	61,400	181,000	78,400	91,100
157 09512860 ¹	137	237	177	471	848	676	892	1,590	1,350	1,760	3,100	2,790	2,720	4,700	4,330	4,010	6,770	6,340	5,730	9,360	8,870	8,800	15,000	14,300
158 09513780 ¹	371	227	329	1,490	815	1,220	3,010	1,530	2,240	6,250	2,930	4,050	9,910	4,480	5,950	14,900	6,500	8,380	21,500	9,060	11,500	33,400	14,900	17,800
159 09513800	357	254	307	1,770	935	1,240	3,940	1,750	2,320	9,000	3,320	4,220	15,100	5,100	6,310	23,900	7,460	9,030	36,100	10,500	12,500	58,800	17,600	20,300
160 09513835 ¹	564	354	468	2,040	1,400	1,710	3,900	2,640	3,090	7,700	4,740	5,420	11,800	7,480	8,300	17,400	11,200	12,200	24,500	16,200	17,400	37,100	30,000	31,100
161 09513860 ¹	130	118	128	381	512	405	658	1,000	744	1,170	1,820	1,400	1,680	2,970	2,200	2,320	4,600	3,320	3,100	6,830	4,830	4,400	13,900	9,810
162 09513910	879	--	--	1,795	--	--	2,597	--	--	3,837	--	--	4,930	--	--	6,168	--	--	7,566	--	--	9,678	--	--
163 09513970	393	--	--	3,849	--	--	12,312	--	--	41,617	--	--	90,311	--	--	179,844	--	--	335,583	--	--	708,624	--	--
164 09515500 ¹	680	873	716	1,990	2,780	2,190	3,460	4,950	3,980	6,180	8,920	7,530	8,950	13,300	11,400	12,500	18,900	16,400	16,800	26,000	22,800	24,200	40,600	35,700
165 09516500	432	1,270	670	2,230	4,250	3,100	5,050	7,550	6,440	11,800	13,100	12,700	20,000	19,800	19,900	31,900	28,600	29,200	48,600	39,900	41,200	79,800	66,000	67,600
166 09517000	637	--	--	1,656	--	--	2,718	--	--	4,601	--	--	6,456	--	--	8,747	--	--	11,541	--	--	16,134	--	--
167 09517490	133	--	--	739	--	--	1,749	--	--	4,259	--	--	7,455	--	--	12,213	--	--	19,027	--	--	32,221	--	--
168 09517500	359	--	--	1,595	--	--	3,385	--	--	7,402	--	--	12,137	--	--	18,797	--	--	27,886	--	--	44,633	--	--
169 09520170	400	--	--	756	--	--	1,044	--	--	1,459	--	--	1,804	--	--	2,177	--	--	2,579	--	--	3,157	--	--
170 09535100	303	--	--	768	--	--	1,247	--	--	2,085	--	--	2,904	--	--	3,909	--	--	5,129	--	--	7,120	--	--
171 09535300	441	--	--	1,044	--	--	1,648	--	--	2,696	--	--	3,714	--	--	4,964	--	--	6,483	--	--	8,978	--	--
172 09537200	65	--	--	214	--	--	391	--	--	735	--	--	1,096	--	--	1,562	--	--	2,154	--	--	3,162	--	--
173 09537500	581	--	--	961	--	--	1,241	--	--	1,619	--	--	1,915	--	--	2,223	--	--	2,542	--	--	2,985	--	--

¹Station used to develop regional regression equations.

133 09505300 ¹	171	62	144	382	165	311	571	285	450	865	492	678	1,120	704	887	1,410	960	1,140	1,740	1,260	1,430	2,230	1,950	2,020
134 09505350 ¹	738	268	666	1,790	737	1,550	2,780	1,250	2,330	4,380	2,110	3,530	5,810	2,970	4,540	7,460	4,010	5,710	9,330	5,220	7,020	12,100	7,630	9,010
135 09505800 ¹	802	460	736	2,100	1,160	1,860	3,390	1,890	2,860	5,570	3,080	4,440	7,590	4,240	5,820	9,960	5,600	7,430	12,700	7,170	9,220	17,000	10,100	11,800
136 09506000	3,420	3,230	3,390	9,090	8,610	8,970	15,000	14,600	14,900	25,500	24,100	24,900	35,800	34,800	35,300	48,400	47,900	48,100	63,700	64,200	64,000	88,700	90,700	90,200
137 09507600	36	28	33	95	85	90	155	147	150	257	258	257	351	361	358	462	480	476	592	616	611	792	936	915
138 09507700 ¹	25	24	25	69	65	67	113	109	111	189	184	186	261	254	256	346	333	337	446	421	427	602	634	628
139 09507980 ¹	922	617	859	2,410	2,170	2,360	3,880	3,730	3,840	6,330	6,480	6,390	8,590	9,100	8,810	11,200	12,200	11,700	14,300	15,700	15,000	18,900	22,000	20,900
140 09508300 ¹	241	76	196	762	280	593	1,340	519	984	2,350	967	1,640	3,340	1,430	2,200	4,520	2,010	2,880	5,920	2,710	3,670	8,100	4,240	5,020
141 09508500 ¹	6,020	4,030	5,740	14,400	10,900	13,900	22,300	18,300	21,400	34,900	30,100	33,600	46,400	43,000	45,200	59,500	58,700	59,200	74,400	77,900	75,900	97,100	108,000	103,000
142 09510070	25	21	24	43	82	50	58	149	82	77	276	145	93	394	213	109	533	290	127	692	381	152	1,070	644
143 09510080 ¹	15	41	27	82	157	128	190	282	257	451	518	505	773	736	741	1,240	991	1,020	1,890	1,280	1,330	3,110	1,930	1,990
144 09510100 ¹	9	19	12	40	75	54	82	139	112	172	262	228	273	380	347	409	520	491	585	683	661	894	1,070	1,040
145 09510150 ¹	100	138	115	377	592	481	736	1,080	943	1,480	2,020	1,850	2,290	2,920	2,770	3,390	3,990	3,860	4,810	5,240	5,170	7,320	7,740	7,690
146 09510200 ¹	338	285	327	1,230	1,340	1,260	2,340	2,540	2,410	4,530	4,860	4,670	6,830	7,210	7,020	9,810	10,100	9,970	13,600	13,600	13,600	19,900	20,200	20,100
147 09512100	34	--	--	193	--	--	464	--	--	1,142	--	--	2,011	--	--	3,307	--	--	5,169	--	--	8,778	--	--
148 09512162	93	--	--	260	--	--	442	--	--	773	--	--	1,104	--	--	1,518	--	--	2,027	--	--	2,870	--	--
149 09512165	281	--	--	3,725	--	--	13,840	--	--	54,495	--	--	130,020	--	--	281,271	--	--	564,994	--	--	1,301,150	--	--
150 09512200	0.0	--	--	1	--	--	3	--	--	8	--	--	13	--	--	21	--	--	32	--	--	53	--	--
151 09512280 ¹	103	95	101	373	410	387	736	815	774	1,530	1,610	1,580	2,450	2,520	2,500	3,760	3,700	3,710	5,560	5,220	5,290	8,970	8,510	8,570
152 09512400	85	149	95	247	709	330	420	1,510	679	725	3,170	1,440	1,020	5,270	2,480	1,380	8,200	3,980	1,810	12,200	6,160	2,500	21,000	12,500
153 09512450	103	182	129	285	510	386	474	932	719	801	1,670	1,330	1,110	2,530	2,060	1,490	3,630	3,000	1,930	5,040	4,220	2,630	8,060	7,030
154 09512500 ¹	382	544	395	1,080	1,720	1,170	1,890	3,120	2,120	3,440	5,570	4,040	5,100	8,330	6,300	7,280	11,800	9,310	10,100	16,300	13,300	15,100	24,800	21,500
155 09512600 ¹	176	135	163	427	428	428	669	781	725	1,070	1,410	1,270	1,440	2,100	1,860	1,870	2,970	2,600	2,370	4,040	3,540	3,150	6,310	5,640
156 09512800	872	973	896	3,550	3,380	3,490	7,400	6,170	6,780	16,200	11,100	12,900	26,900	16,500	19,300	42,400	23,400	27,400	64,400	32,100	37,400	107,000	47,600	53,100
157 09512860 ¹	66	99	80	232	429	341	447	836	704	892	1,630	1,440	1,390	2,490	2,270	2,070	3,580	3,340	2,980	4,950	4,680	4,630	7,850	7,570
158 09513780 ¹	160	97	139	794	409	617	1,730	802	1,210	3,780	1,570	2,320	6,110	2,420	3,390	9,280	3,520	4,740	13,400	4,920	6,370	20,700	7,930	9,300
159 09513800	159	104	130	880	457	600	2,040	914	1,190	4,770	1,820	2,330	8,100	2,850	3,520	12,800	4,210	5,080	19,300	5,950	7,050	31,200	9,720	10,800
160 09513835 ¹	270	129	194	1,210	628	847	2,550	1,350	1,680	5,490	2,830	3,380	8,860	4,700	5,400	13,500	7,310	8,220	19,700	10,900	12,000	30,700	18,800	19,900
161 09513860 ¹	50	42	48	167	212	177	304	480	353	564	1,060	730	831	1,840	1,210	1,170	2,970	1,910	1,580	4,570	2,930	2,270	8,480	6,220
162 09513910	536	--	--	1,171	--	--	1,750	--	--	2,671	--	--	3,499	--	--	4,453	--	--	5,542	--	--	7,210	--	--
163 09513970	153	--	--	1,934	--	--	6,952	--	--	26,251	--	--	60,735	--	--	127,496	--	--	248,659	--	--	551,155	--	--
164 09515500 ¹	290	429	319	1,020	1,520	1,170	1,910	2,810	2,270	3,660	5,150	4,410	5,510	7,760	6,820	7,910	11,100	9,920	10,900	15,200	13,900	16,100	23,300	21,800
165 09516500	273	560	344	1,080	2,180	1,470	2,150	4,230	3,080	4,360	8,080	6,370	6,800	12,600	10,400	10,000	18,600	15,900	14,300	26,400	23,200	21,600	41,600	38,200
166 09517000	363	--	--	796	--	--	1,205	--	--	1,879	--	--	2,507	--	--	3,253	--	--	4,131	--	--	5,525	--	--
167 09517490	131	--	--	329	--	--	530	--	--	876	--	--	1,211	--	--	1,617	--	--	2,104	--	--	2,892	--	--
168 09517500	172	--	--	699	--	--	1,404	--	--	2,871	--	--	4,492	--	--	6,654	--	--	9,458	--	--	14,336	--	--
169 09520170	216	--	--	365	--	--	477	--	--	630	--	--	752	--	--	880	--	--	1,015	--	--	1,202	--	--
170 09535100	129	--	--	335	--	--	551	--	--	934	--	--	1,312	--	--	1,779	--	--	2,349	--	--	3,289	--	--
171 09535300	264	--	--	639	--	--	1,023	--	--	1,696	--	--	2,359	--	--	3,180	--	--	4,186	--	--	5,852	--	--
172 09537200	27	--	--	87	--	--	156	--	--	284	--	--	414	--	--	577	--	--	778	--	--	1,108	--	--
173 09537500	307	--	--	529	--	--	696	--	--	924	--	--	1,105	--	--	1,294	--	--	1,492	--	--	1,768	--	--

¹Station used to develop regional regression equations.

133 09505300 ¹	106	38	91	222	103	187	323	162	253	476	273	368	609	388	473	757	505	592	921	659	741	1,160	904	978
134 09505350 ¹	468	174	433	1,010	459	915	1,490	731	1,280	2,220	1,200	1,850	2,850	1,650	2,300	3,550	2,160	2,830	4,330	2,780	3,450	5,460	3,760	4,420
135 09505800 ¹	546	311	512	1,230	752	1,140	1,860	1,140	1,630	2,860	1,800	2,400	3,740	2,350	3,000	4,750	3,050	3,760	5,890	3,850	4,600	7,610	5,070	5,890
136 09506000	2,270	2,070	2,240	5,470	5,100	5,390	8,600	8,350	8,520	13,900	13,300	13,600	18,800	18,700	18,700	24,700	25,000	24,900	31,700	33,000	32,500	42,700	45,900	45,000
137 09507600	34	18	33	48	54	49	58	87	63	69	151	89	78	206	117	86	267	146	95	340	181	106	452	231
138 09507700 ¹	17	16	17	42	43	43	67	65	66	108	109	108	145	145	145	188	184	186	238	232	233	313	302	304
139 09507980 ¹	461	426	456	1,360	1,290	1,350	2,320	2,260	2,300	3,990	3,870	3,940	5,570	5,220	5,380	7,470	6,920	7,140	9,690	8,870	9,150	13,200	11,900	12,200
140 09508300 ¹	141	46	116	437	158	340	757	288	536	1,320	534	870	1,860	806	1,160	2,500	1,080	1,480	3,260	1,460	1,890	4,430	2,070	2,550
141 09508500 ¹	3,270	2,650	3,210	8,220	6,510	7,960	13,100	10,700	12,500	21,400	16,900	19,900	29,200	23,300	26,500	38,500	31,100	34,600	49,400	40,600	44,300	66,600	55,900	59,800
142 09510070	16	14	15	25	49	28	32	87	45	41	161	81	48	230	123	56	302	164	63	392	216	74	532	291
143 09510080 ¹	10	27	17	50	94	77	113	167	154	262	307	299	443	431	432	705	566	578	1,070	732	755	1,750	987	1,030
144 09510100 ¹	5	12	7	26	44	34	59	80	72	138	151	148	234	220	223	373	291	302	565	383	402	921	527	560
145 09510150 ¹	56	91	69	210	336	271	407	637	552	811	1,190	1,090	1,250	1,700	1,610	1,830	2,280	2,210	2,590	2,990	2,940	3,900	4,120	4,090
146 09510200 ¹	171	182	174	747	718	738	1,530	1,450	1,490	3,140	2,790	2,950	4,890	4,170	4,420	7,170	5,680	6,110	10,100	7,660	8,230	15,000	10,900	11,700
147 09512100	15	--	--	90	--	--	217	--	--	538	--	--	952	--	--	1,573	--	--	2,469	--	--	4,216	--	--
148 09512162	43	--	--	124	--	--	215	--	--	387	--	--	565	--	--	793	--	--	1,082	--	--	1,575	--	--
149 09512165	159	--	--	2,511	--	--	10,266	--	--	44,927	--	--	114,988	--	--	265,308	--	--	565,891	--	--	1,403,385	--	--
150 09512200	0	--	--	1	--	--	1	--	--	3	--	--	6	--	--	9	--	--	14	--	--	24	--	--
151 09512280 ¹	54	53	54	188	211	196	362	426	395	729	845	803	1,150	1,400	1,330	1,730	1,920	1,880	2,520	2,720	2,680	3,970	4,110	4,090
152 09512400	37	74	45	149	329	203	296	730	482	599	1,540	1,100	933	2,830	2,090	1,380	4,010	3,130	1,950	6,050	4,830	2,940	9,850	8,040
153 09512450	52	101	66	137	291	201	223	492	372	370	859	686	509	1,340	1,100	675	1,800	1,520	870	2,490	2,120	1,180	3,650	3,140
154 09512500 ¹	207	323	215	589	963	639	1,020	1,700	1,170	1,840	2,980	2,230	2,700	4,510	3,510	3,810	6,090	5,020	5,220	8,320	7,060	7,660	12,100	10,500
155 09512600 ¹	97	79	91	260	246	253	431	428	429	733	761	753	1,030	1,150	1,120	1,400	1,540	1,510	1,840	2,090	2,050	2,570	2,990	2,930
156 09512800	508	596	527	2,040	1,860	1,970	4,180	3,420	3,760	8,960	6,050	6,940	14,600	9,070	10,200	22,700	12,300	14,000	33,800	16,800	18,900	54,800	24,200	27,200
157 09512860 ¹	34	58	44	127	227	186	252	452	397	522	886	816	832	1,400	1,320	1,260	1,920	1,840	1,850	2,660	2,580	2,940	3,910	3,840
158 09513780 ¹	84	55	75	420	214	323	920	425	615	2,040	832	1,170	3,340	1,350	1,750	5,140	1,840	2,350	7,520	2,590	3,200	11,800	3,860	4,640
159 09513800	81	58	69	495	233	310	1,210	475	605	3,000	949	1,180	5,280	1,580	1,860	8,670	2,180	2,550	13,500	3,100	3,570	22,700	4,710	5,350
160 09513835 ¹	185	65	119	702	291	454	1,370	649	835	2,750	1,380	1,630	4,270	2,540	2,800	6,290	3,590	3,930	8,920	5,410	5,810	13,500	8,810	9,310
161 09513860 ¹	22	19	21	77	94	81	144	217	169	274	487	363	410	974	719	585	1,390	1,000	805	2,190	1,560	1,180	3,730	2,660
162 09513910	307	--	--	677	--	--	1,016	--	--	1,560	--	--	2,051	--	--	2,619	--	--	3,270	--	--	4,272	--	--
163 09513970	65	--	--	989	--	--	3,942	--	--	16,682	--	--	41,619	--	--	93,610	--	--	194,663	--	--	466,910	--	--
164 09515500 ¹	145	255	166	544	830	639	1,080	1,530	1,300	2,220	2,780	2,570	3,520	4,250	4,060	5,320	5,780	5,680	7,750	7,930	7,900	12,200	11,500	11,600
165 09516500	126	314	180	624	1,120	838	1,370	2,210	1,860	3,070	4,180	3,840	5,080	6,830	6,430	7,880	9,450	9,170	11,700	13,400	13,200	18,500	20,400	20,200
166 09517000	240	--	--	432	--	--	589	--	--	819	--	--	1,015	--	--	1,230	--	--	1,468	--	--	1,818	--	--
167 09517490	26	--	--	147	--	--	343	--	--	809	--	--	1,375	--	--	2,184	--	--	3,294	--	--	5,333	--	--
168 09517500	76	--	--	314	--	--	641	--	--	1,337	--	--	2,121	--	--	3,184	--	--	4,585	--	--	7,065	--	--
169 09520170	94	--	--	169	--	--	228	--	--	311	--	--	378	--	--	449	--	--	524	--	--	629	--	--
170 09535100	61	--	--	157	--	--	255	--	--	424	--	--	588	--	--	787	--	--	1,025	--	--	1,409	--	--
171 09535300	141	--	--	330	--	--	515	--	--	831	--	--	1,133	--	--	1,499	--	--	1,937	--	--	2,646	--	--
172 09537200	12	--	--	40	--	--	72	--	--	131	--	--	192	--	--	269	--	--	364	--	--	521	--	--
173 09537500	153	--	--	301	--	--	422	--	--	595	--	--	739	--	--	892	--	--	1,056	--	--	1,290	--	--

¹Station used to develop regional regression equations.

133 09505300 ¹	64	25	56	132	63	111	189	104	155	275	170	223	348	234	282	430	307	351	519	394	432	650	528	555
134 09505350 ¹	310	118	288	629	292	572	894	470	795	1,280	752	1,120	1,610	1,020	1,370	1,960	1,330	1,660	2,340	1,680	1,980	2,890	2,260	2,470
135 09505800 ¹	353	222	332	780	497	722	1,160	762	1,040	1,750	1,160	1,510	2,280	1,530	1,890	2,860	1,930	2,320	3,520	2,380	2,780	4,510	3,120	3,460
136 09506000	1,600	1,380	1,570	3,800	3,240	3,660	5,930	5,180	5,670	9,530	8,060	8,820	12,900	11,200	11,900	17,000	14,700	15,500	21,800	19,100	19,800	29,400	28,500	28,700
137 09507600	32	12	31	39	34	38	43	56	44	47	95	52	50	130	60	53	167	70	56	211	81	59	261	102
138 09507700 ¹	11	11	11	27	28	27	41	44	42	63	71	67	82	94	90	104	119	114	128	146	142	165	176	174
139 09507980 ¹	287	311	290	818	842	824	1,380	1,400	1,390	2,350	2,380	2,360	3,290	3,270	3,280	4,410	4,250	4,310	5,730	5,410	5,510	7,820	7,100	7,250
140 09508300 ¹	87	29	71	253	93	196	429	169	311	731	310	498	1,020	453	652	1,360	625	837	1,750	838	1,050	2,370	1,170	1,360
141 09508500 ¹	2,390	1,820	2,340	5,290	4,210	5,140	7,890	6,670	7,640	12,000	10,300	11,500	15,600	14,200	15,000	19,600	18,600	19,100	24,200	23,800	24,000	31,000	34,900	33,500
142 09510070	5	9	7	15	30	23	27	53	43	49	98	84	71	138	124	99	184	170	134	238	224	191	297	288
143 09510080 ¹	7	19	12	32	58	48	70	103	94	158	187	182	261	262	262	405	348	353	602	447	457	960	558	574
144 09510100 ¹	4	8	5	15	26	19	28	48	39	56	90	78	85	129	117	124	175	163	173	229	219	259	292	288
145 09510150 ¹	32	63	45	124	207	167	244	375	328	490	702	646	759	1,010	959	1,120	1,360	1,330	1,580	1,790	1,760	2,380	2,340	2,340
146 09510200 ¹	113	123	115	421	432	424	809	815	811	1,580	1,580	1,580	2,410	2,340	2,370	3,480	3,270	3,340	4,840	4,420	4,530	7,150	6,150	6,320
147 09512100	8	--	--	49	--	--	121	--	--	309	--	--	556	--	--	932	--	--	1,483	--	--	2,571	--	--
148 09512162	21	--	--	62	--	--	110	--	--	203	--	--	301	--	--	429	--	--	595	--	--	884	--	--
149 09512165	99	--	--	1,647	--	--	7,007	--	--	32,265	--	--	85,742	--	--	205,267	--	--	454,030	--	--	1,180,509	--	--
150 09512200	0.0	--	--	0.0	--	--	1	--	--	2	--	--	3	--	--	4	--	--	7	--	--	12	--	--
151 09512280 ¹	29	31	29	100	116	106	194	229	212	396	454	433	629	709	687	957	1,040	1,020	1,410	1,470	1,460	2,250	2,250	2,250
152 09512400	19	38	24	82	166	113	170	357	258	359	759	591	572	1,270	1,040	862	2,000	1,690	1,240	3,030	2,630	1,920	5,260	4,740
153 09512450	30	59	38	72	166	107	111	290	196	174	494	357	232	726	554	298	1,010	798	373	1,380	1,120	487	2,120	1,800
154 09512500 ¹	123	199	129	340	569	370	583	995	669	1,040	1,710	1,260	1,520	2,500	1,960	2,150	3,460	2,850	2,950	4,670	4,010	4,350	7,070	6,350
155 09512600 ¹	59	49	56	158	144	151	264	254	257	456	444	447	648	645	645	889	887	888	1,190	1,190	1,190	1,690	1,720	1,720
156 09512800	302	380	321	1,220	1,120	1,180	2,500	1,980	2,210	5,350	3,460	4,010	8,730	5,060	5,790	13,500	7,010	7,940	20,200	9,460	10,600	32,600	14,200	15,400
157 09512860 ¹	19	35	27	76	129	110	160	249	228	351	488	466	582	745	727	918	1,070	1,060	1,390	1,480	1,480	2,300	2,160	2,170
158 09513780 ¹	47	33	42	237	119	178	522	232	343	1,160	454	645	1,900	699	927	2,910	1,010	1,280	4,260	1,420	1,720	6,630	2,130	2,430
159 09513800	39	33	36	302	127	166	822	253	330	2,270	506	641	4,260	795	957	7,390	1,170	1,370	12,000	1,670	1,900	21,400	2,580	2,820
160 09513835 ¹	89	33	56	370	147	222	756	317	418	1,590	679	825	2,530	1,140	1,310	3,830	1,800	2,000	5,550	2,720	2,970	8,650	4,690	4,940
161 09513860 ¹	11	9	10	39	44	41	75	101	84	146	228	182	220	403	313	316	664	510	438	1,050	806	643	1,910	1,500
162 09513910	167	--	--	409	--	--	649	--	--	1,059	--	--	1,450	--	--	1,920	--	--	2,480	--	--	3,377	--	--
163 09513970	30	--	--	486	--	--	2,005	--	--	8,798	--	--	22,470	--	--	51,620	--	--	109,448	--	--	268,787	--	--
164 09515500 ¹	86	158	102	338	487	394	691	878	788	1,480	1,570	1,540	2,420	2,330	2,350	3,760	3,260	3,340	5,620	4,440	4,580	9,160	6,670	6,860
165 09516500	74	183	107	357	624	480	782	1,190	1,020	1,760	2,240	2,100	2,940	3,470	3,370	4,610	5,070	5,010	6,920	7,190	7,160	11,200	11,600	11,600
166 09517000	171	--	--	274	--	--	351	--	--	458	--	--	544	--	--	636	--	--	734	--	--	874	--	--
167 09517490	29	--	--	71	--	--	114	--	--	188	--	--	258	--	--	344	--	--	446	--	--	611	--	--
168 09517500	35	--	--	147	--	--	300	--	--	622	--	--	984	--	--	1,472	--	--	2,111	--	--	3,236	--	--
169 09520170	51	--	--	83	--	--	106	--	--	137	--	--	161	--	--	186	--	--	213	--	--	249	--	--
170 09535100	31	--	--	81	--	--	132	--	--	221	--	--	307	--	--	412	--	--	537	--	--	740	--	--
171 09535300	79	--	--	177	--	--	268	--	--	416	--	--	551	--	--	708	--	--	890	--	--	1,173	--	--
172 09537200	5	--	--	21	--	--	42	--	--	84	--	--	130	--	--	190	--	--	266	--	--	395	--	--
173 09537500	90	--	--	181	--	--	257	--	--	367	--	--	460	--	--	561	--	--	669	--	--	825	--	--

¹Station used to develop regional regression equations.

133 09505300 ¹	40	17	34	88	47	76	130	70	105	193	113	152	247	151	189	306	197	233	371	247	279	466	324	347
134 09505350 ¹	207	85	192	433	206	396	618	322	551	886	510	773	1,110	674	938	1,340	871	1,120	1,590	1,080	1,300	1,940	1,420	1,570
135 09505800 ¹	226	163	217	528	365	497	797	543	726	1,200	823	1,060	1,550	1,060	1,320	1,930	1,330	1,590	2,350	1,610	1,870	2,940	2,070	2,260
136 09506000	1,200	945	1,160	2,760	2,160	2,640	4,240	3,380	3,980	6,640	5,160	6,010	8,850	6,940	7,820	11,400	9,120	9,980	14,400	11,500	12,400	19,000	16,900	17,300
137 09507600	31	9	30	35	24	34	37	40	37	39	67	40	41	88	43	42	114	46	43	140	50	45	169	58
138 09507700 ¹	8	8	8	19	21	19	28	32	30	44	52	48	57	66	63	72	84	80	88	101	98	113	119	118
139 09507980 ¹	197	231	201	537	497	529	882	962	902	1,470	1,620	1,520	2,010	2,170	2,090	2,660	2,830	2,760	3,410	3,520	3,490	4,570	4,510	4,530
140 09508300 ¹	72	20	60	158	56	135	234	106	194	351	192	288	451	272	366	563	373	458	686	488	562	867	660	712
141 09508500 ¹	1,560	1,270	1,540	3,480	2,770	3,390	5,250	4,420	5,090	8,120	6,740	7,700	10,700	8,990	10,000	13,800	11,700	12,700	17,300	14,700	15,700	22,800	21,200	21,600
142 09510070	3	7	5	12	18	16	25	36	33	52	66	63	83	91	90	124	120	121	177	152	154	269	184	187
143 09510080 ¹	5	14	9	23	35	30	48	70	63	102	127	121	162	173	172	241	229	230	344	288	292	521	349	356
144 09510100 ¹	3	6	4	10	16	12	18	32	24	33	59	48	49	82	71	68	111	99	91	142	131	129	175	169
145 09510150 ¹	22	46	31	81	112	97	154	246	208	296	457	407	445	640	596	635	861	823	872	1,110	1,080	1,270	1,410	1,400
146 09510200 ¹	66	87	71	282	215	258	559	507	539	1,090	970	1,030	1,640	1,400	1,500	2,320	1,950	2,060	3,130	2,570	2,700	4,410	3,460	3,590
147 09512100	4	--	--	26	--	--	63	--	--	156	--	--	272	--	--	443	--	--	682	--	--	1,131	--	--
148 09512162	11	--	--	33	--	--	58	--	--	106	--	--	156	--	--	220	--	--	301	--	--	442	--	--
149 09512165	69	--	--	1,151	--	--	4,902	--	--	22,569	--	--	59,938	--	--	143,358	--	--	316,725	--	--	822,034	--	--
150 09512200	0.0	--	--	0.0	--	--	0.0	--	--	1	--	--	2	--	--	2	--	--	4	--	--	6	--	--
151 09512280 ¹	17	20	18	61	62	61	118	131	125	243	254	250	387	383	384	589	554	561	866	765	778	1,390	1,130	1,150
152 09512400	10	23	14	48	82	61	103	184	141	219	374	310	348	605	524	520	930	830	741	1,360	1,250	1,120	2,280	2,150
153 09512450	19	38	23	40	116	59	58	177	99	84	292	177	107	414	270	132	571	397	159	755	558	199	1,130	913
154 09512500 ¹	78	132	82	210	357	227	353	613	401	619	1,030	741	891	1,460	1,130	1,240	2,000	1,630	1,680	2,630	2,270	2,420	3,880	3,530
155 09512600 ¹	39	33	37	102	94	98	170	159	163	292	272	278	415	383	389	568	522	529	758	683	691	1,070	962	969
156 09512800	188	256	203	737	649	704	1,490	1,220	1,350	3,150	2,090	2,420	5,080	2,960	3,410	7,780	4,070	4,610	11,500	5,360	5,960	18,400	7,860	8,430
157 09512860 ¹	11	24	17	45	69	60	93	149	134	201	285	271	331	422	411	516	598	591	775	809	807	1,270	1,140	1,140
158 09513780 ¹	35	21	29	142	65	108	282	136	198	563	259	356	862	386	503	1,250	553	684	1,730	754	894	2,540	1,090	1,220
159 09513800	23	21	22	174	68	93	459	144	194	1,210	281	363	2,170	426	520	3,600	619	722	5,610	858	967	9,360	1,280	1,380
160 09513835 ¹	53	20	34	207	72	121	405	163	228	808	336	428	1,240	543	648	1,820	836	953	2,550	1,230	1,350	3,810	2,040	2,150
161 09513860 ¹	6	5	6	21	22	21	40	49	43	76	105	88	112	177	144	158	283	228	215	431	351	308	757	632
162 09513910	100	--	--	266	--	--	437	--	--	732	--	--	1,016	--	--	1,358	--	--	1,764	--	--	2,413	--	--
163 09513970	16	--	--	262	--	--	1,062	--	--	4,501	--	--	11,131	--	--	24,683	--	--	50,404	--	--	117,494	--	--
164 09515500 ¹	55	105	65	211	286	237	430	535	481	920	936	930	1,510	1,340	1,380	2,360	1,860	1,940	3,550	2,470	2,590	5,830	3,620	3,750
165 09516500	48	116	69	231	341	281	502	678	602	1,110	1,240	1,200	1,810	1,860	1,850	2,790	2,670	2,690	4,110	3,680	3,730	6,480	5,770	5,810
166 09517000	130	--	--	188	--	--	229	--	--	283	--	--	325	--	--	369	--	--	414	--	--	477	--	--
167 09517490	9	--	--	37	--	--	72	--	--	142	--	--	215	--	--	307	--	--	422	--	--	611	--	--
168 09517500	19	--	--	77	--	--	153	--	--	307	--	--	470	--	--	680	--	--	944	--	--	1,384	--	--
169 09520170	30	--	--	53	--	--	70	--	--	95	--	--	115	--	--	136	--	--	158	--	--	189	--	--
170 09535100	18	--	--	44	--	--	70	--	--	115	--	--	159	--	--	212	--	--	277	--	--	383	--	--
171 09535300	51	--	--	108	--	--	158	--	--	236	--	--	304	--	--	381	--	--	467	--	--	596	--	--
172 09537200	3	--	--	13	--	--	26	--	--	51	--	--	78	--	--	112	--	--	154	--	--	222	--	--
173 09537500	56	--	--	119	--	--	174	--	--	255	--	--	324	--	--	401	--	--	484	--	--	605	--	--

¹Station used to develop regional regression equations.

133	09505300 ¹	0.016	0.042	0.011	0.018	0.032	0.012	0.023	0.028	0.013	0.034	0.024	0.014	0.043	0.023	0.015	0.055	0.023	0.016	0.068	0.025	0.018	0.088	0.024	0.024
134	09505350 ¹	0.007	0.041	0.006	0.008	0.031	0.006	0.011	0.027	0.008	0.016	0.022	0.009	0.021	0.021	0.011	0.028	0.021	0.012	0.036	0.022	0.014	0.048	0.022	0.022
135	09505800 ¹	0.009	0.041	0.008	0.009	0.031	0.007	0.013	0.027	0.009	0.020	0.022	0.011	0.026	0.021	0.012	0.033	0.022	0.013	0.040	0.023	0.014	0.051	0.022	0.022
136	09506000	0.010	0.043	0.008	0.012	0.033	0.009	0.016	0.029	0.010	0.023	0.024	0.012	0.031	0.024	0.013	0.040	0.024	0.015	0.051	0.025	0.017	0.068	0.025	0.025
137	09507600	0.027	0.043	0.017	0.033	0.033	0.016	0.043	0.029	0.017	0.061	0.024	0.017	0.076	0.024	0.018	0.094	0.024	0.019	0.113	0.025	0.021	0.141	0.025	0.025
138	09507700 ¹	0.024	0.045	0.016	0.029	0.035	0.016	0.039	0.031	0.017	0.055	0.027	0.018	0.070	0.026	0.019	0.087	0.027	0.021	0.106	0.029	0.023	0.135	0.029	0.029
139	09507980 ¹	0.007	0.043	0.006	0.005	0.033	0.004	0.008	0.029	0.006	0.013	0.024	0.008	0.017	0.023	0.010	0.021	0.024	0.011	0.025	0.025	0.013	0.032	0.025	0.025
140	09508300 ¹	0.010	0.042	0.008	0.012	0.032	0.009	0.016	0.028	0.010	0.022	0.023	0.011	0.029	0.023	0.013	0.038	0.023	0.014	0.048	0.024	0.016	0.065	0.024	0.024
141	09508500 ¹	0.006	0.043	0.005	0.007	0.033	0.005	0.009	0.029	0.007	0.013	0.024	0.008	0.018	0.023	0.010	0.023	0.023	0.012	0.030	0.024	0.014	0.041	0.024	0.024
142	09510070	0.045	0.043	0.022	0.056	0.033	0.021	0.076	0.029	0.021	0.109	0.024	0.020	0.136	0.024	0.020	0.167	0.024	0.021	0.199	0.025	0.022	0.247	0.025	0.025
143	09510080 ¹	0.072	0.045	0.028	0.086	0.035	0.025	0.113	0.031	0.024	0.159	0.026	0.023	0.201	0.026	0.023	0.248	0.026	0.024	0.301	0.028	0.026	0.380	0.028	0.028
144	09510100 ¹	0.031	0.046	0.018	0.036	0.035	0.018	0.047	0.031	0.019	0.067	0.027	0.019	0.087	0.026	0.020	0.110	0.027	0.022	0.137	0.029	0.024	0.178	0.029	0.029
145	09510150 ¹	0.033	0.045	0.019	0.042	0.034	0.019	0.055	0.030	0.020	0.080	0.026	0.019	0.102	0.025	0.020	0.127	0.025	0.021	0.156	0.027	0.023	0.199	0.027	0.027
146	09510200 ¹	0.011	0.045	0.009	0.013	0.035	0.009	0.017	0.030	0.011	0.024	0.026	0.012	0.032	0.025	0.014	0.041	0.026	0.016	0.053	0.027	0.018	0.071	0.027	0.027
147	09512100	0.037	--	--	0.046	--	--	0.063	--	--	0.091	--	--	0.117	--	--	0.146	--	--	0.180	--	--	0.232	--	--
148	09512162	0.017	--	--	0.021	--	--	0.028	--	--	0.040	--	--	0.052	--	--	0.065	--	--	0.080	--	--	0.103	--	--
149	09512165	0.104	--	--	0.130	--	--	0.179	--	--	0.259	--	--	0.331	--	--	0.411	--	--	0.500	--	--	0.632	--	--
150	09512200	0.035	--	--	0.028	--	--	0.044	--	--	0.071	--	--	0.092	--	--	0.114	--	--	0.138	--	--	0.172	--	--
151	09512280 ¹	0.016	0.043	0.012	0.021	0.033	0.013	0.030	0.029	0.015	0.045	0.024	0.016	0.059	0.023	0.017	0.077	0.024	0.018	0.098	0.025	0.020	0.130	0.026	0.026
152	09512400	0.007	0.043	0.006	0.008	0.033	0.007	0.011	0.029	0.008	0.017	0.024	0.010	0.022	0.024	0.011	0.027	0.024	0.013	0.034	0.025	0.014	0.044	0.025	0.025
153	09512450	0.026	0.043	0.016	0.032	0.033	0.016	0.043	0.029	0.017	0.060	0.024	0.017	0.075	0.024	0.018	0.093	0.024	0.019	0.111	0.025	0.021	0.139	0.025	0.025
154	09512500 ¹	0.004	0.041	0.004	0.006	0.031	0.005	0.008	0.027	0.006	0.012	0.022	0.008	0.017	0.021	0.009	0.023	0.021	0.011	0.030	0.023	0.013	0.042	0.022	0.022
155	09512600 ¹	0.013	0.041	0.010	0.016	0.031	0.011	0.022	0.027	0.012	0.031	0.022	0.013	0.039	0.022	0.014	0.048	0.022	0.015	0.058	0.023	0.016	0.073	0.023	0.023
156	09512800	0.013	0.043	0.010	0.018	0.033	0.011	0.024	0.029	0.013	0.037	0.024	0.015	0.050	0.024	0.016	0.066	0.024	0.018	0.085	0.025	0.019	0.114	0.025	0.025
157	09512860 ¹	0.038	0.043	0.020	0.052	0.033	0.020	0.072	0.029	0.020	0.104	0.024	0.020	0.134	0.023	0.020	0.167	0.024	0.021	0.204	0.025	0.022	0.258	0.025	0.025
158	09513780 ¹	0.014	0.043	0.010	0.016	0.033	0.011	0.022	0.028	0.012	0.032	0.024	0.014	0.042	0.023	0.015	0.053	0.023	0.016	0.067	0.025	0.018	0.088	0.025	0.025
159	09513800	0.034	0.043	0.019	0.041	0.033	0.018	0.054	0.029	0.019	0.077	0.024	0.018	0.098	0.024	0.019	0.122	0.024	0.020	0.150	0.025	0.022	0.191	0.025	0.025
160	09513835 ¹	0.031	0.046	0.018	0.031	0.035	0.017	0.047	0.032	0.019	0.071	0.027	0.020	0.092	0.027	0.021	0.113	0.027	0.022	0.135	0.029	0.024	0.167	0.032	0.032
161	09513860 ¹	0.009	0.051	0.008	0.010	0.040	0.008	0.015	0.036	0.010	0.022	0.031	0.013	0.028	0.031	0.015	0.036	0.032	0.017	0.044	0.035	0.019	0.058	0.025	0.025
162	09513910	0.011	--	--	0.014	--	--	0.020	--	--	0.030	--	--	0.039	--	--	0.049	--	--	0.060	--	--	0.078	--	--
163	09513970	0.106	--	--	0.131	--	--	0.176	--	--	0.252	--	--	0.321	--	--	0.399	--	--	0.487	--	--	0.616	--	--
164	09515500 ¹	0.011	0.041	0.009	0.012	0.031	0.009	0.018	0.027	0.011	0.026	0.022	0.012	0.034	0.022	0.013	0.043	0.022	0.014	0.053	0.023	0.016	0.069	0.023	0.023
165	09516500	0.030	0.043	0.018	0.035	0.033	0.017	0.044	0.029	0.017	0.063	0.024	0.017	0.081	0.024	0.018	0.105	0.024	0.019	0.133	0.025	0.021	0.177	0.025	0.025
166	09517000	0.013	--	--	0.017	--	--	0.024	--	--	0.035	--	--	0.046	--	--	0.059	--	--	0.073	--	--	0.095	--	--
167	09517490	0.035	--	--	0.041	--	--	0.053	--	--	0.076	--	--	0.097	--	--	0.123	--	--	0.152	--	--	0.197	--	--
168	09517500	0.036	--	--	0.042	--	--	0.059	--	--	0.086	--	--	0.110	--	--	0.136	--	--	0.165	--	--	0.207	--	--
169	09520170	0.011	--	--	0.013	--	--	0.017	--	--	0.023	--	--	0.029	--	--	0.036	--	--	0.044	--	--	0.055	--	--
170	09535100	0.007	--	--	0.009	--	--	0.012	--	--	0.018	--	--	0.025	--	--	0.032	--	--	0.042	--	--	0.056	--	--
171	09535300	0.006	--	--	0.007	--	--	0.010	--	--	0.016	--	--	0.021	--	--	0.028	--	--	0.035	--	--	0.048	--	--
172	09537200	0.017	--	--	0.015	--	--	0.023	--	--	0.035	--	--	0.046	--	--	0.057	--	--	0.069	--	--	0.086	--	--
173	09537500	0.001	--	--	0.002	--	--	0.002	--	--	0.003	--	--	0.004	--	--	0.006	--	--	0.007	--	--	0.010	--	--

¹Station used to develop regional regression equations.

133	09505300 ¹	0.009	0.045	0.008	0.010	0.032	0.008	0.014	0.028	0.009	0.021	0.027	0.012	0.027	0.026	0.013	0.034	0.027	0.015	0.042	0.028	0.017	0.054	0.021	0.021
134	09505350 ¹	0.005	0.044	0.004	0.006	0.031	0.005	0.007	0.026	0.006	0.011	0.026	0.008	0.014	0.025	0.009	0.019	0.025	0.011	0.025	0.026	0.013	0.035	0.019	0.019
135	09505800 ¹	0.008	0.044	0.007	0.008	0.031	0.006	0.011	0.026	0.008	0.016	0.026	0.010	0.021	0.025	0.011	0.026	0.025	0.013	0.033	0.026	0.015	0.045	0.019	0.019
136	09506000	0.008	0.046	0.007	0.010	0.033	0.008	0.014	0.028	0.009	0.021	0.028	0.012	0.028	0.027	0.014	0.038	0.028	0.016	0.049	0.029	0.018	0.066	0.022	0.022
137	09507600	0.028	0.046	0.017	0.033	0.033	0.016	0.042	0.028	0.017	0.059	0.028	0.019	0.075	0.027	0.020	0.094	0.028	0.021	0.115	0.029	0.023	0.146	0.022	0.022
138	09507700 ¹	0.020	0.048	0.014	0.023	0.034	0.014	0.029	0.030	0.015	0.042	0.030	0.017	0.054	0.029	0.019	0.069	0.030	0.021	0.086	0.031	0.023	0.112	0.024	0.024
139	09507980 ¹	0.010	0.044	0.008	0.008	0.033	0.006	0.011	0.029	0.008	0.016	0.029	0.010	0.021	0.027	0.012	0.026	0.028	0.013	0.032	0.029	0.015	0.043	0.022	0.022
140	09508300 ¹	0.010	0.045	0.008	0.011	0.032	0.008	0.013	0.028	0.009	0.019	0.028	0.011	0.025	0.026	0.013	0.034	0.027	0.015	0.044	0.028	0.017	0.061	0.021	0.021
141	09508500 ¹	0.006	0.047	0.005	0.005	0.033	0.004	0.007	0.028	0.006	0.010	0.028	0.008	0.013	0.027	0.009	0.017	0.027	0.011	0.022	0.028	0.012	0.030	0.021	0.021
142	09510070	0.017	0.046	0.012	0.010	0.033	0.008	0.017	0.028	0.011	0.028	0.028	0.014	0.037	0.027	0.016	0.045	0.028	0.017	0.053	0.029	0.019	0.063	0.022	0.022
143	09510080 ¹	0.067	0.047	0.027	0.075	0.034	0.024	0.097	0.030	0.023	0.137	0.030	0.025	0.176	0.029	0.025	0.222	0.030	0.026	0.275	0.031	0.028	0.355	0.025	0.025
144	09510100 ¹	0.028	0.048	0.018	0.032	0.035	0.017	0.043	0.031	0.018	0.061	0.031	0.020	0.078	0.030	0.021	0.098	0.031	0.023	0.123	0.032	0.025	0.161	0.025	0.025
145	09510150 ¹	0.034	0.045	0.019	0.040	0.034	0.019	0.053	0.030	0.019	0.076	0.030	0.022	0.099	0.029	0.022	0.126	0.030	0.024	0.158	0.031	0.026	0.207	0.024	0.024
146	09510200 ¹	0.011	0.044	0.009	0.012	0.034	0.009	0.016	0.030	0.010	0.023	0.030	0.013	0.030	0.029	0.015	0.040	0.030	0.017	0.052	0.031	0.020	0.073	0.024	0.024
147	09512100	0.040	--	--	0.047	--	--	0.062	--	--	0.089	--	--	0.115	--	--	0.147	--	--	0.185	--	--	0.246	--	--
148	09512162	0.017	--	--	0.021	--	--	0.028	--	--	0.041	--	--	0.054	--	--	0.070	--	--	0.088	--	--	0.117	--	--
149	09512165	0.123	--	--	0.151	--	--	0.203	--	--	0.293	--	--	0.378	--	--	0.477	--	--	0.591	--	--	0.768	--	--
150	09512200	0.034	--	--	0.029	--	--	0.043	--	--	0.065	--	--	0.083	--	--	0.103	--	--	0.127	--	--	0.165	--	--
151	09512280 ¹	0.016	0.046	0.012	0.021	0.033	0.013	0.029	0.029	0.014	0.045	0.029	0.017	0.061	0.027	0.019	0.081	0.028	0.021	0.106	0.029	0.023	0.146	0.023	0.023
152	09512400	0.011	0.046	0.009	0.012	0.033	0.009	0.017	0.028	0.011	0.025	0.028	0.013	0.032	0.027	0.015	0.041	0.028	0.017	0.051	0.029	0.018	0.068	0.022	0.022
153	09512450	0.030	0.046	0.018	0.035	0.033	0.017	0.046	0.028	0.018	0.065	0.028	0.020	0.082	0.027	0.020	0.103	0.028	0.022	0.126	0.029	0.023	0.161	0.022	0.022
154	09512500 ¹	0.004	0.044	0.004	0.006	0.031	0.005	0.008	0.027	0.006	0.013	0.027	0.009	0.019	0.025	0.011	0.026	0.026	0.013	0.035	0.026	0.015	0.049	0.020	0.020
155	09512600 ¹	0.018	0.044	0.013	0.021	0.031	0.013	0.028	0.027	0.014	0.041	0.027	0.016	0.052	0.025	0.017	0.066	0.026	0.019	0.081	0.027	0.020	0.104	0.020	0.020
156	09512800	0.015	0.046	0.011	0.019	0.033	0.012	0.026	0.028	0.014	0.042	0.028	0.017	0.058	0.027	0.018	0.078	0.028	0.021	0.102	0.029	0.022	0.140	0.022	0.022
157	09512860 ¹	0.040	0.045	0.021	0.055	0.033	0.020	0.075	0.028	0.021	0.112	0.028	0.023	0.146	0.027	0.023	0.186	0.028	0.024	0.231	0.029	0.026	0.300	0.023	0.023
158	09513780 ¹	0.019	0.045	0.013	0.020	0.032	0.012	0.025	0.028	0.013	0.035	0.028	0.016	0.048	0.027	0.017	0.063	0.028	0.019	0.082	0.029	0.021	0.114	0.023	0.023
159	09513800	0.041	0.046	0.022	0.046	0.033	0.019	0.059	0.028	0.019	0.083	0.028	0.021	0.107	0.027	0.022	0.137	0.028	0.023	0.171	0.029	0.025	0.226	0.022	0.022
160	09513835 ¹	0.040	0.049	0.022	0.042	0.035	0.019	0.059	0.031	0.020	0.086	0.032	0.023	0.108	0.030	0.024	0.134	0.032	0.026	0.162	0.033	0.027	0.206	0.028	0.028
161	09513860 ¹	0.012	0.055	0.010	0.013	0.040	0.010	0.017	0.035	0.012	0.025	0.036	0.015	0.032	0.035	0.017	0.042	0.037	0.020	0.053	0.038	0.022	0.072	0.022	0.022
162	09513910	0.013	--	--	0.016	--	--	0.022	--	--	0.031	--	--	0.041	--	--	0.052	--	--	0.066	--	--	0.088	--	--
163	09513970	0.137	--	--	0.163	--	--	0.220	--	--	0.322	--	--	0.418	--	--	0.530	--	--	0.657	--	--	0.850	--	--
164	09515500 ¹	0.015	0.044	0.011	0.016	0.031	0.011	0.022	0.027	0.012	0.032	0.027	0.015	0.042	0.025	0.016	0.054	0.026	0.018	0.068	0.027	0.019	0.092	0.020	0.020
165	09516500	0.022	0.046	0.015	0.026	0.033	0.014	0.032	0.028	0.015	0.046	0.028	0.018	0.061	0.027	0.019	0.081	0.028	0.021	0.106	0.029	0.023	0.148	0.022	0.022
166	09517000	0.009	--	--	0.011	--	--	0.016	--	--	0.024	--	--	0.032	--	--	0.042	--	--	0.054	--	--	0.072	--	--
167	09517490	0.021	--	--	0.014	--	--	0.024	--	--	0.039	--	--	0.051	--	--	0.064	--	--	0.078	--	--	0.100	--	--
168	09517500	0.033	--	--	0.038	--	--	0.051	--	--	0.073	--	--	0.093	--	--	0.116	--	--	0.143	--	--	0.184	--	--
169	09520170	0.007	--	--	0.009	--	--	0.012	--	--	0.018	--	--	0.023	--	--	0.028	--	--	0.034	--	--	0.043	--	--
170	09535100	0.007	--	--	0.009	--	--	0.013	--	--	0.020	--	--	0.028	--	--	0.037	--	--	0.049	--	--	0.069	--	--
171	09535300	0.006	--	--	0.008	--	--	0.011	--	--	0.017	--	--	0.024	--	--	0.032	--	--	0.042	--	--	0.058	--	--
172	09537200	0.016	--	--	0.015	--	--	0.021	--	--	0.030	--	--	0.039	--	--	0.048	--	--	0.060	--	--	0.080	--	--
173	09537500	0.002	--	--	0.002	--	--	0.003	--	--	0.004	--	--	0.005	--	--	0.007	--	--	0.009	--	--	0.013	--	--

¹Station used to develop regional regression equations.

133	09505300 ¹	0.008	0.046	0.007	0.009	0.031	0.007	0.012	0.022	0.008	0.018	0.021	0.010	0.023	0.018	0.010	0.029	0.019	0.011	0.036	0.019	0.013	0.046	0.021	0.021
134	09505350 ¹	0.004	0.045	0.003	0.004	0.029	0.004	0.006	0.021	0.005	0.008	0.020	0.006	0.011	0.017	0.007	0.014	0.017	0.008	0.019	0.018	0.009	0.026	0.019	0.019
135	09505800 ¹	0.006	0.045	0.005	0.006	0.030	0.005	0.008	0.021	0.006	0.012	0.020	0.008	0.016	0.017	0.008	0.020	0.017	0.009	0.025	0.018	0.010	0.033	0.019	0.019
136	09506000	0.007	0.047	0.006	0.008	0.032	0.007	0.011	0.023	0.008	0.017	0.022	0.009	0.023	0.019	0.010	0.030	0.020	0.012	0.038	0.020	0.013	0.052	0.022	0.022
137	09507600	0.004	0.047	0.004	0.004	0.032	0.004	0.007	0.023	0.005	0.011	0.022	0.007	0.014	0.019	0.008	0.017	0.020	0.009	0.021	0.020	0.010	0.025	0.022	0.022
138	09507700 ¹	0.017	0.049	0.013	0.019	0.033	0.012	0.025	0.024	0.012	0.035	0.023	0.014	0.046	0.020	0.014	0.057	0.021	0.015	0.071	0.022	0.017	0.092	0.024	0.024
139	09507980 ¹	0.008	0.045	0.007	0.008	0.032	0.007	0.011	0.023	0.007	0.016	0.022	0.009	0.021	0.019	0.010	0.028	0.020	0.012	0.037	0.020	0.013	0.051	0.022	0.022
140	09508300 ¹	0.009	0.046	0.008	0.010	0.031	0.008	0.013	0.023	0.008	0.018	0.021	0.010	0.024	0.019	0.011	0.032	0.019	0.012	0.041	0.020	0.013	0.056	0.021	0.021
141	09508500 ¹	0.004	0.047	0.004	0.005	0.031	0.004	0.007	0.023	0.005	0.010	0.021	0.007	0.014	0.019	0.008	0.019	0.019	0.009	0.025	0.019	0.011	0.034	0.021	0.021
142	09510070	0.012	0.047	0.010	0.007	0.032	0.006	0.012	0.023	0.008	0.021	0.022	0.011	0.028	0.019	0.011	0.035	0.020	0.013	0.041	0.020	0.014	0.050	0.022	0.022
143	09510080 ¹	0.060	0.048	0.027	0.070	0.033	0.023	0.091	0.025	0.019	0.128	0.023	0.020	0.163	0.021	0.019	0.205	0.022	0.019	0.251	0.022	0.020	0.322	0.024	0.024
144	09510100 ¹	0.036	0.049	0.021	0.040	0.034	0.018	0.051	0.025	0.017	0.074	0.024	0.018	0.097	0.022	0.018	0.124	0.022	0.019	0.157	0.023	0.020	0.208	0.025	0.025
145	09510150 ¹	0.034	0.045	0.019	0.040	0.033	0.018	0.052	0.025	0.017	0.074	0.023	0.018	0.096	0.021	0.017	0.121	0.021	0.018	0.150	0.022	0.019	0.194	0.024	0.024
146	09510200 ¹	0.014	0.045	0.011	0.014	0.033	0.010	0.018	0.025	0.010	0.027	0.024	0.013	0.036	0.021	0.013	0.049	0.022	0.015	0.064	0.023	0.017	0.088	0.025	0.025
147	09512100	0.041	--	--	0.048	--	--	0.063	--	--	0.090	--	--	0.116	--	--	0.147	--	--	0.183	--	--	0.240	--	--
148	09512162	0.017	--	--	0.024	--	--	0.032	--	--	0.049	--	--	0.064	--	--	0.083	--	--	0.105	--	--	0.138	--	--
149	09512165	0.139	--	--	0.173	--	--	0.234	--	--	0.340	--	--	0.436	--	--	0.548	--	--	0.675	--	--	0.867	--	--
150	09512200	0.035	--	--	0.030	--	--	0.045	--	--	0.068	--	--	0.087	--	--	0.109	--	--	0.133	--	--	0.170	--	--
151	09512280 ¹	0.015	0.046	0.011	0.020	0.032	0.012	0.027	0.024	0.013	0.041	0.022	0.014	0.056	0.020	0.015	0.074	0.020	0.016	0.096	0.021	0.017	0.130	0.023	0.023
152	09512400	0.018	0.047	0.013	0.021	0.032	0.012	0.027	0.023	0.013	0.039	0.022	0.014	0.051	0.019	0.014	0.065	0.020	0.015	0.082	0.020	0.016	0.108	0.022	0.022
153	09512450	0.027	0.047	0.017	0.032	0.032	0.016	0.042	0.023	0.015	0.059	0.022	0.016	0.075	0.019	0.015	0.093	0.020	0.016	0.114	0.020	0.017	0.144	0.022	0.022
154	09512500 ¹	0.005	0.045	0.004	0.006	0.030	0.005	0.008	0.022	0.006	0.013	0.020	0.008	0.019	0.018	0.009	0.025	0.018	0.011	0.034	0.018	0.012	0.047	0.020	0.020
155	09512600 ¹	0.021	0.045	0.014	0.027	0.030	0.014	0.036	0.022	0.014	0.052	0.020	0.015	0.067	0.018	0.014	0.084	0.018	0.015	0.103	0.019	0.016	0.132	0.020	0.020
156	09512800	0.014	0.047	0.011	0.019	0.032	0.012	0.026	0.023	0.012	0.041	0.022	0.014	0.056	0.019	0.014	0.075	0.020	0.016	0.098	0.020	0.017	0.133	0.022	0.022
157	09512860 ¹	0.044	0.046	0.022	0.060	0.032	0.021	0.082	0.023	0.018	0.122	0.022	0.019	0.158	0.020	0.018	0.199	0.020	0.018	0.247	0.021	0.019	0.318	0.023	0.023
158	09513780 ¹	0.018	0.046	0.013	0.020	0.031	0.012	0.025	0.023	0.012	0.036	0.022	0.014	0.048	0.020	0.014	0.064	0.020	0.015	0.083	0.021	0.016	0.113	0.022	0.022
159	09513800	0.045	0.047	0.023	0.052	0.032	0.020	0.067	0.023	0.017	0.094	0.022	0.018	0.121	0.019	0.016	0.153	0.020	0.017	0.190	0.020	0.018	0.248	0.022	0.022
160	09513835 ¹	0.037	0.050	0.021	0.034	0.034	0.017	0.051	0.026	0.017	0.077	0.025	0.019	0.098	0.023	0.019	0.121	0.023	0.019	0.145	0.024	0.021	0.180	0.026	0.026
161	09513860 ¹	0.013	0.056	0.010	0.014	0.038	0.010	0.019	0.029	0.011	0.027	0.028	0.014	0.035	0.019	0.012	0.045	0.027	0.017	0.057	0.029	0.019	0.076	0.032	0.032
162	09513910	0.013	--	--	0.017	--	--	0.022	--	--	0.032	--	--	0.041	--	--	0.052	--	--	0.065	--	--	0.085	--	--
163	09513970	0.159	--	--	0.191	--	--	0.264	--	--	0.394	--	--	0.512	--	--	0.648	--	--	0.799	--	--	1.025	--	--
164	09515500 ¹	0.014	0.045	0.011	0.018	0.030	0.011	0.026	0.022	0.012	0.040	0.021	0.014	0.055	0.018	0.014	0.073	0.018	0.015	0.094	0.019	0.016	0.129	0.020	0.020
165	09516500	0.029	0.047	0.018	0.033	0.032	0.016	0.041	0.023	0.015	0.058	0.022	0.016	0.077	0.019	0.015	0.100	0.020	0.016	0.130	0.020	0.017	0.177	0.022	0.022
166	09517000	0.005	--	--	0.006	--	--	0.009	--	--	0.013	--	--	0.018	--	--	0.023	--	--	0.029	--	--	0.039	--	--
167	09517490	0.037	--	--	0.041	--	--	0.051	--	--	0.072	--	--	0.093	--	--	0.119	--	--	0.150	--	--	0.198	--	--
168	09517500	0.034	--	--	0.039	--	--	0.053	--	--	0.077	--	--	0.098	--	--	0.122	--	--	0.150	--	--	0.191	--	--
169	09520170	0.009	--	--	0.011	--	--	0.014	--	--	0.020	--	--	0.026	--	--	0.032	--	--	0.039	--	--	0.050	--	--
170	09535100	0.007	--	--	0.008	--	--	0.011	--	--	0.017	--	--	0.023	--	--	0.030	--	--	0.039	--	--	0.053	--	--
171	09535300	0.006	--	--	0.007	--	--	0.010	--	--	0.016	--	--	0.021	--	--	0.029	--	--	0.037	--	--	0.051	--	--
172	09537200	0.015	--	--	0.014	--	--	0.021	--	--	0.031	--	--	0.040	--	--	0.050	--	--	0.061	--	--	0.079	--	--
173	09537500	0.003	--	--	0.003	--	--	0.004	--	--	0.005	--	--	0.007	--	--	0.010	--	--	0.013	--	--	0.018	--	--

¹Station used to develop regional regression equations.

133	09505300 ¹	0.007	0.040	0.006	0.008	0.028	0.006	0.011	0.023	0.007	0.016	0.021	0.009	0.021	0.018	0.010	0.026	0.018	0.011	0.033	0.017	0.011	0.044	0.014	0.014
134	09505350 ¹	0.003	0.039	0.003	0.004	0.027	0.003	0.005	0.022	0.004	0.007	0.020	0.005	0.009	0.017	0.006	0.012	0.016	0.007	0.016	0.016	0.008	0.022	0.013	0.013
135	09505800 ¹	0.006	0.039	0.006	0.005	0.027	0.005	0.008	0.022	0.006	0.011	0.020	0.007	0.015	0.017	0.008	0.019	0.016	0.009	0.024	0.016	0.009	0.032	0.012	0.012
136	09506000	0.007	0.040	0.006	0.009	0.029	0.007	0.012	0.023	0.008	0.019	0.022	0.010	0.026	0.019	0.011	0.034	0.018	0.012	0.045	0.018	0.013	0.062	0.015	0.015
137	09507600	0.002	0.040	0.002	0.001	0.029	0.001	0.002	0.023	0.002	0.004	0.022	0.003	0.005	0.019	0.004	0.006	0.018	0.004	0.007	0.018	0.005	0.009	0.015	0.015
138	09507700 ¹	0.014	0.043	0.012	0.016	0.030	0.011	0.021	0.024	0.011	0.030	0.023	0.013	0.039	0.020	0.013	0.049	0.020	0.014	0.062	0.019	0.015	0.081	0.016	0.016
139	09507980 ¹	0.007	0.039	0.007	0.008	0.029	0.006	0.010	0.024	0.007	0.015	0.022	0.009	0.021	0.019	0.010	0.029	0.019	0.011	0.038	0.018	0.012	0.052	0.015	0.015
140	09508300 ¹	0.009	0.040	0.008	0.010	0.028	0.007	0.012	0.023	0.008	0.017	0.021	0.010	0.023	0.019	0.010	0.030	0.018	0.011	0.039	0.018	0.012	0.054	0.015	0.015
141	09508500 ¹	0.004	0.041	0.003	0.004	0.028	0.004	0.005	0.023	0.004	0.008	0.021	0.006	0.010	0.018	0.007	0.014	0.018	0.008	0.019	0.017	0.009	0.026	0.014	0.014
142	09510070	0.033	0.040	0.019	0.040	0.029	0.017	0.053	0.023	0.016	0.075	0.022	0.017	0.095	0.019	0.016	0.117	0.018	0.016	0.144	0.018	0.016	0.184	0.015	0.015
143	09510080 ¹	0.057	0.042	0.034	0.065	0.030	0.021	0.083	0.025	0.019	0.119	0.023	0.020	0.153	0.021	0.018	0.194	0.020	0.018	0.240	0.020	0.018	0.312	0.017	0.017
144	09510100 ¹	0.024	0.043	0.019	0.027	0.031	0.014	0.037	0.026	0.015	0.053	0.024	0.017	0.068	0.021	0.016	0.086	0.021	0.017	0.107	0.021	0.017	0.140	0.018	0.018
145	09510150 ¹	0.036	0.039	0.024	0.041	0.030	0.017	0.054	0.025	0.017	0.077	0.023	0.018	0.100	0.021	0.017	0.128	0.020	0.017	0.160	0.020	0.018	0.210	0.017	0.017
146	09510200 ¹	0.011	0.039	0.010	0.013	0.030	0.009	0.017	0.025	0.010	0.024	0.024	0.012	0.032	0.021	0.013	0.042	0.021	0.014	0.055	0.020	0.015	0.077	0.017	0.017
147	09512100	0.044	--	--	0.051	--	--	0.067	--	--	0.095	--	--	0.124	--	--	0.160	--	--	0.202	--	--	0.269	--	--
148	09512162	0.018	--	--	0.024	--	--	0.034	--	--	0.051	--	--	0.068	--	--	0.089	--	--	0.114	--	--	0.152	--	--
149	09512165	0.144	--	--	0.182	--	--	0.248	--	--	0.362	--	--	0.471	--	--	0.599	--	--	0.748	--	--	0.979	--	--
150	09512200	0.035	--	--	0.030	--	--	0.045	--	--	0.066	--	--	0.085	--	--	0.106	--	--	0.132	--	--	0.173	--	--
151	09512280 ¹	0.015	0.041	0.012	0.020	0.029	0.012	0.027	0.024	0.013	0.042	0.022	0.015	0.057	0.020	0.015	0.077	0.019	0.015	0.101	0.019	0.016	0.139	0.016	0.016
152	09512400	0.020	0.040	0.014	0.023	0.029	0.013	0.030	0.023	0.013	0.043	0.022	0.014	0.057	0.019	0.014	0.073	0.018	0.015	0.094	0.018	0.015	0.127	0.015	0.015
153	09512450	0.022	0.040	0.015	0.026	0.029	0.014	0.034	0.023	0.014	0.048	0.022	0.015	0.061	0.019	0.014	0.076	0.018	0.015	0.094	0.018	0.015	0.120	0.015	0.015
154	09512500 ¹	0.004	0.039	0.004	0.005	0.027	0.005	0.008	0.022	0.006	0.013	0.020	0.008	0.018	0.018	0.009	0.025	0.017	0.010	0.033	0.016	0.011	0.047	0.013	0.013
155	09512600 ¹	0.021	0.039	0.016	0.028	0.027	0.014	0.039	0.022	0.014	0.058	0.020	0.015	0.077	0.018	0.014	0.098	0.017	0.015	0.123	0.017	0.015	0.162	0.014	0.014
156	09512800	0.015	0.040	0.011	0.019	0.029	0.011	0.026	0.023	0.012	0.042	0.022	0.014	0.058	0.019	0.014	0.078	0.018	0.015	0.103	0.018	0.015	0.142	0.015	0.015
157	09512860 ¹	0.050	0.040	0.030	0.068	0.029	0.020	0.094	0.024	0.019	0.140	0.022	0.019	0.183	0.020	0.018	0.233	0.019	0.018	0.292	0.019	0.018	0.380	0.016	0.016
158	09513780 ¹	0.019	0.040	0.015	0.021	0.029	0.012	0.025	0.024	0.012	0.037	0.022	0.014	0.050	0.020	0.014	0.066	0.019	0.015	0.087	0.019	0.015	0.120	0.016	0.016
159	09513800	0.058	0.040	0.025	0.063	0.029	0.020	0.080	0.023	0.018	0.116	0.022	0.018	0.153	0.019	0.017	0.198	0.018	0.017	0.253	0.018	0.017	0.337	0.015	0.015
160	09513835 ¹	0.041	0.044	0.026	0.039	0.031	0.017	0.057	0.026	0.018	0.084	0.025	0.019	0.107	0.023	0.019	0.132	0.022	0.019	0.160	0.022	0.019	0.203	0.019	0.019
161	09513860 ¹	0.014	0.049	0.011	0.015	0.035	0.010	0.020	0.030	0.012	0.028	0.029	0.014	0.037	0.026	0.015	0.048	0.026	0.017	0.061	0.026	0.018	0.084	0.024	0.024
162	09513910	0.017	--	--	0.023	--	--	0.031	--	--	0.047	--	--	0.063	--	--	0.083	--	--	0.107	--	--	0.145	--	--
163	09513970	0.172	--	--	0.202	--	--	0.284	--	--	0.433	--	--	0.571	--	--	0.730	--	--	0.909	--	--	1.179	--	--
164	09515500 ¹	0.015	0.039	0.012	0.020	0.027	0.011	0.027	0.022	0.012	0.043	0.021	0.014	0.060	0.018	0.014	0.080	0.017	0.014	0.105	0.017	0.014	0.145	0.014	0.014
165	09516500	0.028	0.040	0.017	0.032	0.029	0.015	0.041	0.023	0.015	0.060	0.022	0.016	0.081	0.019	0.015	0.109	0.018	0.016	0.143	0.018	0.016	0.199	0.015	0.015
166	09517000	0.003	--	--	0.004	--	--	0.006	--	--	0.009	--	--	0.012	--	--	0.015	--	--	0.019	--	--	0.026	--	--
167	09517490	0.019	--	--	0.014	--	--	0.023	--	--	0.036	--	--	0.047	--	--	0.060	--	--	0.074	--	--	0.095	--	--
168	09517500	0.034	--	--	0.039	--	--	0.052	--	--	0.075	--	--	0.096	--	--	0.121	--	--	0.150	--	--	0.194	--	--
169	09520170	0.006	--	--	0.008	--	--	0.010	--	--	0.015	--	--	0.019	--	--	0.024	--	--	0.029	--	--	0.038	--	--
170	09535100	0.007	--	--	0.009	--	--	0.011	--	--	0.017	--	--	0.024	--	--	0.032	--	--	0.042	--	--	0.058	--	--
171	09535300	0.005	--	--	0.007	--	--	0.009	--	--	0.015	--	--	0.020	--	--	0.027	--	--	0.036	--	--	0.049	--	--
172	09537200	0.017	--	--	0.019	--	--	0.025	--	--	0.035	--	--	0.046	--	--	0.059	--	--	0.077	--	--	0.105	--	--
173	09537500	0.003	--	--	0.003	--	--	0.004	--	--	0.006	--	--	0.008	--	--	0.011	--	--	0.015	--	--	0.021	--	--

¹Station used to develop regional regression equations.

133	09505300 ¹	0.009	0.041	0.007	0.010	0.031	0.007	0.013	0.025	0.008	0.018	0.023	0.010	0.024	0.020	0.011	0.030	0.018	0.011	0.039	0.017	0.012	0.053	0.013	0.013
134	09505350 ¹	0.004	0.040	0.003	0.004	0.030	0.004	0.005	0.024	0.004	0.007	0.022	0.005	0.009	0.019	0.006	0.013	0.017	0.007	0.017	0.015	0.008	0.024	0.011	0.011
135	09505800 ¹	0.006	0.040	0.005	0.006	0.030	0.005	0.008	0.024	0.006	0.011	0.022	0.007	0.014	0.019	0.008	0.018	0.017	0.009	0.024	0.015	0.009	0.034	0.011	0.011
136	09506000	0.006	0.041	0.006	0.007	0.032	0.006	0.010	0.026	0.007	0.015	0.024	0.009	0.021	0.021	0.011	0.029	0.019	0.012	0.038	0.017	0.012	0.053	0.013	0.013
137	09507600	0.001	0.041	0.001	0.001	0.032	0.001	0.001	0.026	0.001	0.001	0.024	0.001	0.002	0.021	0.002	0.002	0.019	0.002	0.003	0.017	0.002	0.003	0.013	0.013
138	09507700 ¹	0.015	0.044	0.011	0.016	0.034	0.011	0.021	0.027	0.012	0.030	0.025	0.014	0.040	0.022	0.014	0.051	0.020	0.015	0.066	0.019	0.015	0.088	0.014	0.014
139	09507980 ¹	0.007	0.040	0.006	0.007	0.030	0.006	0.009	0.026	0.007	0.014	0.024	0.009	0.020	0.021	0.010	0.027	0.019	0.011	0.037	0.018	0.012	0.052	0.013	0.013
140	09508300 ¹	0.007	0.041	0.006	0.006	0.031	0.005	0.008	0.025	0.006	0.011	0.023	0.008	0.015	0.020	0.008	0.019	0.019	0.009	0.024	0.017	0.010	0.034	0.013	0.013
141	09508500 ¹	0.003	0.042	0.003	0.004	0.032	0.004	0.006	0.025	0.005	0.009	0.023	0.007	0.014	0.020	0.008	0.019	0.019	0.009	0.025	0.017	0.010	0.036	0.013	0.013
142	09510070	0.056	0.041	0.024	0.062	0.032	0.021	0.078	0.026	0.019	0.112	0.024	0.020	0.147	0.021	0.018	0.188	0.019	0.017	0.238	0.017	0.016	0.314	0.013	0.013
143	09510080 ¹	0.056	0.042	0.024	0.060	0.032	0.021	0.075	0.027	0.020	0.108	0.025	0.021	0.141	0.023	0.019	0.182	0.021	0.019	0.230	0.019	0.018	0.305	0.015	0.015
144	09510100 ¹	0.024	0.044	0.015	0.024	0.034	0.014	0.032	0.028	0.015	0.046	0.026	0.017	0.059	0.023	0.017	0.075	0.021	0.017	0.093	0.020	0.016	0.124	0.016	0.016
145	09510150 ¹	0.034	0.040	0.019	0.038	0.031	0.017	0.048	0.027	0.017	0.070	0.025	0.019	0.093	0.022	0.018	0.120	0.021	0.018	0.154	0.019	0.017	0.206	0.015	0.015
146	09510200 ¹	0.014	0.040	0.011	0.014	0.030	0.010	0.017	0.028	0.010	0.024	0.026	0.012	0.033	0.023	0.014	0.045	0.021	0.015	0.061	0.020	0.015	0.087	0.016	0.016
147	09512100	0.046	--	--	0.051	--	--	0.064	--	--	0.090	--	--	0.119	--	--	0.156	--	--	0.201	--	--	0.275	--	--
148	09512162	0.018	--	--	0.024	--	--	0.034	--	--	0.052	--	--	0.071	--	--	0.095	--	--	0.123	--	--	0.168	--	--
149	09512165	0.146	--	--	0.184	--	--	0.247	--	--	0.364	--	--	0.482	--	--	0.626	--	--	0.801	--	--	1.080	--	--
150	09512200	0.037	--	--	0.032	--	--	0.045	--	--	0.064	--	--	0.081	--	--	0.103	--	--	0.131	--	--	0.180	--	--
151	09512280 ¹	0.016	0.041	0.011	0.020	0.032	0.012	0.028	0.026	0.014	0.044	0.024	0.016	0.063	0.021	0.016	0.086	0.020	0.016	0.115	0.018	0.016	0.161	0.014	0.014
152	09512400	0.024	0.041	0.015	0.025	0.032	0.014	0.031	0.026	0.014	0.044	0.024	0.015	0.059	0.021	0.015	0.078	0.019	0.015	0.103	0.017	0.015	0.144	0.013	0.013
153	09512450	0.017	0.041	0.012	0.019	0.032	0.012	0.024	0.026	0.013	0.035	0.024	0.014	0.045	0.021	0.014	0.058	0.019	0.014	0.072	0.017	0.014	0.095	0.013	0.013
154	09512500 ¹	0.004	0.040	0.004	0.005	0.031	0.004	0.007	0.024	0.006	0.012	0.022	0.008	0.018	0.019	0.009	0.024	0.018	0.010	0.033	0.016	0.011	0.047	0.012	0.012
155	09512600 ¹	0.021	0.040	0.014	0.028	0.031	0.015	0.038	0.024	0.015	0.058	0.022	0.016	0.078	0.020	0.016	0.102	0.018	0.015	0.131	0.016	0.014	0.176	0.012	0.012
156	09512800	0.014	0.041	0.011	0.018	0.032	0.012	0.026	0.026	0.013	0.041	0.024	0.015	0.059	0.021	0.015	0.080	0.019	0.015	0.107	0.017	0.015	0.150	0.013	0.013
157	09512860 ¹	0.050	0.041	0.022	0.067	0.031	0.021	0.092	0.026	0.020	0.139	0.024	0.021	0.186	0.021	0.019	0.242	0.020	0.018	0.307	0.018	0.017	0.408	0.014	0.014
158	09513780 ¹	0.023	0.041	0.015	0.017	0.031	0.011	0.024	0.026	0.013	0.034	0.024	0.014	0.043	0.021	0.014	0.055	0.020	0.014	0.070	0.018	0.014	0.097	0.014	0.014
159	09513800	0.061	0.041	0.025	0.061	0.032	0.021	0.076	0.026	0.019	0.111	0.024	0.020	0.149	0.021	0.018	0.198	0.019	0.017	0.258	0.017	0.016	0.353	0.013	0.013
160	09513835 ¹	0.037	0.045	0.020	0.035	0.034	0.017	0.049	0.029	0.018	0.071	0.027	0.019	0.089	0.024	0.019	0.111	0.023	0.019	0.138	0.021	0.018	0.183	0.017	0.017
161	09513860 ¹	0.014	0.050	0.011	0.014	0.039	0.010	0.019	0.032	0.012	0.026	0.031	0.014	0.034	0.028	0.015	0.045	0.027	0.017	0.060	0.025	0.018	0.085	0.021	0.021
162	09513910	0.021	--	--	0.025	--	--	0.032	--	--	0.047	--	--	0.062	--	--	0.082	--	--	0.107	--	--	0.149	--	--
163	09513970	0.185	--	--	0.203	--	--	0.288	--	--	0.447	--	--	0.597	--	--	0.773	--	--	0.975	--	--	1.283	--	--
164	09515500 ¹	0.015	0.040	0.011	0.019	0.030	0.012	0.026	0.025	0.013	0.042	0.023	0.015	0.060	0.020	0.015	0.082	0.018	0.015	0.109	0.016	0.014	0.152	0.012	0.012
165	09516500	0.029	0.041	0.017	0.032	0.032	0.016	0.039	0.026	0.016	0.058	0.024	0.017	0.080	0.021	0.017	0.109	0.019	0.016	0.147	0.017	0.016	0.208	0.013	0.013
166	09517000	0.002	--	--	0.003	--	--	0.004	--	--	0.006	--	--	0.008	--	--	0.010	--	--	0.013	--	--	0.018	--	--
167	09517490	0.025	--	--	0.028	--	--	0.035	--	--	0.048	--	--	0.062	--	--	0.080	--	--	0.103	--	--	0.141	--	--
168	09517500	0.034	--	--	0.038	--	--	0.048	--	--	0.068	--	--	0.087	--	--	0.111	--	--	0.140	--	--	0.186	--	--
169	09520170	0.009	--	--	0.010	--	--	0.013	--	--	0.019	--	--	0.025	--	--	0.033	--	--	0.041	--	--	0.054	--	--
170	09535100	0.006	--	--	0.008	--	--	0.011	--	--	0.018	--	--	0.026	--	--	0.036	--	--	0.048	--	--	0.068	--	--
171	09535300	0.005	--	--	0.005	--	--	0.007	--	--	0.011	--	--	0.015	--	--	0.021	--	--	0.028	--	--	0.038	--	--
172	09537200	0.019	--	--	0.019	--	--	0.024	--	--	0.035	--	--	0.049	--	--	0.067	--	--	0.089	--	--	0.125	--	--
173	09537500	0.003	--	--	0.004	--	--	0.004	--	--	0.007	--	--	0.010	--	--	0.013	--	--	0.018	--	--	0.027	--	--

¹Station used to develop regional regression equations.

